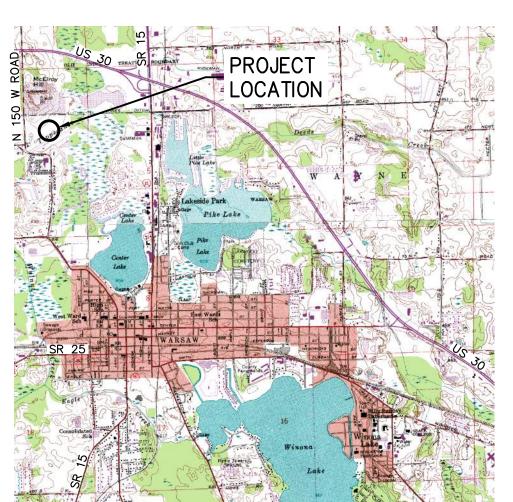
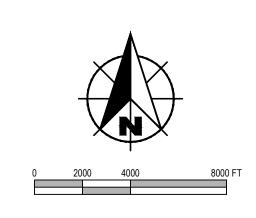
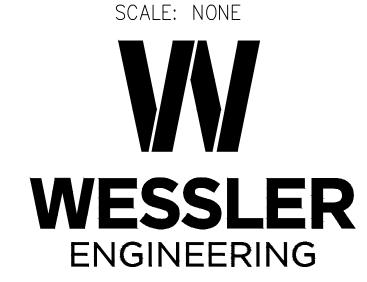
# **WASTEWATER TREATMENT PLANT EXPANSION - 2017** FOR THE CITY OF WARSAW, INDIANA





WOLUME 1
HONORABLE JOSEPH M. THANKEMER, MAYOR





STATE LOCATION MAP

More than a Project™

PROJECT NO. 162813-04-003

HONORABLE JOSEPHAM. THALLEMER, PRESIDENT WEFF GROSE GEORGE CLEMENS

JAMES EMANS, P.E., CITY ENGINEER

RIAN DAVISON, UTILITY MANAGER LARRY HYDEN, ASSISTANT UTILITY MANAGER KROTKE, ASSISTANT OPERATIONS MANAGER

**SEPTEMBER 4, 2018** 

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**ENGINEERING** 

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**INDEX TO DRAWINGS** 

PAGE NO.

ISSUE DATE

SEPTEMBER 4, 2018

PROJECT NUMBER

162813-04-003

INDEX TO DRAWINGS

- 1. THE CONTRACTOR SHALL NOTE THAT THE WORK SHOWN ON THESE DRAWINGS IS OCCURRING ON A PLANT SITE IN WHICH BURIED ELECTRICAL CONDUITS AND SMALL PIPING MAY EXIST THROUGHOUT, ALL OF WHICH IN THE VICINITY OF THIS PROJECT MAY NOT BE SHOWN ON THESE DRAWINGS. THE CONTRACTOR SHALL EXPECT TO ENCOUNTER BURIED ELECTRICAL AND COMMUNICATION WIRING, WITH OR WITHOUT CONDUITS, SMALL PIPING, AND FIELD TILE WHILE DIGGING ON THIS SITE; AND SHALL INCLUDE COSTS IN THE BID TO REPLACE, REPAIR OR RELOCATE SUCH WIRING, CONDUITS, TILE, AND PIPING WHICH ARE DAMAGED OR IN CONFLICT WITH NEW WORK.
- 2. ALL LOCATIONS, SIZES AND INVERTS OF EXISTING UTILITIES AND YARD PIPING ARE SHOWN BASED ON THE BEST INFORMATION AVAILABLE. HOWEVER, WESSLER ENGINEERING DOES NOT GUARANTEE OR ASSURE THAT SUCH INFORMATION IS TRUE OR EVEN APPROXIMATE. THE CONTRACTOR SHALL DETERMINE WHICH UTILITIES MAY CONFLICT WITH HIS WORK AND VERIFY, AT A MINIMUM BUT NOT LIMITED TO, THE LOCATIONS, MATERIALS, SIZES, AND INVERTS. THE CONTRACTOR SHALL ADJUST HIS WORK ACCORDINGLY, AND NOTIFY WESSLER ENGINEERING OF ANY SUCH CONFLICTS AND RESOLUTION. THE CONTRACTOR SHALL REFER TO APPLICABLE SECTIONS OF THE SPECIFICATIONS RELATIVE TO THE ABOVE.
- 3. THE CONTRACTOR SHALL VERIFY THE LOCATION, MATERIAL AND SIZE OF EACH EXISTING UTILITY SERVICE PROVIDED TO THE SITE WITH THE RESPECTIVE UTILITY COMPANIES, AND ADJUST HIS WORK ACCORDINGLY. THE CONTRACTOR SHALL PROVIDE WESSLER ENGINEERING RECORD DRAWING INFORMATION OF EACH EXISTING UTILITY SERVICE LOCATED.
- 4. COMPLETELY REMOVE UNDERGROUND PIPING, THAT HAS PREVIOUSLY BEEN TAKEN OUT OF SERVICE OR IS BEING TAKEN OUT OF SERVICE UNDER THIS CONTRACT, IN CONFLICT WITH NEW WORK. UNLESS OTHERWISE NOTED ON THE DRAWINGS, ABANDONED UNDERGROUND PIPING NOT IN CONFLICT WITH NEW WORK MAY BE LEFT IN PLACE. DO NOT LEAVE PIPING ABANDONED UNDER THIS CONTRACT LIVE. SEE SPECIFICATION SECTION 02050 FOR DEMOLITION PROCEDURES. SEE SECTION 01550 FOR PLANT OPERATIONS DURING CONSTRUCTION FOR COORDINATION OF DEMOLITION WORK AND NEW CONSTRUCTION.
- 5. ALL EXISTING PIPING MAY NOT BE SHOWN. THE CONTRACTOR SHALL REFERENCE EXISTING RECORD DRAWINGS ON FILE WITH THE OWNER AND WESSLER ENGINEERING FOR ADDITIONAL INFORMATION OF EXISTING PIPING AND CONDUIT THROUGHOUT THE PLANT SITE.
- 6. NEW PIPING CARRYING LIQUIDS SHALL HAVE MINIMUM COVER AS DEFINED IN THE MISCELLANEOUS SITE DETAILS, UNLESS SPECIFIC ELEVATIONS ON THE DRAWINGS INDICATE OTHERWISE.
- 7. ALL EQUIPMENT TO BE REMOVED THAT HAS ELECTRICAL COMPONENTS, CONDUIT AND WIRING, AND/OR SMALL PIPING CONNECTED TO IT SHALL HAVE THE ELECTRICAL COMPONENTS AND SMALL PIPING REMOVED BACK TO THE SOURCE.
- 8. THE CONTRACTOR SHALL BE RESPONSIBLE FOR BRACING AND/OR PROTECTING ALL UTILITY POLES AND EXISTING STRUCTURES ADJACENT TO NEW EXCAVATIONS. UTILITY POLE BRACING SHALL BE AS DIRECTED BY THE GOVERNING UTILITY COMPANIES.
- 9. THE CONTRACTOR IS RESPONSIBLE FOR REPAIRING, RESTORING, OR REPLACING ALL PAVEMENT IN STREETS AND DRIVES OUTSIDE AND WITHIN THE CONSTRUCTION LIMITS WHICH IS DAMAGED BY CONSTRUCTION ACTIVITIES AND TRAFFIC, AND SHALL INCLUDE ALL SUCH COSTS IN HIS BID.
- 10. THE CONTRACTOR SHALL INSPECT THE SITE PRIOR TO BIDDING THE PROJECT TO SEE THE EXTENT OF THE DEMOLITION WORK INVOLVED AND TO INCLUDE THE NECESSARY WORK IN HIS BID.
- 11. THE CONTRACTOR SHALL BE RESPONSIBLE FOR CONTACTING EACH PERMIT ISSUING AGENCIES WITHIN THE TIME PERIOD SPECIFIED BY THAT AGENCY PRIOR TO BEGINNING CONSTRUCTION.
- 12. THE CONTRACTOR SHALL REFER TO SPECIFICATION SECTION 02101 FOR EROSION AND DUST CONTROL DURING CONSTRUCTION.
- 13. THIS PROJECT REQUIRES THE REMOVAL OF EQUIPMENT, SOME OF WHICH HAVE ASSOCIATED MERCURY FILLED COMPONENTS INCLUDING, BUT NOT LIMITED TO, GAUGES AND CONTROLS. IT SHALL BE THE CONTRACTOR'S RESPONSIBILITY TO REMOVE AND DISPOSE OF ALL MERCURY FILLED COMPONENTS DESIGNATED FOR REMOVAL WITHIN FEDERAL, STATE AND LOCAL REGULATIONS.
- 14. THE CONTRACTOR SHALL PROTECT ALL BUILDING SUMP PUMPS FROM CONTRUCTION DEBRIS THROUGHOUT THE COURSE OF CONSTRUCTION.
- 15. THE CONTRACTOR SHALL COMPLETELY CLEAN ALL FLOOR DRAINS AND DRAIN PIPING, IN EACH EXISTING BUILDING THAT HAS HAD CONSTRUCTION ACTIVITY, TO REMOVE ALL CONSTRUCTION DEBRIS. CONSTRUCTION DEBRIS IN DRAIN PIPING SHALL BE FLUSHED TO THE NEAREST SITE MANHOLE, THEN COMPLETELY REMOVED.

		11	ment Plant Design Summ	•			
	Warsaw Wastewater Tre	eatment Plant	NPDES Permit No. IN0060917				
Component	Description	Design Parameters	Component	Description	Design Parameters		
Influent Characteristics	Average Daily Flow (ADF)	6.00 MGD	Receiving Stream	Name	Tippecanoe River		
	Peak Hourly Flow (PHF)	18.00 MGD		Tributary to	Wabash River		
	CBOD5	220 mg/L		Stream Uses	Full Body Contact Recreation		
	TSS	220 mg/L		7-day, 1-in-10 year low flow	3.1 cfs		
	NH3-N	25.0 mg/L		100-Yr Flood Elevation	810.00-ft		
	Phosphorus	5.0 mg/L	Phosphorus Removal	Anaerobic Selector Tanks	Two (2) at 39.5-ft x 42-ft x 13.5-ft SWD		
			Biological		One (1) at 35-ft x 33-ft x 13.5-ft SWD		
Influent Screening	Mechanical Cylindrical	Two (2) @ 0.25-inch bar spacing:		Total Volume	283,000 gallans		
	Fine Screens (existing)	12.0 MGD peak capacity, each		Detention Time @ ADF	69 minutes		
	Manual Bypass Bar	1.75-inch O.C. bar spacing		Chemical	Alum		
Grit Removal	Aerated Grit	20.0 MGD with Air Lift Grit Transport		Design Dosage	5.5 mg/l		
		Two (2) @ 21-ft x 21-ft, 14-ft SWD		Peristaltic Feed Pumps	Thire (3) at 52 gph each		
Primary Clarifiers	Number and Size	Two (2) at 85-ft diameter, 12-ft SWD	Chemical	Storage Tanks	Tyo (2) at 4,050 gallons each		
	Surface Area	5,675 sq.ft. per clarifier		Secondary Containment	11,380 gallons		
	Surface Overflow Rate	529 gpd/sf @ ADF		Application Points	Grit Effluent Box &		
		1,586 gpd/sf @ PHF			Oxidation Ditch #1 & #2 Effluent Boxes		
Oxidation Ditch	Volume	Two (2) @ 1,460,000 gallons each	Anaerobic Digestion	Number and Size	Two (2) at 60-ft dia. x 21-ft SWD		
(existing)	Detention Time	11.7 hours at ADF		Volume	472,000 gallons each		
		3.89 Hours at PDF		Organic oading	62.2 lb VS/day/1000 cu.ft.		
	Organic Loading	18.3 lb BOD/1,000 cu.ft.		Detention Time	26.4 days		
	Solids Retention Time	11.4 days	Sludge Holding Tanks	WAS	45-ft x 22.5-ft SWD, 267,670 Gal		
	Aeration Blowers	Two (2) at 4,600 scfm each (Turbo)		Lige ted Sludge	45-ft x 22.5-ft SWD, 267,670 Gal		
		One (1) at 1,400 scfm (Centrifugal)		Septage	45-ft x 22.5-ft SWD, 267,670 Gal		
Sludge Pump Station #1	RAS Pumps	Two (2) at 2,800 gpm each	Gravity Belt Thickener (existing	Number and Capacity	One (1): 470 gpm, 930 lb/hr		
(existing)	WAS Pumps	Two (2) at 370 gpm each	Volute Press Dewatering	Number and Capacity	One (1): 260 gpm, 2,650 lb/hr		
	Scum Pump	One (1) at 150 gpm	Stand-by Generator (existing)	Number and Size	One (1) 900-kW		
Sludge Pump Station #2	RAS Pumps	Two (2) at 2,800 gpm each	Flow Meters	Raw Sewage:			
	Scum Pump	One (1) at 150 gpm		- Center Street LS	20" MagMeter		
Final Clarifiers (2	Number and Size	Three (3) at 90-ft diameter, 14-ft SWD		- Anchorage FM	18" MagMeter		
existing)	Surface Area	6,362 sq.ft. per clarifier	J - O	- Danek FM	10" MagMeter		
	Surface Overflow Rate	314 gpd/sq.ft. @ ADF		Effluent	2.0' Parshall Flume		
		943 gpd/sq.ft. @ PHF		Primary Sludge	6" MagMeter		
Disinfection	Туре	Ultraviolet	10	RAS	Two (2) 12" MagMeters		
	Channel Size	23.00-ft x 2.00-ft x 5.42-ft max W\$E		WAS	4" MagMeter		
	UV Transmittance	65% UVT	<b>Y</b>	Digester Sludge	Two (2) 4" MagMeters		
	Level Control	Serpentine weir		Digested Sludge	6" MagMeter		
Post-Aeration (existing)	Post-Aeration Basins	Four (4) 5-Hp Surface Aera ors		Gravity Belt Thickener Feed	6" Mag Meter		
	Post-Aeration Cascade	4-step cascade		TWAS	4" MagMeter		
				Volute Press Feed	4" Mag Meter		

WBJ

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APPROVED BY

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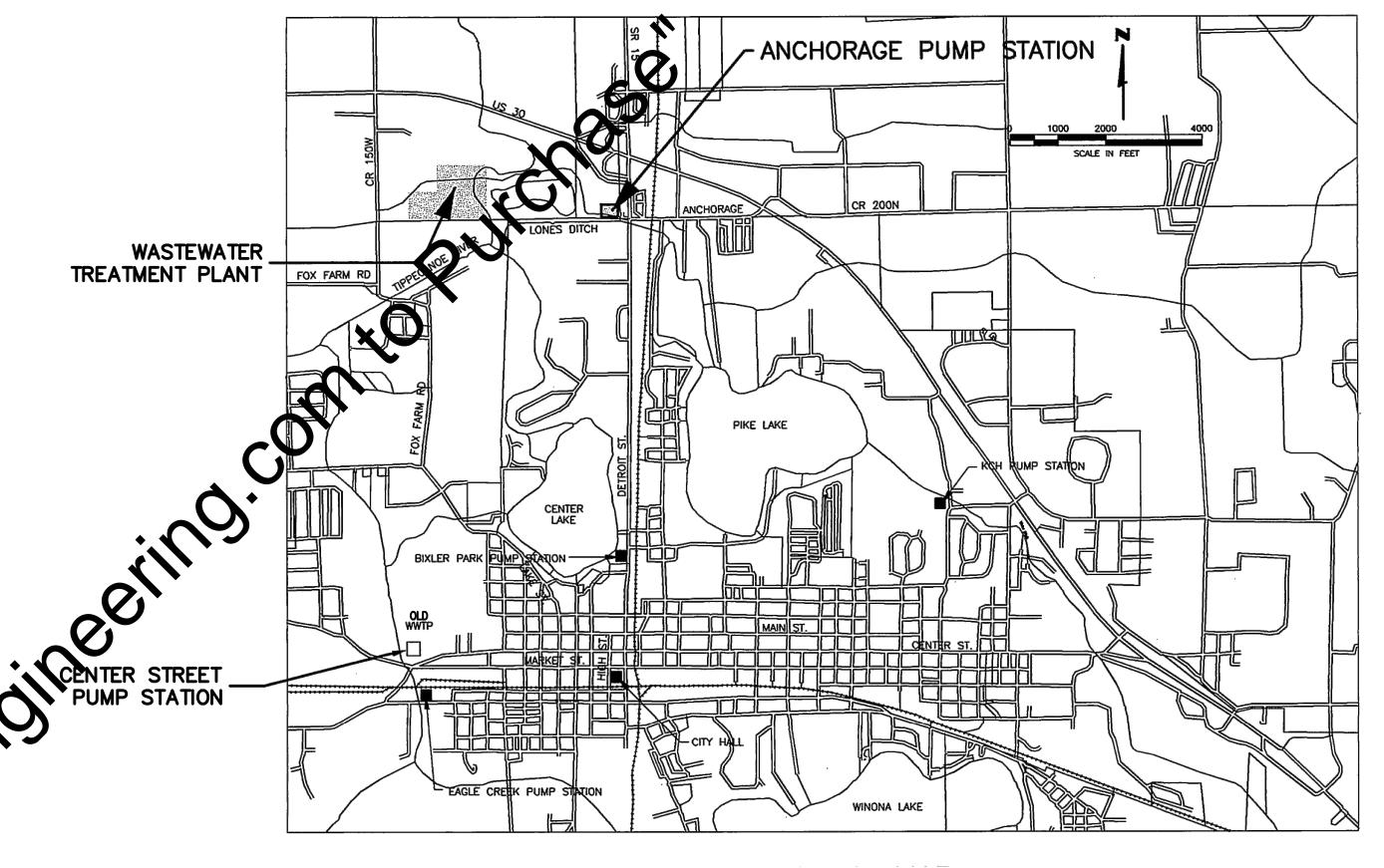
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## PROJECT LOCATION PLAN

### UTILITY CONTACTS

#### **ELECTRIC**

KOSCIUSKO REMC 370 SOUTH 250 EAST WARSAW, IN 46582 800-790-7362 ATTN: RYAN MILLER

# **ELECTRIC**

NIPSCO 801 E. 86TH AVE. MERRILVILLE, IN 46410 219-647-5036 ATTN: ROCKY YBARRA

#### <u>WATER</u>

<u>GAS</u> NIPSCO 801 E. 86TH AVE. MERRILVILLE, IN 46410 219-647-5311 317-885-2447 ATTN: DOUG BENDA ATTN: EZAT NAYEN

#### FIBER OPTIC/TELEPHONE

CENTURY LINK 213 W. LAPORTE ST PLYMOUTH, IN 46563 574-935-1247 ATTN: BRUCE EMERICK

INDIANA-AMERICAN WATER COMPANY, INC 555 E. COUNTY LINE RD., STE 201 GREENWOOD, IN 46143

IN THE EVENT OF WATER CONFLICT, CONTACT

EZAT NAYEN AT INDIANA-AMERICAN WATER

COMPANY TO COORDINATE WORK

#### WARSAW ENGINEERING

WARSAW UTILITIES

2056 N. 150 W

574-372-9562

WARSAW, IN 46580

ATTN: BRIAN DAVISON

102 S. BUFFALO ST. WARSAW, IN 46580 574-372-9548 ATTN: JAMES EMANS



# **WASTEWATER TREATMENT PLANT EXPANSION - 2017**

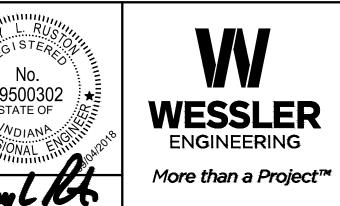
CITY OF WARSAW, INDIANA

PROJECT LOCATION MAP, GENERAL NOTES AND **WASTEWATER TREATMENT PLANT DESIGN SUMMARY**  SHEET NO.

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ABBREVIATION	DESCRIPTION	ABBREVIATION	DESCRIPTION	ABBREVIATION	DESCRIPTION
A/E	ARCHITECT/ENGINEER	EPA	US ENVIRONMENTAL PROTECTION AGENCY	NSF	NATIONAL SANITATION FOUNDATION
ABAN	ABANDON AGGREGATE BASE COURSE		EQUIPMENT ELAPSED TIME METER	NTS OC	NOT TO SCALE ON CENTER
ABC ACI	AMERICAN CONCRETE INSTITUTE	ETM	EACH WAY	OD	OUTSIDE DIAMETER
ADA	AMERICANS WITH DISABILITIES ACT	EWC	ELECTRIC WATER COOLER	OSHA	OCCUPATIONAL SAFETY AND HEALTH ADMINISTRATION
ADC	AIR DIFFUSION COUNCIL	EX	EXISTING	P&ID	PROCESS AND INSTRUMENTATION DIAGRAM
ADDL ADD <b>M</b>	ADDITIONAL ADDENDUM	EX GR EXH FN	EXISTING GRADE  EXHAUST FAN	PB PC	PUSHBUTTON  BEGINNIAG DE CURVE (POINT OF CURVATURE)
ADJ	ADJACENT / ADJOINING / ADJUSTABLE	EXP	EXPANSION	PCI	BEGINN AS OF CURVE (POINT OF CURVATURE) PRECAL (PFESTRESSED CONCRETE INSTITUTE
ADMIN	ADMINISTRATION	FD	FLOOR DRAIN	PDC	PC NET DISTRIBUTION CABINET
AGGR	AGGREGATE	FE	FIRE EXTINGUISHER	PDIPFI	UMBING AND DRAINAGE INSTITUTE  P FABRICATORS INSTITUTE
AHU AIA	AIR HANDLING UNIT  AMERICAN INSTITUTE OF ARCHITECTS	FE FEL	FLOW SENSOR FINISH FLOOR ELEVATION	PI	MIGENT/TANGENT INTERSECTION (POINT OF INTERSECT
AISI	AMERICAN INSTITUTE OF ARCHITECTS  AMERICAN IRON AND STEEL INSTITUTE	FHWA	FEDERAL HIGHWAY ADMINISTRATION	P	PROPERTY LINE
ALM	ALARM	FIN GR	FINISH GRADE	PNL	PANEL
ALNMT	ALIGNMENT	FM	FORCE MAIN	POLN	POLYETHYLENE POINT ON TANGENT
ALT ALUM.	ALTERNATE ALUMINUM	FMC FND	FLEXIBLE METALLIC CONDUIT FOUND		POWER PANEL
AMT	AMOUNT	FPT	FEMALE PIPE THREAD	PPU	POSITIVE PRESSURE UNIT
ANSI	AMERICAN NATIONAL STANDARDS INSTITUTE	FRP	FIBERGLASS REINFORCED PLASTIC	PRI	PRIMARY POUNDS PER SQUARE INCH
APA	ENGINEERED WOOD ASSOCIATION, THE	FS ET	FEDERAL SPECIFICATIONS FOOT/FEET	PT	CURVE/TANGENT INTERSECTION (POINT OF TANGENCY)
APP APPROX	APPARENT APPROXIMATE/APPROXIMATELY	FTG	FOOTING	PT	POINT
ARI	AMERICAN REFRIGERATION INSTITUTE	FUT	FUTURE	PV	PLUG VALVE
ASHRAE	AMERICAN SOCIETY OF HEATING, REFRIGERATING, AND AIR	GA	GAGE/GUAGE	PVC QTY	POLYVINYL CHLORIDE  QUANTITY
ASLA	CONDITIONING ENGINEERS  AMERICAN SOCIETY OF LANDSCAPE ARCHITECTS	GAL GALV	GALLON GALVANIZED	R	RADIUS
ASME	AMERICAN SOCIETY OF LANDSCAPE ARCHITECTS  AMERICAN SOCIETY OF MECHANICAL ENGINEERS	GPD	GALLONS PER DAY	RAS	RETURN ACTIVATED SLUDGE
ASPH	ASPHALT	GPM	GALLONS PER MINUTE	RCP	REINFORCED CONCRETE PIPE
ASSOC	ASSOCIATES	GPS	GLOBAL POSITIONING SYSTEM	RD RDT	ROAD ROTATING DRUM THICKENER
ASTM ATS	AMERICAN SOCIETY FOR TESTING AND MATERIALS AUTOMATIC TRANSFER SWITCH	GRS GV	GALVANIZED RIGID STEEL  GATE VALVE	RECIRC	RECIRCULATION
AVE	AVENUE	HDPE	HIGH DENSITY POLYETHYLENE	RECPT	RECEPTACLE[S]
AVG	AVERAGE	HH	HANDHOLE OR HAND HOLE	RED	REDUCER RIGID GALVANIZED STEEL
AWS	AMERICAN WELDING SOCIETY	HID HMA	HIGH INTENSITY DISCHARGE HOT MIX ASPHALT	RO	ROUGH OPENING
AWWA BF	AMERICAN WATER WORKS ASSOCIATION BLIND FLANGE	HOA	HAND-OFF-AUTOMATIV	ROW	RIGHT-OF-WAY
BFV	BUTTERFLYVALVE	HOR	HAND-OFF-RENOT HORIZONTAL	<u>s</u>	SECOND
ВНМА	BUILDER'S HARDWARE MANUFACTURER'S ASSOCIATION	HORIZ		SAE	SOUTH SOCIETY OF AUTOMOTIVE ENGINEERS
BIA BLDG	BRICK INDUSTRY ASSOCIATION BUILDING	HP HPS	HORSEPO VEI HIGH PRESSURE SODIUM (LTG)	SB	SOIL BORING
BLVD	BOULEVARD	HPU	HYD J. VLIC POWER UNIT	SCHED	SCHEDULE
BM	BENCHMARK	HVAC	HEATING YENTILATING, AND AIR CONDITIONING	SDR	STANDARD DIMENSION RATIO
BRG	BEARING	ID ID	INS DEPLOMETER	SEC SECT	SECONDARY SECTION
BTWN BV	BALL VALVE	IDEM F	NDIANA DEPARTMENT OF ENVIRONMENTAL MANAGEMENT INVERT ELEVATION	SF	SQUARE FOOT (FEET)
C	CONDUIT	IN (C	INCH	SF	SUPPLY FAN
CABO	COUNCIL OF AMERICAN BUILDING OFFICIALS	INC INCO	INCORPORATED	SHT SLDG	SHEET SLUDGE
CB	CATCH BASIN		INDIANA DEPARTMENT OF TRANSPORTATION	SPEC	SPECIFICATION(S)
CCW CF	COUNTERCLOCKWISE  CUBIC FEET		INFLUENT INDIANA STATE PLUMBING CODE	SQ	SQUARE
CHKV	CHECK VALVE	INTR	INSTRUMENT	SR	STATE ROAD/STATE ROUTE
CI	CAST IRON	SPC	INDIANA STATE PLANE COORDINATE	SRF SSPC	STATE REVOLVING FUND SOCIETY FOR PROTECTIVE COATINGS, THE
CI	CURB INLET	KFV LB	KNIFE GATE VALVE POUND(S)	SST	STAINLESS STEEL
CIP CISP	CAST-IN-PLACE CAST IRON SOIL PIPE	LCP	LOCAL CONTROL PANEL	ST	STREET
CISPI	CAST IRON SOIL PIPE INSTITUTE	LE	LEVEL SENSOR	STA STD	STATION STANDARD
CMA	COLD MIX ASPHALT	LF	LINEAR FEET (FOOT)	STI	STEEL TANK INSTITUTE
CMP CMU	CORRUGATED METAL PIPE  CONCRETE MASONRY UNIT	LIT LN	LEVEL INDICATING TRANSMITTER  LANE	STR	STARTER
CNR	CONCRETE MASONRY UNIT CORNER	LOR	LOCAL_OFF_REMOTE	SVA	SERVICE VALVE ASSEMBLY
CO	CLEANOUT	LP	LIGHTING PANEL	SW SW	STORMWATER SWITCH
CO	COMPANY	LS	LIFT STATION	SWBD	SWITCHBOARD
COAX CONC	COAXIAL CABLE CONCRETE	LTG MA EX	LIGHTING MATCH EXISTING	SWCD	SOIL AND WATER CONSERVATION DISTRICT
CONT	CONTINUOUS	MAG	MAGNETIC	SWGR	SWITCHGEAR
CONTR	CONTRACTOR	MATL	MATERIAL	SWPPP SYD	STORMWATER POLLUTION PREVENTION PLAN SQUARE YARD(S)
CP	CONTROL POINT	MAU MAX	MAKEUP AIR UNIT MAXIMUM	TB	TERMINAL BLOCK
CPP CR STN	CORRUGATED PLAST C PINE CRUSHED STONE	MB	MAIL BOX	ТВМ	TEMPORARY BENCHMARK
CU	COPPER	мсс	MOTOR CONTROL CENTER	TC TYP	TOP OF CASTING TYPICAL
CW	CITY WATER	MFR	MANUFACTURER  MILLION CALLONS REP DAY	UH	UNIT HEATER
CYD	CUBIC YARD CENTER LINE	MGD MH	MILLION GALLONS PER DAY  MANHOLE	UL	UNDERWRITER'S LABORATORIES, INC.
D m		MIL	MILITARY SPECIFICATIONS	UNO	UNLESS NOTED OTHERWISE
DBL	DE 11P	MIL STD	MILITARY STANDARD	USACE USBR	U.S. ARMY CORPS OF ENGINEERS U.S. BUREAU OF RECLAMATION
DHI	R AND HARDWARE INSTITUTE	MIN	MINIMUM MISCELLANEOUS	USGS	U.S. GEOLOGICAL SURVEY
DI MJ	DUCTILE IRON DUCTILE IRON MECHANICAL JOINT	MISC MJ	MISCELLANEOUS  MECHANICAL JOINT	UV	ULTRAVIOLET
DIA	DIAMETER	MNTR	MONITOR	V	VOLT(S)
DIP 1	DUCTILE IRON PIPE	MO	MASONRY OPENING	VERT VFD	VERTICAL VARIABLE FREQUENCY DRIVE
DIPC	DUCTILE IRON PIPE SIZE	MPT MS4	MALE PIPE THREAD  MUNICIPAL SEPARATE STORM SEWER SYSTEM	VLV	VARIABLE FREQUENCY DRIVE  VALVE
D. C.	DISCONNECT DEPARTMENT OF NATURAL RESOURCES	MSS	MANUFACTURER'S STANDARDIZATION SOCIETY	VSD	VARIABLE SPEED DRIVE
D.O.	DISSOLVED OXYGEN	N	NORTHING, NORTH	W W	WATT WEST
DOH	DEPARTMENT OF HEALTH	NA NC	NOT APPLICABLE  NOPMALL VCLOSED		WEST
DOT	DEPARTMENT OF TRANSPORTATION	NC NEC	NORMALLY CLOSED  NATIONAL ELECTRICAL CODE (ANSI/NFPA-70)	WAS	WASTE ACTIVATED SLUDGE
DR D&UE	DRIVE DRAINAGE AND UTILITY EASEMENT	NEMA	NATIONAL ELECTRICAL CODE (ANSI/NFPA-70)  NATIONAL ELECTRICAL MANUFACTURERS ASSOCIATION	WEF	WATER ENVIRONMENT FEDERATION
DWG	DRAWING	NEUT	NEUTRAL	WH	WATER HEATER
E	EASTING, EAST	NFPA	NATIONAL CEODETIC SURVEY	WL	WATER LINE WATER METER
EA	EACHEACE	NGS NIST	NATIONAL GEODETIC SURVEY  NATIONAL INSTITUTE OF STANDARDS AND TECHNOLOGY	WP	WEATHERPROOF
<u>EF</u> EFFL	EACH FACE EFFLUENT	NO NO	NORMALLY OPEN	WSE	WATER SURFACE ELEVATION
EJ	EAST JORDAN IRON WORKS	NO.	NUMBER	WTR XFER	WATER TRANSFER
EL	ELEVATION	NPT NPW	NATIONAL PIPE THREAD  NON-POTABLE WATER	YR	YEAR
			IN ANTELLIBRIE VALET	•	-
EMER	EMERGENCY  BLE IS A LISTING OF TYPICAL ABBREVIATIONS AND MAY NOT INC	<u>L</u>			

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		PROJECT NUMBER						
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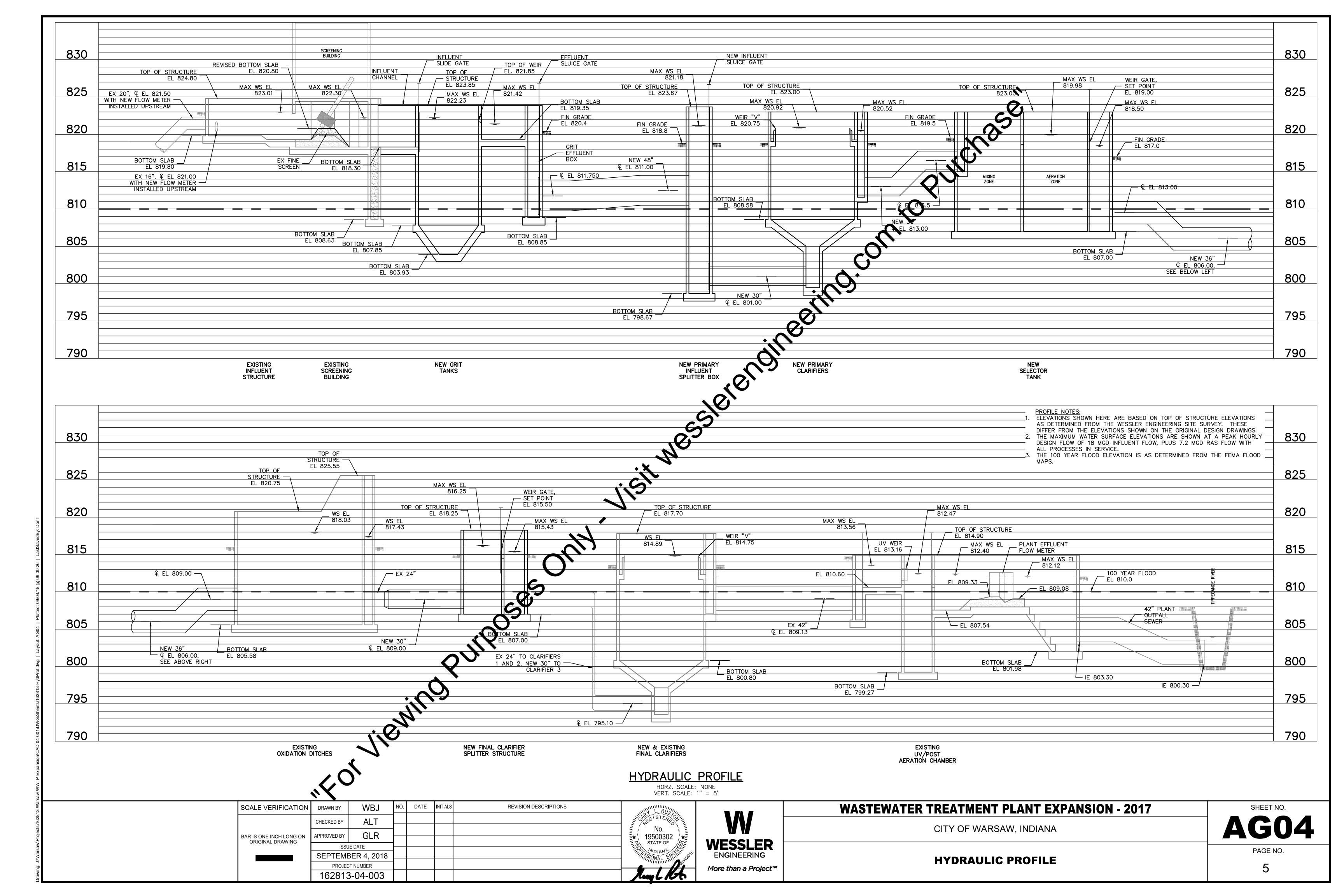


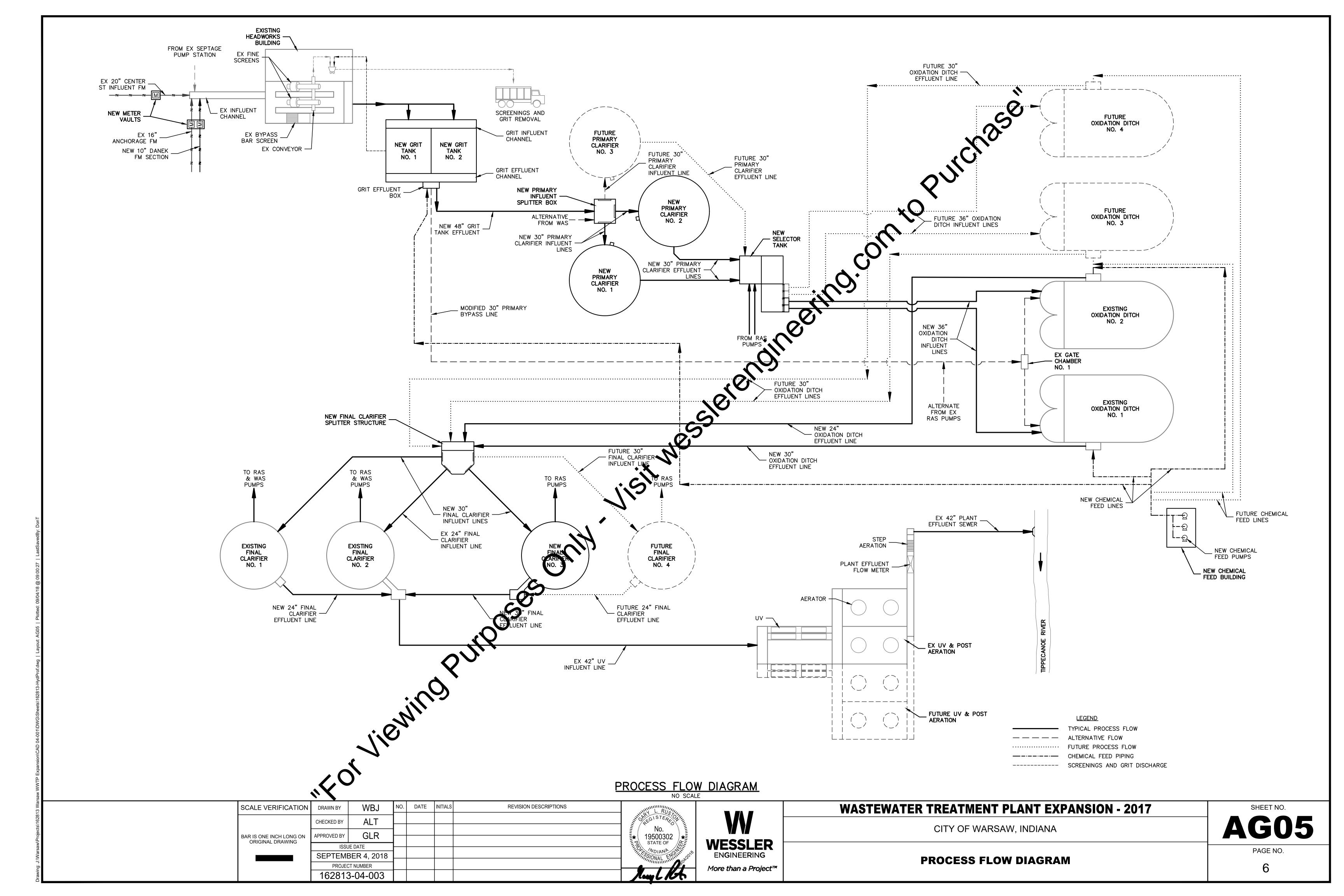
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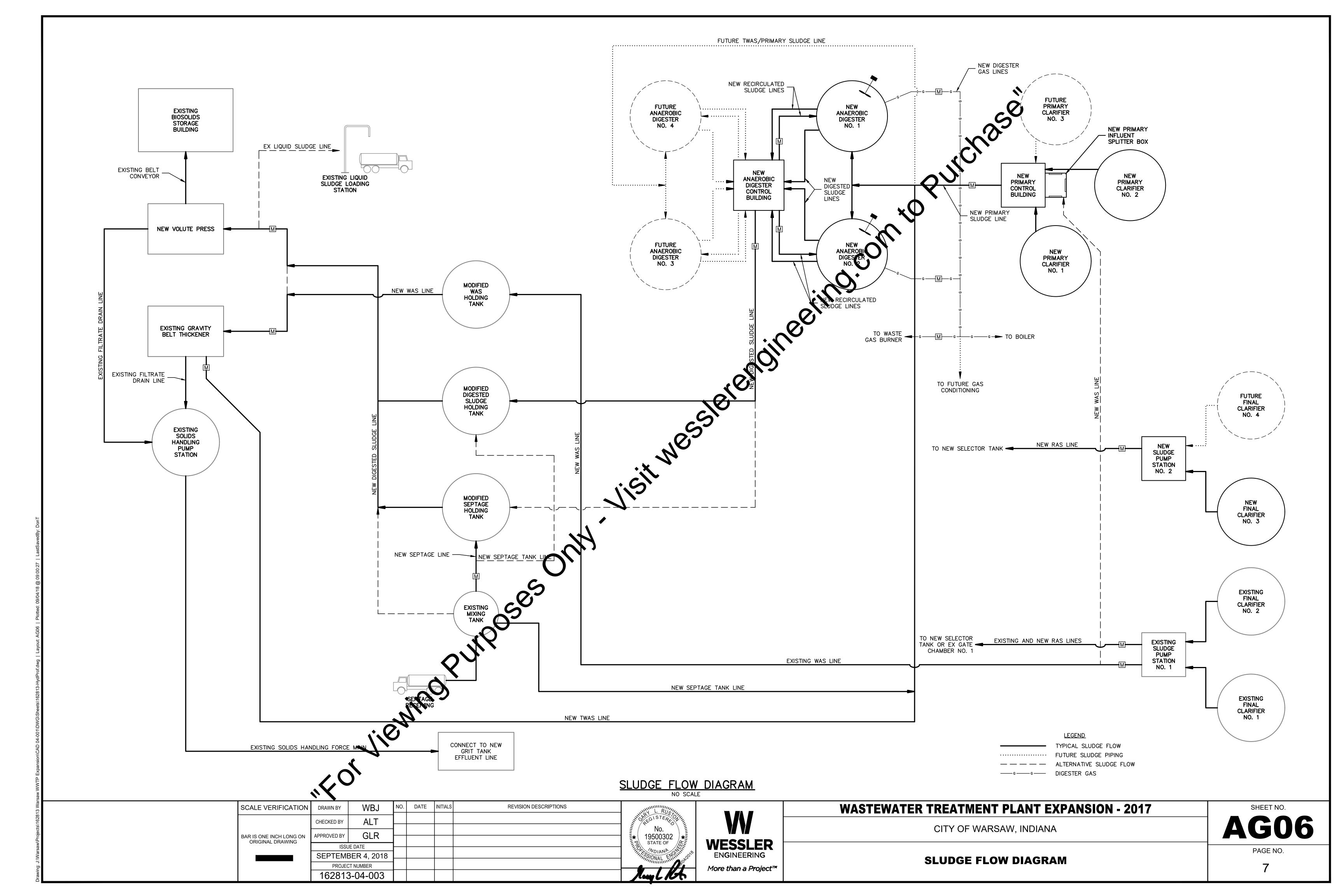
AGU3

TYPICAL ABBREVIATIONS

4







#### **GENERAL**

The structure has been designed for the in—service loads only. The methods, procedures, and sequences of construction are the responsibility of the Contractor. Supporting formwork for the concrete construction shall not be removed before the concrete has gained sufficient strength to safely support the dead and superimposed loads which will be subsequently applied. The Contractor shall take all necessary precautions to maintain and ensure the integrity of the structure at all stages of construction.

Refer to the architectural, civil, and process drawings for additional information.

All work shall be performed in accordance with the Indiana Building Code, 2014 Edition (2012 International Building Code, first printing, with Indiana Amendments).

Where new work is to be fitted to old work, the Contractor shall check all dimensions and conditions in the field, and report any errors or discrepancies to the Structural Engineer prior to the fabrication and erection of any new members. The Contractor shall be responsible for the correctness and fit of the new parts to the old parts.

Do not determine dimensions by "scaling" off the plans. The Contractor shall accept all risk associated with "scaling" and shall be responsible for all inadequate work resulting therefrom. Questions regarding missing or conflicting dimensions shall be directed, in writing, to the Structural Engineer.

Existing materials to be removed and reinstalled as part of this contract, but become damaged, shall be replaced with approved new material of equivalent quality and appearance at the Contractor's expense.

All work shall be performed without damage to adjacent retained work. Adequate protection of areas nearby work against dust, dirt and debris accumulation shall be maintained at all times.

Principal openings in the structure are indicated on the structural drawings. Refer to the architectural, mechanical, electrical, and plumbing drawings for sleeves, curbs, inserts, etc. not herein indicated. Openings in slabs with a maximum side dimension or diameter of 10 inches or less shall not require additional framing or reinforcement, unless noted otherwise. The location of sleeves or openings not shown in structural members shall be approved by the Structural Engineer.

The location of sleeves or openings not shown in structural members shall be approved by the Structural Engineer.

The Contractor shall relocate all mechanical piping, ducts, equipment, electrical conduits, wiring and plumbing that interfere with the proposed construction. Service shall be maintained to all equipment that is served by mechanical, electrical or plumbing conduit being relocated.

The shoring and/or re-shoring design is the responsibility of the Contractor. Temporary shoring for slabs, beams, and girders shall be adequate to carry the total weight of the slab-beam-girder system and any temporary construction loads to be imposed on the structural system. Shoring for a level shall not be removed until the concrete at that level has attained the specified 28 day compressive strength (f'c). Removal of shoring and/or reshoring shall not cause overstress in any structural element.

Opening dimensions shown on the plans and elevation views are nominal rough openings. It shall be the Contractor's responsibility to coordinate the specific clear opening dimension with the selected door manufacturer and door installer. Clear opening dimension shall account for any shimming and construction tolerances needed by the Contractor to complete their work. Refer to the Architectural plans for door locations and sizes.

#### **FOUNDATIONS**

Exterior footings shall bear 3'-0" minimum below finish grade and shall bear on undisturbed soil.

Foundation excavation and all other soils related work shall be performed in accordance with the geotechnical engineering report prepared by Earth Exploration, Inc. dated December 4, 2017 and all associated supplements.

Foundation and soils related work shall be performed under the direct supervision of a qualified Geotechnical Engineer.

Foundation excavations shall be made to plan elevations. The Contractor shall have a qualified Geotechnical Engineer verify that the allowable soil bearing pressure meets or exceeds that assumed for the foundation design. If the underlying soils are found to be unacceptable, one of the following procedures shall be followed:

Remove the unacceptable soil and backfill with an engineered structural fill as directed by the inspecting Geotechnical Engineer.

Lower the footing to an acceptable soil. Contact the Structural Engineer for potential modifications to the foundation system.

Subgrade structural elements subjected to differential lateral soil pressure shall be adequately braced until the structural elements which provide lateral restraint have been placed and allowed to cure for a minimum of 7 days, or until the concrete has achieved 75% of its specified compressive strength, whichever is more unless noted otherwise.

Excavations for spread footings, combined footings, continuous footings and/or mat foundations shall be cleaned and hand tamped to a uniform surface. Foundation excavations shall be adequately protected against detrimental change in condition from disturbance, rain, freezing, etc. Surface runoff shall not be allowed to enter the excavation.

Foundation conditions noted during construction, which differ from those described in the geotechnical report shall be reported to the Structural Engineer and Geotechnical Engineer before further construction is attempted.

Center all column and wall footings under the column or wall above unless otherwise indicated.

ORIGINAL DRAWING

**CONCRETE** 

Reinforced concrete has been designed in accordance with the latest edition[s] of the Building Code Requirements for Reinforced Concrete (ACI 318) and Environmental Engineering Concrete Structures (ACI 350) by the American Concrete Institute (ACI).

Slabs—on—grade shall be constructed in accordance with the latest edition of the Guide for Concrete Floor and Slab Construction (ACI 302.1R).

Mixing, transporting, and placing of concrete shall conform to the latest edition of the Standard Practice for Selecting Proportions for Normal, Heavyweight, and Mass Concrete (ACI 211.1) and the Standard Specifications for Structural Concrete (ACI 301). Concrete curing shall conform to the latest editions of the Standard Practice for Concrete Curing (ACI 308) and the Standard Specification for Curing Concrete (ACI 308.1). In case of a discrepancy, the plans and specifications shall govern.

Unless noted otherwise, concrete shall have natural sand fine aggregate and normal weight coarse aggregates conforming to ASTM C33, and Type I or III Portland Cement conforming to ASTM C150. The Contractor shall submit a mix design for each proposed class of concrete. Mix designs shall indicate proportions by weight, water—cement ratio, slump, air content, synthetic fiber size and quantity, sieve analyses of fine and coarse aggregates, standard deviation analysis, and required average strength and documentation of average strength verifying compliance with ACI 318. The Contractor shall not vary from the mix design without approval from the Structural Engineer.

Unless noted otherwise, fly ash may be used as a pozzolan to replace a portion of the Portland Cement in a concrete mix. Fly ash, when used, shall conform to ASTM C618, Type C. Concrete mixes using fly ash shall be proportioned to account for the properties of the specific fly ash used and to account for the specific properties of the fly ash concrete thus resulting. The ratio of the amount of the fly ash to the total amount of fly ash plus cement in the mix shall not exceed 25 percent.

Water—reducing admixtures conforming to ASTM C494 may be used in the concrete mix design. Maximum slump shall be 5 inches for mixes containing water—reducing admixtures and 5 to 8 inches for mixes containing high range water—reducing admixtures.

Concrete compressive strength tests shall be performed in accordance with ASTM C39. The tests shall be performed by an independent testing company at the Contractor's expense. Copies of the test results shall be forwarded to the Structural Engineer. One set of specimens shall be taken for each day's pour of appreciable size and for each 50 cubic yards in accordance with the latest edition of ASTM C31. Each set shall include one specimen tested at 7 days, 2 specimens tested at 28 days and one specimen retained in reserve. These test cylinders shall be laboratory cured.

When the ambient temperature is expected to fall below 40 degrees during the course of a concrete pour or subsequent curing period, it shall be placed and cured in accordance with the latest edition of Cold Weather Concreting (ACI 306R) and an additional set of concrete test cylinders shall be made. These cylinders shall be stored immediately adjacent to, and cured under the same conditions as the building concrete. Special curing boxes are not permitted for these test cylinders.

Concrete mixed, transported, placed, and cured under conditions of high ambient temperature, low humidity, solar radiation, or high winds shall conform to the latest edition of Hot Weather Concreting (ACI 305R) and an additional set of concrete test cylinders shall be made. These cylinders shall be stored immediately adjacent to, and cured under the same conditions as the building concrete. Special curing boxes are not permitted for these test cylinders.

Slump tests shall be made prior to and following the addition of plasticizers. Where concrete is placed by pumping methods, concrete for test cylinders and slump tests shall be taken at the point of final

Water shall not be added to the concrete at the job site. The Contractor is responsible for coordinating a pumpable and workable mix without the addition of water at the job site. The use of plasticizers, retardants and other additives shall be at the option of the Contractor subject to the approval of the Structural Engineer. Follow the recommendations of the manufacturer for the proper use of additives. Use of calcium chloride or other chloride bearing salts is prohibited.

Place concrete in a manner so as to prevent segregation of the minimal Delay floating and troweling operations until the concrete has lost surface water sheen or all free water. Do not sprinkle free cement on the lab surface. Finishing of slab surfaces shall conform to the latest editions ACI 302.1R and ACI 304R (Guide for Measuring, Mixing, Transporting and Placing Concrete).

Where an epoxy adhesive is specified for bonding plastic concrete to hardened concrete, it shall conform to the latest edition of the Standard Specification for Bonding Plastic Concrete to Hardened Concrete with a Multi-Component Epoxy Adhesive (ACI 503.2).

Maintain concrete in a moist condition for at least 5 days at ambient temperatures above 70 degrees, and at least 4 days at ambient temperatures above 50 degrees. Curing companies of moisture retention covers shall be used for all non-formed purities. Formed surfaces shall be cured by leaving forms in place. During ho, all weather, keep forms moist by sprinkling. When forms are removed prior to the end of the curing period, apply curing compound to the exposed surfaces. It shall be the Contractor's responsibility to provide a curing compound compatible with Contractor's responsibility to provide a curing compound compatible with other project requirements.

All interior slabs shall receive a flar "troweled finish". Exterior slabs, sidewalks, and stoops shall receive a "broom (or other type of slip resistant) finish". All flamed surfaces not exposed to public view shall receive a "rough form finish", exposed surfaces shall receive a "smooth form finish". Concrete finishes shall be as defined in ACI 301.

Protect finished concrete surfaces from damage, rain, hail, running water, other in interiors of fortions.

Protect the concrete surface between finishing operations on hot, dry days ime plastic shrinkage cracks could develop by using wet burlap, pembranes or fogging.

Construction joints at locations shall be submitted to the Structural Engineer for approval.

Construction joints shall be prepared by roughening the contact surface in an approved manner to a full amplitude of approximately 1/4 inch leaving the contact surface clean and free of laitance.

Control joints shall be made in concrete slabs—on—grade at major column centerlines, at points of discontinuity, at reentrant corners, and at other locations shown on the plans.

Provide 3/4 inch chamfers on all exposed corners of concrete except those abutting masonry.

The Contractor shall verify the location of sleeves, openings, embedded items, etc. and shall ensure that they are in place prior to the placement of the concrete.

Earth cuts shall not be used as forms ("bank forming") for vertical or sloping surfaces unless otherwise approved by the Structural Engineer Where bank forming is permitted, the concrete element shall be increased at least 3 inches on all sides exposed to earth to account for possible soil contamination during concrete placement.

#### CONCRETE SCHEDULE

CLASS	28 DAY COMPRESSIVE STRENGTH	AIR CONTENT	MIN. CEMENT LB/CY (SACKS/CY)	MAX. WATER/ CEMENT RATIO	CONCRETE LOCATION	REMARKS
Α	4,500 psi	6% <u>+</u> 1%	611 (65)	0.45	Foundation Walls, Spread Footings Wall Walls	
В	4,000 psi	≤ 3%	517 (5.5)	0.48	Interior Slabs on Grade	
С	4,500 psi	6% <u>+</u> 1%	611 (6.5)	0.42	Retaining Walls, Basement Walls, Tank Mat Foundations, Tank Structural Slabs, Tank Walls, Base Slabs, Slab floor systems	Crystalline Waterproofing Admixture
D	4,500 psi	6% <u>+</u> 1%	611 (6.5)	0.40	Exterior Slabs on Grade, Stoops, & Sidewalks	Synthetic Fibers (NS lbs/cyds)
Е	5,000 psi	6% <u>+</u> 1%	658 (7.0)	0.48	High Volume Grout	synthetic Fibers (1.5 lbs/cyds)

#### REINFORCING STEEL

Reinforcing bar detailing, fabricating, and placing shall conform to the latest edition of the following standards: Specifications for Structural Concrete for Buildings (ACI 301), ACI Detailing Manual (SP66). The latest editions of Concrete Reinforcing Steel institute's Reinforcing Bar Detailing and Placing Reinforcing Bars may also be used.

Provide standard bar chairs, slab bolsters, spacers, etc. as required to maintain concrete protection specified. Reinforcing steel shall be tied to prevent displacement during observe placement. Pulling up of welded wire fabric in slabs—on—grade and in metal deck is not permitted.

tall not be tack welded, welded, heated or cut unless otherwise indicated or poroved by the Structural Engineer.

Welding of reintercement bars, when approved by the Structural Engineer, shall conform to the latest edition of American Welding Society Standard D1.4. Electrodes for shop and field welding of reinforcement bars shall conform to TM A233, Class E90XX.

Concrete cover over reinforcement, unless otherwise noted, shall be as shecified in the latest edition[s] of ACI 318 and ACI 350 with the most stingent requirements governing.

Welled wire fabric in slabs—on—grade shall be placed 2" down from the top of the slab unless otherwise noted. Welded wire fabric in slabs on metal deck shall be placed anywhere from 3/4" to 1 1/4" down from the top of the slab unless otherwise noted.

Unless noted otherwise, splicing of reinforcing bars shall conform to the latest edition of ACI 318. Where the length of lap is not indicated, provide a Class "B" lap at tension splices or 30 bar diameter compression laps at compression splices.

CONCRETE REINFORCING STEEL LAP SPLICE SCHEDULE								
BAR SIZE	TENSION	COMPRESSION						
DAIN SIZE	TOP BAR	OTHER	SPLICE					
#3	21"	16"	12"					
#4	28"	24"	15"					
<b>#</b> 5	35"	<b>30</b> "	19" 23"					
<b>#</b> 6	42"	36"						
<b>#</b> 7	49"	42"	26"					
#8	56"	48"	30"					
#9	63"	57 <b>"</b>	34"					
#10	76"	66"	38"					
<i>#</i> 11	93"	72 <b>"</b>	42"					

Horizontal bars in walls, masonry bond beams, and continuous wall footings shall be bent at corners and intersections in such a way that continuity is provided through the joint. Separate corner bars of the same size and spacing as the horizontal reinforcing may be substituted for the bent portion of the continuous bars.

Unless noted otherwise, provide 2-#5 bars (one each face) around unframed openings and diagonally at reentrant corners of vertical height offsets in concrete walls. Place bars parallel to the sides of the opening and extend 24 inches beyond corners.

The Contractor shall prepare detailed working or shop drawings to enable him to fabricate, erect and construct all parts of the work in accordance with the drawings and specifications and shall submit one reproducible copy and one blue line copy to the Structural Engineer for review prior to fabrication. These shop drawings will be reviewed for design concepts only. The Contractor shall be responsible for all dimensions, accuracy, and fit of work.

#### **MASONRY**

Engineered concrete masonry has been designed in accordance with the latest edition of the ACI Building Code Requirements for Masonry Structures (ACI 530/ASCE 5).

Concrete masonry construction shall conform to the latest edition of the ACI Specifications for Masonry Structures (ACI 536.1) ASSE 6).

Structural masonry construction shall inspected v a qualified independent "special inspector" in accordance with Section 1704.5 of the 2006 International Building Code, third printing. If the absence of an Owner's special inspection program, costs of masons inspection shall be borne by the Contractor.

Mortar shall be type N for interior non-load bearing walls. For exterior and load bearing walls, mortar shall be type M below grade and type S above grade. Mortar shall conform to the requirements of the latest edition of ASTM C270. Portland Cerent-lime without air entrainment shall be used in the mortar mix.

Provide standard spaces, etc. as required to prevent reinforcing steel displacement du in grout placement.

Provide reinforcing steel in vertical cores as indicated. In addition, provide reinforcing steel in vertical cores on each side of all openings and each corner of all valls. Grout cores with reinforcing steel solid.

Reinforcand steel lap splices in concrete masonry shall be 60 bar diameters minum) unless otherwise noted. All splices shall be wired toaether.

Massiry cores (where specified) and bond beams shall be filled with coarse arout conforming to the requirements of the latest edition of ASTM C476  $^lacktriangle$  and having a minimum 28—day compressive strength of 3,000 psi, 3/4 inch maximum aggregate, and an 8 to 11 inch maximum slump.

Bearings for beams, lintels, joists, etc. shall be bond beams or hollow masonry units with cores filled solid with grout. The minimum bearing length shall be 8 inches unless otherwise indicated.

The Contractor shall prepare detailed working or shop drawings to enable him to fabricate, erect and construct all parts of the work in accordance with the drawings and specifications and shall submit one reproducible copy and one blue line copy to the Structural Engineer for review prior to fabrication. These shop drawings will be reviewed for design concepts only. The Contractor shall be responsible for all dimensions, accuracy, and fit of

MASONRY REINFORCING STEEL LAP SPLICE SCHEDULE										
f'm = 2,00	f'm = 2,000 psi									
BAR SIZE #3 #4 #5 #6 #7 #8										
8" CMU	1'-6"	2'-0"	2'-6"	4'-6"	5'-9"	8'-6"				

#### STRUCTURAL STEEL

Structural steel detailing, fabrication and erection shall conform to the latest editions of the AISC Specification for Structural Steel Buildings, Allowable Stress Design and Plastic Design and the AISC Code of Standard Practice for Steel Buildings and Bridges.

Erector shall maintain minimum temporary bracing at each bay in each direction until the roof diaphragm and permanent lateral load resisting system construction are complete.

Structural steel shall be shop-painted with a rust inhibiting primer. Steel which will be exposed to weather shall receive one additional finish coat. Steel that will be normally visible to the building's occupants or exposed to weather shall receive a field applied finish coat matching the existing surrounding surfaces. All abrasions caused by handling after shop painting shall be touched-up after erection is complete.

Design connections not shown in accordance with the latest AISC Specification and Manual of Steel Construction (allowable stress design method). Design simple span non-composite beam connections not shown to support one—half the beam load capacity as given in the AISC Uniform Load Constants for Beams Laterally Supported tables. Connection angles shall be double web angles, 5/16" minimum thickness.

Unless otherwise noted, bolted connections for structural steel members shall be bearing—type using 3/4" diameter ASTM A325 high strength bolts with standard 13/16" diameter holes tightened to the snug tight

High strength bolted connections shall conform to the latest edition of the Specification for Structural Joints Using ASTM A325 or A490 Bolts, approved by the Research Council on Structural Connections of the Engineering Foundation.

Welding procedures shall conform to the latest edition of the American Welding Society's (AWS) Structural Welding Codes for: Steel ANSI/AWS D1.1 and Sheet Steel ANSI/AWS D1.3.

Welded connections using ASTM A572 and A992 steel as a base metal shall be made with E70XX Low Hydrogen electrodes. Unless otherwise noted, other welded connections shall be made with regular E70XX electrodes. Welding shall be performed only where shown and to the extent indicated.

Field drilled holes shall be reamed, cleaned and deburred prior to assembly of the connection.

Thermal cutting shall preferably be done by machine. Hand thermally cut edges which will be subjected to substantial stress, or which are to have weld metal deposited on them, shall be reasonably free from notches or gouges. Notches or gouges greater than 3/16" that remain from cuttina shall be removed by grinding. Re-entrant corners shall be shaped notch-free to a radius of at least 1/2".

Paint on surfaces adjacent to joints to be field welded shall be wire brushed to reduce the paint film to a minimum.

Surfaces within 2" of any field weld shall be free of materials that would prevent proper welding or produce toxic fumes while welding is being

Splicing of structural steel members where not detailed is prohibited without the prior approval of the Structural Engineer as to location, type of splice and connection to be made.

Beams with specified camber shall be cambered upward. Beams without specified camber shall be fabricated so that after erection any minor camber due to rolling or shop assembly is upward.

Unless otherwise noted, provide a 5/8" thick steel bearing plate with two 1/2" diameter x 0'-6" stud anchors at the ends of steel beams bearing on concrete or masonry supports.

Continuous beam cantilevers and other beams framing over the top of a column shall have a full height 3/8" web stiffener on each side of the beam, centered above the supporting column. The beam—to—column connection shall contain no less than 4 bolts (2 each side of beam).

The Contractor shall prepare detailed working or shop drawings to enable him to fabricate, erect and construct all parts of the work in accordance with the drawings and specifications and shall submit one reproducible copy and one blue line copy to the Structural Engineer for review prior to fabrication. These shop drawings will be reviewed for design concepts only. The Contractor shall be responsible for all dimensions, accuracy, and fit of work.

#### POST-INSTALLED EXPANSION/ADHESIVE ANCHORS

Post—installed anchors shall be installed by qualified personnel in accordance with the drawings and specifications.

Post—installed anchors shall be installed by qualified personnel in accordance with the Manufacturer's Printed Installation Instructions (MPII), the drawings and specifications. Installation of adhesive anchors shall be performed by personnel trained to install adhesive anchors.

Post—installed anchors shall be as manufactured by Hilti Fastening Systems or approved equivalent.

Masonry cores receiving post-installed anchors shall be filled with course grout conforming to the requirements specified herein. Anchors shall not be installed in a masonry mortar joint.

The Contractor shall inspect the masonry or concrete surface at each proposed post-installed anchor location prior to installation. If the anchor locations align with mortar joints or the masonry or concrete is honeycombed, cracked or otherwise unsound, the anchors shall be repositioned so as to be located in sound material and be in accordance with the manufacturer's minimum spacing and edge distance requirements.

Adhesive anchors shall be subject to the following additional requirements:

Anchors shall meet the requirements of ACI 355.4.

Proof loading of adhesive anchors is not required.

Anchors shall not be installed in concrete cured less than 21-days

Anchors shall not be installed until the concrete has reached a minimum compressive strength of 2,500 psi.

Concrete temperature must be greater than 50 °F and less than 80 °F prior to installation of the anchors unless otherwise permitted by the

Anchors shall be installed in holes drilled with a rotary or rock drill. Follow the MPII for size and depth of holes required.

The acceptability of certification other than the ACI/CRSI Adhesive Anchor Installer Certification shall be the responsibility of the Structural Engineer. Adhesive anchors installed in horizontal or upwardly inclined orientations to resist sustained tension loads shall be continuously inspected during installation by an inspector specially approved for that purpose by the building official. The special inspector shall furnish a report to the licensed design professional and building official that the work covered by the report has been performed and that the materials used and the installation procedures used conform to the approved contract documents and MPII.

Installation of anchors shall be inspected in accordance with ACI 318-11, Section 1.3 and the Indiana Building Code.

### NON-SHRINK GROUT

Grout shall be a high early strength, non-metallic, shrinkage resistant (when tested in accordance with the latest edition of ASTM C827 or CRD-C621), premixed, non-corrosive, non-staining product conforming to the requirements of the latest edition of ASTM C1107 and containing Portland Cement, silica sands, shrinkage compensating agents and fluidity improving compounds.

Grout compressive strength tests shall be performed in accordance with the latest edition of ASTM C109, with a restraining plate placed over the

Grout shall be installed in accordance with the manufacturer's instructions.

Grout shall be pumped into place and shall have forms built around it for confinement.

l 10 Shoshone Drive 317.818.1912 Carmel, IN 46032 cesolutionsinc.com

SCALE VERIFICATION MAS DATE INITIALS **REVISION DESCRIPTIONS** DRAWN BY CEB **CHECKED BY** CEB BAR IS ONE INCH LONG ON APPROVED BY ISSUE DATE SEPTEMBER 4, 2018 PROJECT NUMBER 162813-04-003



**WASTEWATER TREATMENT PLANT EXPANSION - 2017** 

CITY OF WARSAW, INDIANA

SHEET NO. **AS01** 

**GENERAL STRUCTURAL NOTES PART 1** 

PAGE NO.

All notes hereafter are typically applicable unless otherwise noted on plans, sections or details.

STRUCTURAL LUMBER

Structural lumber shall be detailed, fabricated and erected in accordance with the latest editions of the Timber Construction Manual by the American Institute of Timber Construction (AITC) and the National Design Specification for Wood Construction by the American Forest & Paper Association (ANSI/NFoPA NDS).

Bolts, lag screws, nails and other wood fastenings, unless otherwise noted, shall conform to the latest edition of the National Design Specification for Wood Construction. Standard cut washers shall be used between the wood and bolt head and the wood and nut.

Wood construction connectors plates shall be as manufactured by Simpson Strong—Tie Company, Inc. or approved equivalent.

Except where epoxy injection is specified; bolted, lag screwed, or nailed wood member connections shall be glued using adhesives conforming to APA Specification AFG-01 (PL-400) in accordance with the manufacturer's recommendations.

Structural load bearing wall studs, not otherwise continuously braced on both sides by gypsum board, plywood/performance rated panel sheathing, hardboard panel siding, or other Indiana Building Code (IBC) approved sheathing; shall be braced at third points with horizontal solid wood blocking not less than 2 inches (nominal) in thickness and of the same width as the studs fitted snugly and nailed thereto to provide adequate lateral support.

Wood members that are in contact with concrete or masonry or exposed to weather shall be pressure treated with a water borne treatment to a net retention level of 0.3 pcf in accordance with applicable American Wood Preservers' Association latest requirements.

Rough sawn timbers shall be treated and finished where specified. Ends exposed to weather shall be treated with CCA.

#### PLYWOOD / PERFORMANCE RATED PANELS

Plywood and performance rated panels (oriented strand board) shared detailed, fabricated and erected in accordance with the latest citiering established by the American Plywood Association (APA) including the latest edition of the Plywood Design Specification (and its Supplements).

Plywood panels shall be identified with the appropriate trade gark of the APA and shall meet the requirements of the latest ection of the U.S. Product Standard PS 1 for Construction and Industrial Plywood. iate trademark Performance rated panels shall be identified with the of the APA and shall meet the requirements of the local edition of the APA PRP-108 Performance Standards and Prisie for Structural-Use Panels, or the U.S. Product Standard PS 2 for Vood-Based Structural-Use Panels.

Roof panels shall be installed with the line timension (face grain) across on the same continuous over the more supports (minimum 3) span condition).

Stagger panel end joints. End is shall only occur over a support. Unless recommended otherwise by the panel manufacturer, provide a 1/8" gap between panel ency and edges. Panel edges shall be tongue—and—groove or supported on 2" (nominal) lumber blocking installed between joists. Shear was anel edges shall be supported on 2" (nominal) lumber blocking installed between studs.

#### METAL-PLATE-CONNECTED WOOD TRUSSES

Prefabricated wood trusses shall be detailed, fabricated and erected in accordance with the latest editions of the Timber Construction Manual by the American Institute of Timber Construction (AITC) and the National Design Specification for Wood Construction by the American Forest & Paper Association (ANSI/NFoPA NDS) and the latest criteria established by the Truss Plate Institute (TPI) and the Wood Truss Council of America (WTCA).

Temporary and permanent bracing of wood trusses shall be in accordance with the latest edition of the Commentary and Recommendations for Handling, Installing and Bracing Metal Plate Connected Wood Trusses (HIB) by the TPI.

Wood roof trusses shall be designed to support the solowing superimposed loads in addition to the weight of the trusses:

Top Chord Dead Load Top Chord Live Load Bottom Chord Dead Load Bottom Chord Live Load Wind Load (horizontal) Wind Load (net vertical)

Deflection due to live load stall be limited to 1/360 of the truss span. For truss cantilevers, the deflection due to live load at the end of the cantilever shall be limited to 1/180 of the cantilever dimension.

Truss plates shall be valvanized steel and shall be applied to both faces of the members king connected.

conform to the geometry shown. Minimum lumber size for top Trusses and bottom anord members shall be 2"x 6" (nominal). Web member size and configuration shall be the option of the fabricator.

The was manufacturer shall prepare detailed working or shop drawings and hall submit one reproducible copy and one blue line copy, including coculations, to the Structural Engineer for review prior to fabrication. le drawings and calculations shall show the design forces in the truss members, the sizes of the truss plates; the lumber species, commercial grade and normal duration design values; required bracing and details necessary to enable the truss manufacturer to fabricate, erect and construct all parts of the work in accordance with the drawings and specifications. These shop drawings will be reviewed for design concepts only. The truss manufacturer shall be responsible for all dimensions, accuracy, and fit of work. The trusses shall be designed by, and the shop drawings and calculations shall bear the seal and signature of, a registered professional engineer in the State of Indiana.

The contractor shall install all permanent truss bracing as shown on the truss manufacturer's shop drawings.

#### COORDINATION WITH OTHER TRADES

The Contractor shall coordinate and check all dimensions relating to architectural finishes, structural framing, mechanical openings, equipment, etc. The Structural Engineer shall be notified of any discrepancies before proceeding with work in an area under guestion.

#### SPECIAL NOTES TO THE OWNER

Under normal conditions and for conventional buildings/structures such as the subject structure, reinforced concrete will develop cracks. The cracks are due to inherent shrinkage of the concrete, creep, ambient temperature variation, and restraining effects of vertical and other structural elements.

The cracks formed are normally cosmetic. The concrete maintains its serviceability and strength requirements. It is possible that a number of hairline cracks, which would normally spread over a wide area, will integrate into a single crack with a width exceeding 0.01 inch. It is emphasized that although special effort is made to reduce the potential causes and number of such cracks, it is not practical to provide total articulation and thereby achieve complete inhibition of all cracks.

The majority of these cracks develop within the first three years of service. Cracks which are wider than 0.01 inch may require sealing or epoxy

The object of the joints provided in the structure is to allow movement. Movements due to creep and shrinkage may be noticeable at joints up to two years after construction, beyond which movements due to variations in temperature will persist.

#### <u>DESIGN</u>

Building Code: Indiana Building Code, 2014 Edition (2012 International Building Code, with Indiana Amendments)

2,500 psf Allowable net bearing pressure: Unit weight of soil 125 pcf Effective Fluid Pressure 90 pcf Coefficient of friction between soil and concrete footing 0.30 (assumed) 175 pci Subgrade Modulus

#### Concrete:

Soil information:

28 day compressive strength (f'c) see schedule

#### Masonry:

Reinforcing steel (deformed bars of new billet steel):

## Stirrup and tie

ASTM A615, Grade 60 Welded wire fabric (smooth) ASTM A185

1,500 psi

L/360

25 psf with drift considerations

#### Plywood/Performance Rated Panels:

28 day compressive strength (f'm)

24/16 Span Rating 1/2" Thickness Exposure

#### Non-shrink grout:

28 day compressive strength 5,000 psi

## Live loads:

Roof

Exposure

Floors: 100 psf Slab on Grade

Tank Walkways 100 psf Live Load Deflection Limitation:

## Wind loads:

Basic wind speed (3-second gust) 120 mph Occupancy Risk Category

#### Seismic loads:

Occupancy Risk Category MCE Seismic Spectral Response Acceleration at Short Periods, Ss 10.2% g MCE Seismic Spectral Response Acceleration at 1 Second, Importance factor, IE 1.25 Site Class Seismic Design Category

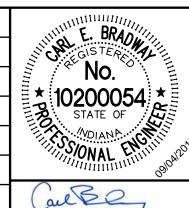
#### Structural Lumber (surfaced dry, used at 19% moisture content): Southern Pine, No. 2 (U.O.N) All members

ANSI/ASME B18.2 Bolts/Lag Screws FF-N-105B

#### Laminated Veneer Lumber (LVL) (as manufactured by Weyerhauser): Allowable Bending Stress 2,600 psi

285 psi Allowable Shear Stress Modulus of Elasticity (10<sup>6</sup> psi) 2.0E 0.50 Specific Gravity

MAS **SCALE VERIFICATION** DATE INITIALS REVISION DESCRIPTIONS DRAWN BY CEB CHECKED BY CEB APPROVED BY BAR IS ONE INCH LONG ON ORIGINAL DRAWING ISSUE DATE SEPTEMBER 4, 2018 PROJECT NUMBER 162813-04-003





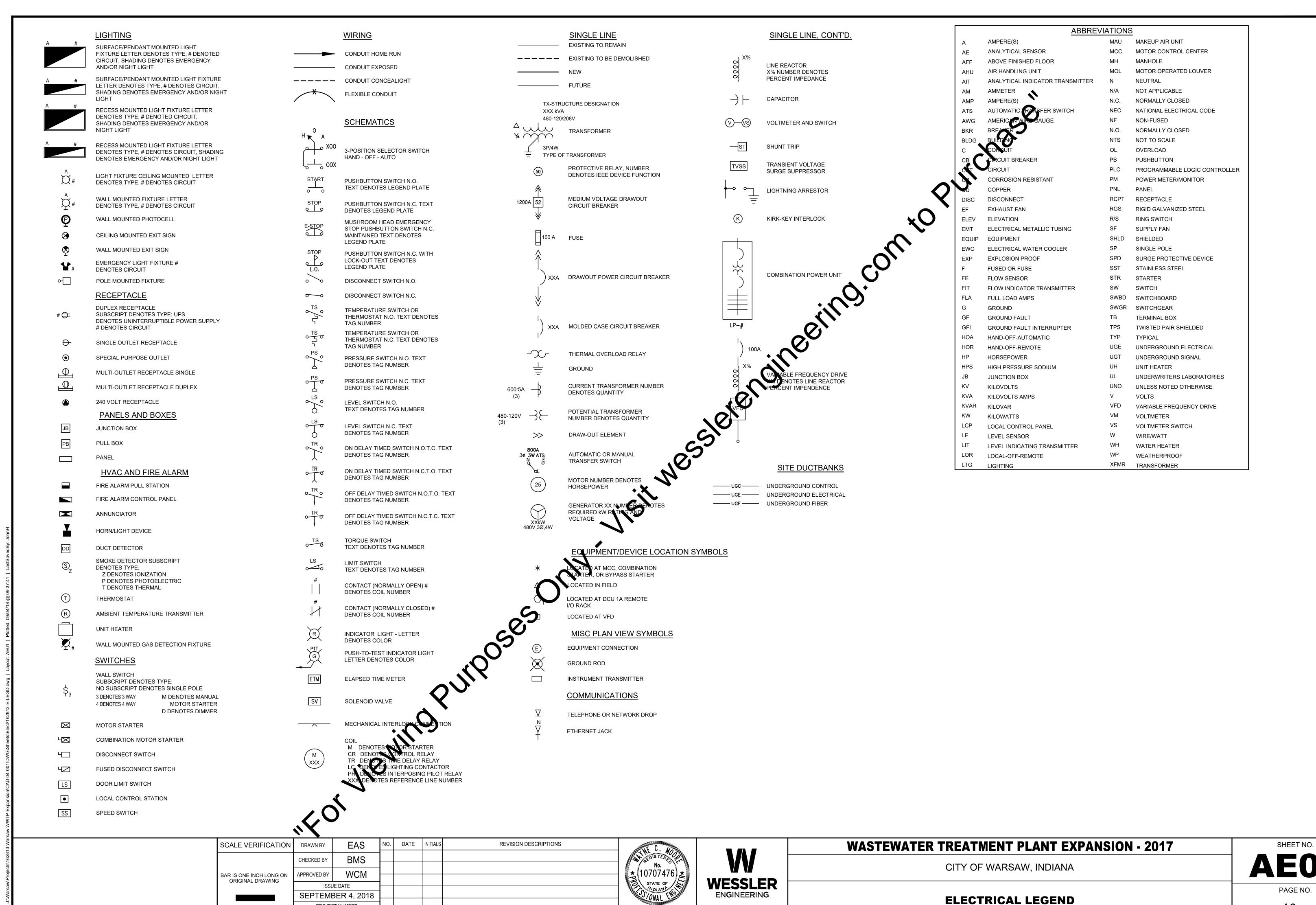
WASTEWATER TREATMENT PLANT EXPANSION - 2017

CITY OF WARSAW, INDIANA

SHEET NO.

PAGE NO.

GENERAL STRUCTURAL NOTES PART 2

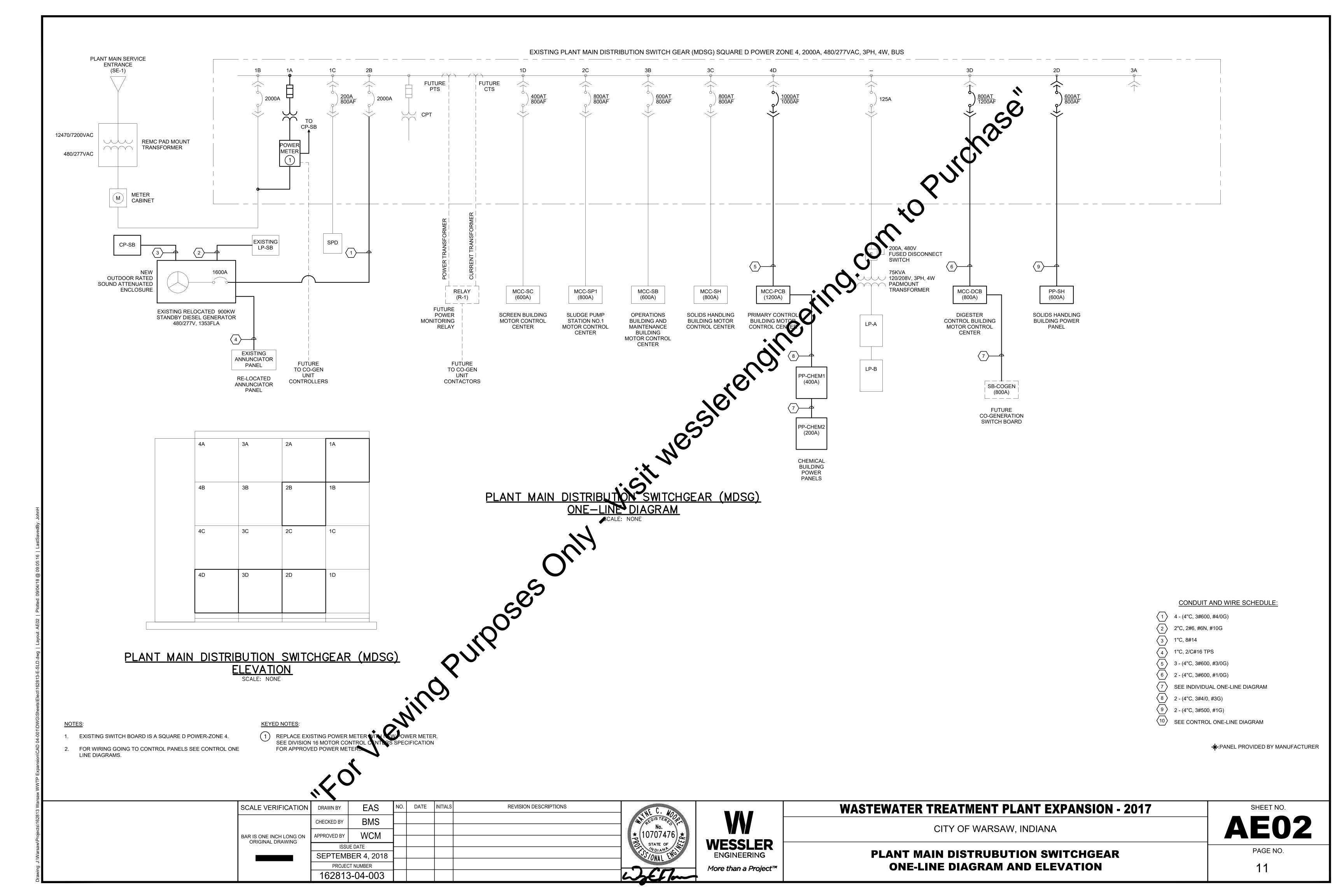


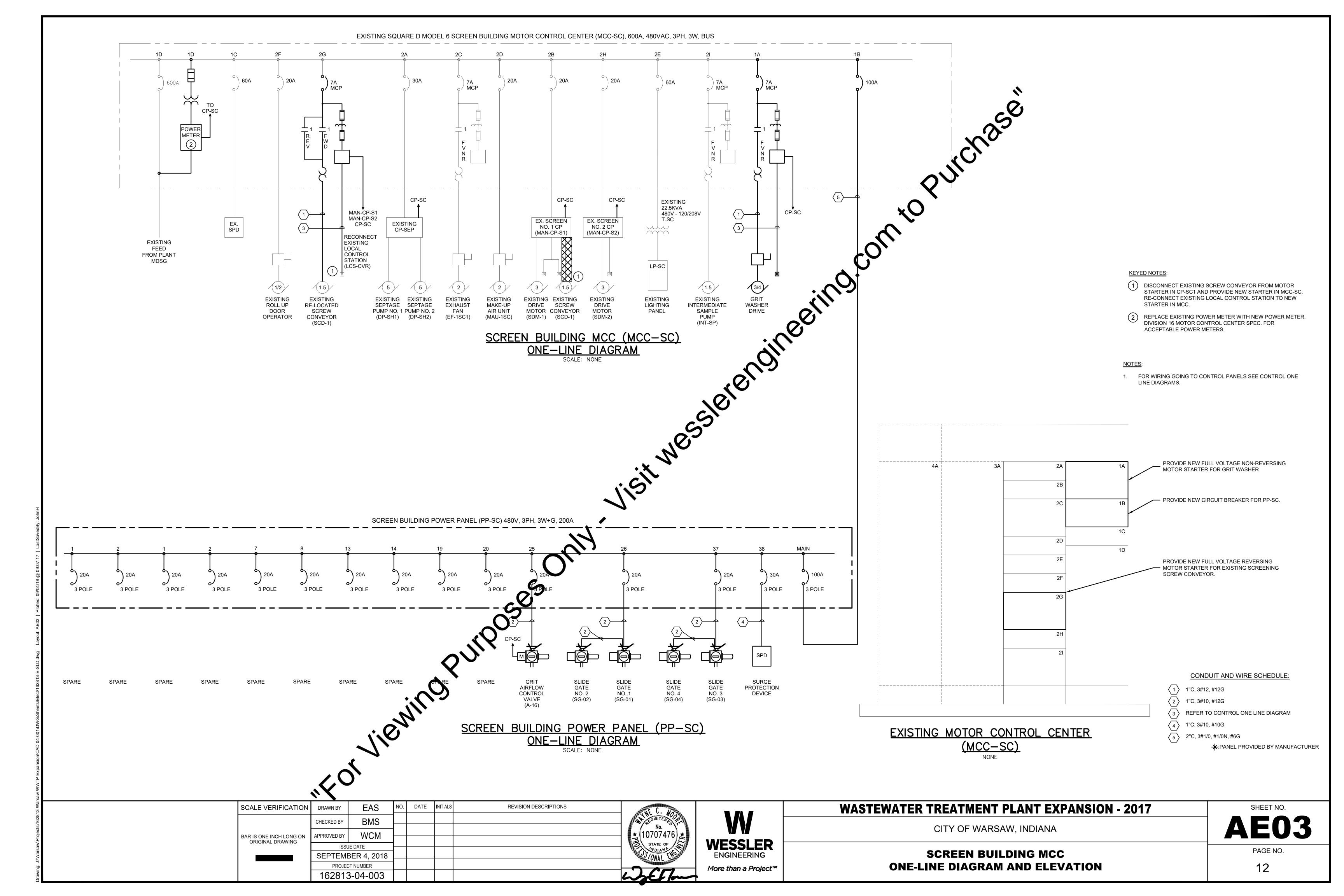
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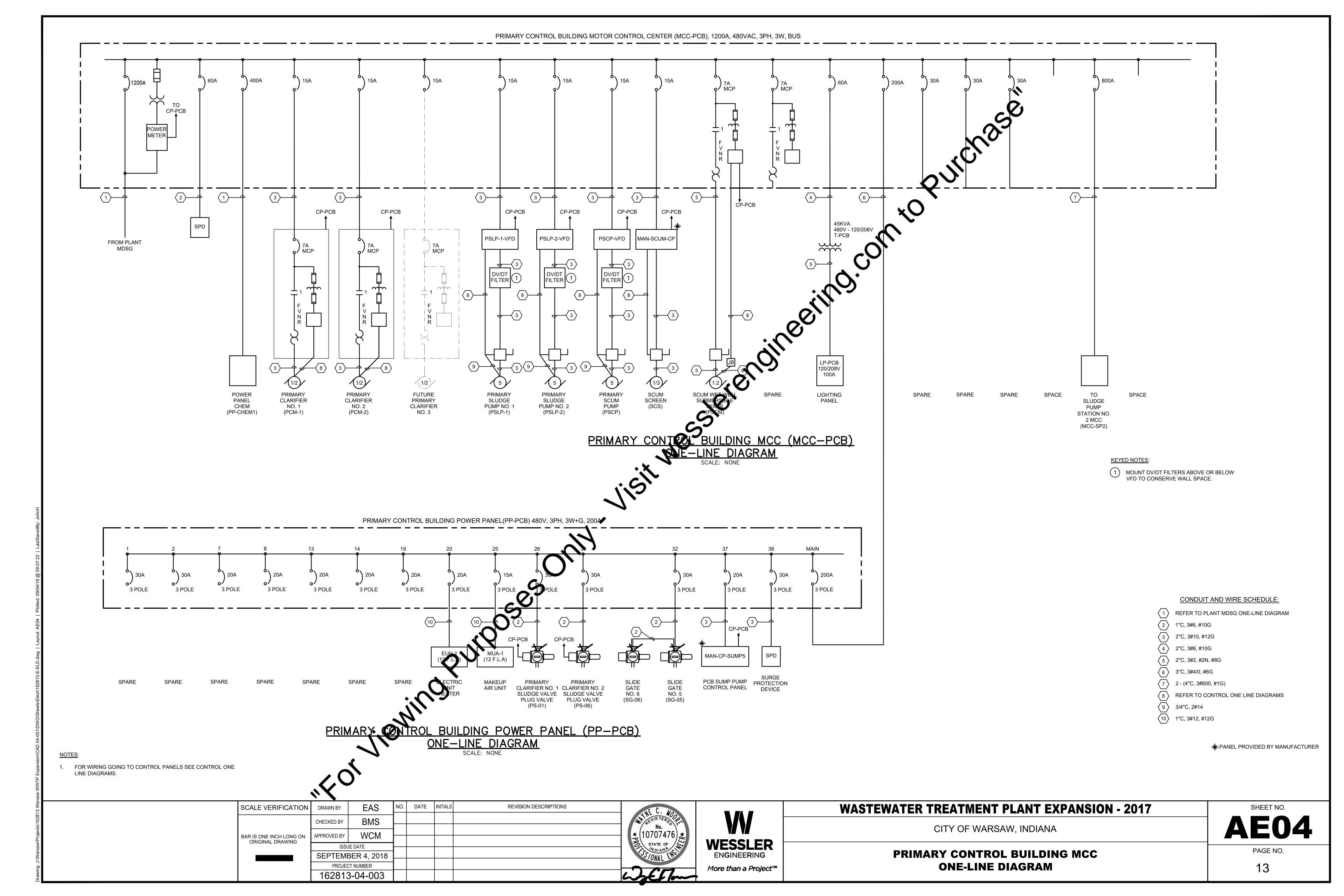
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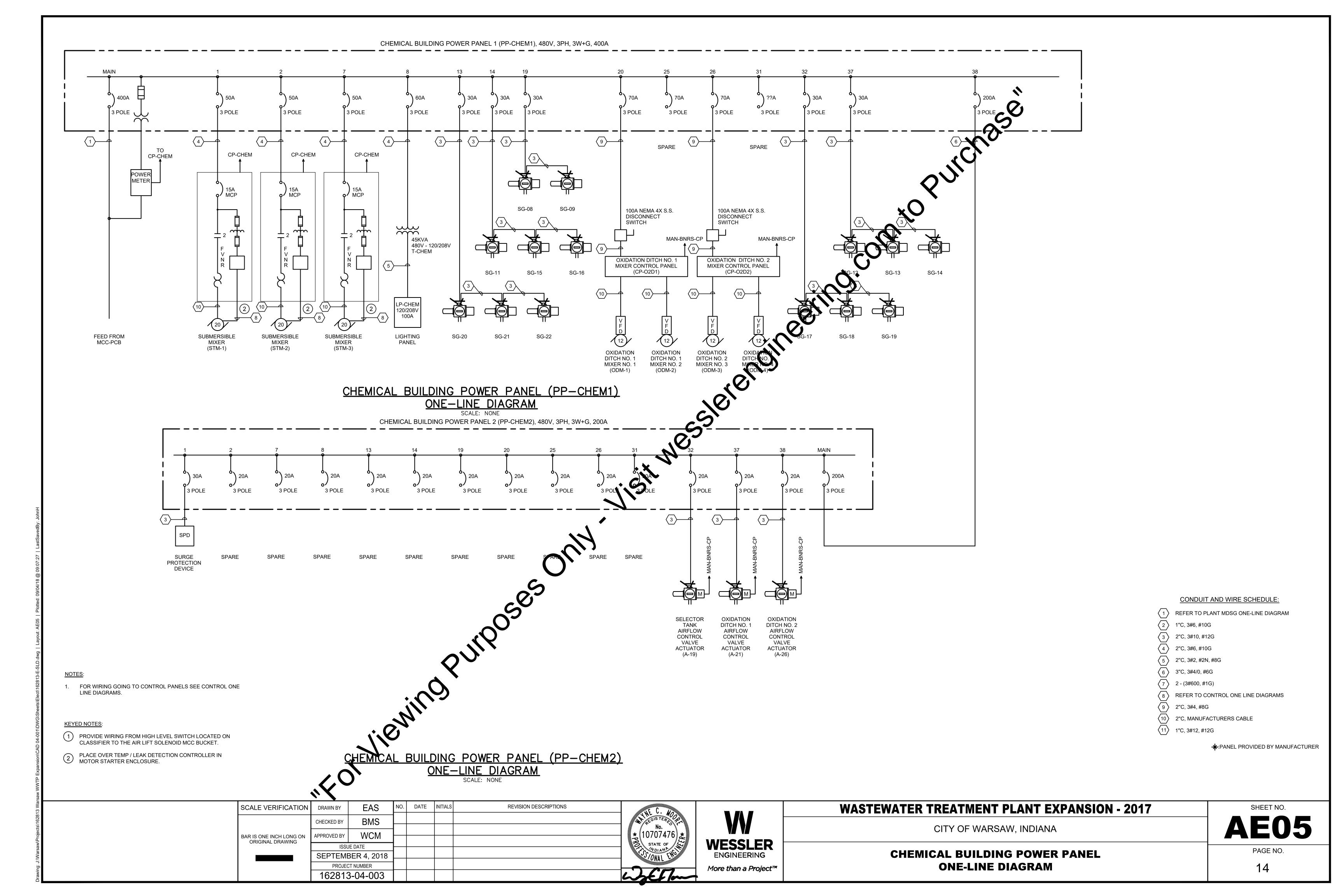
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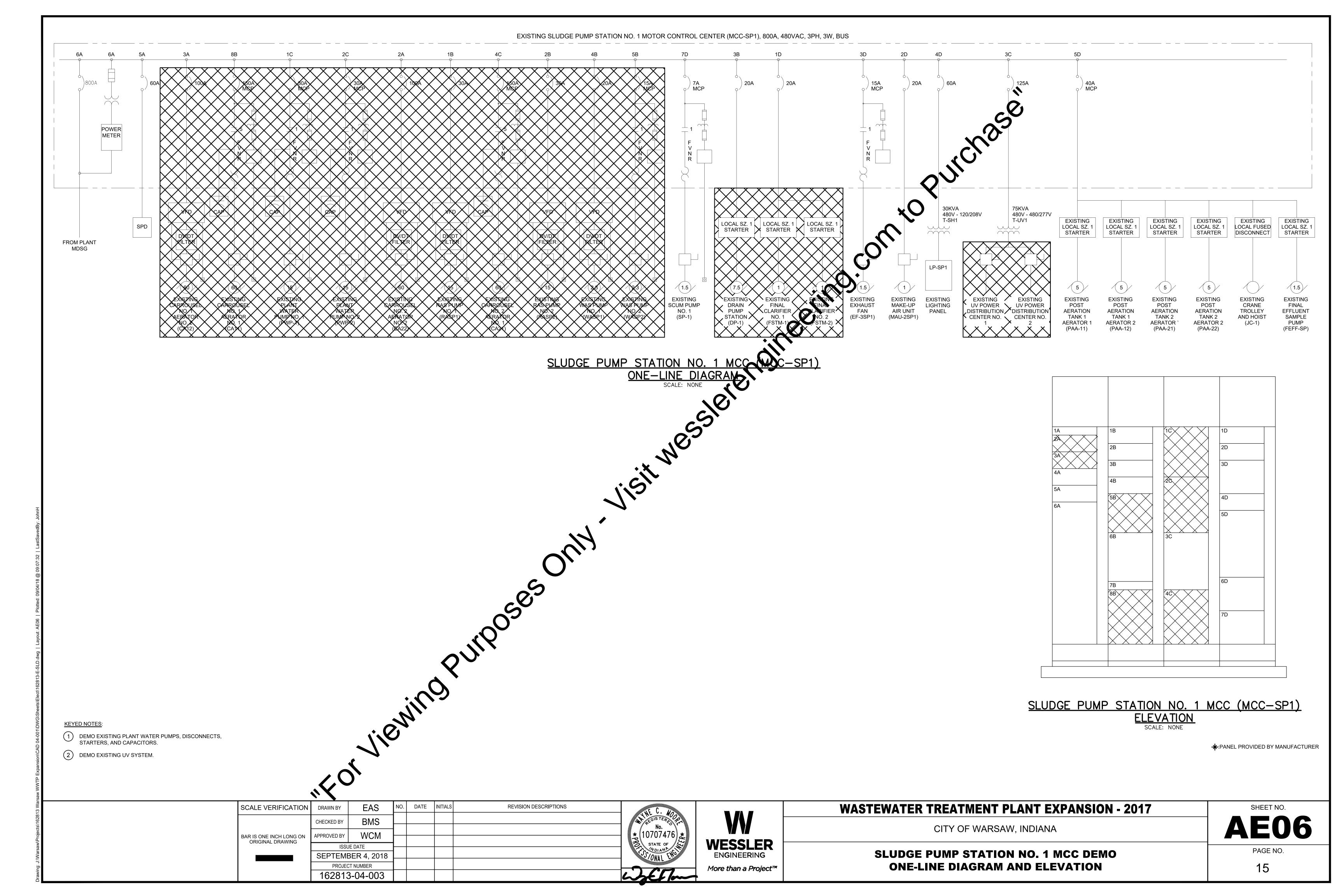
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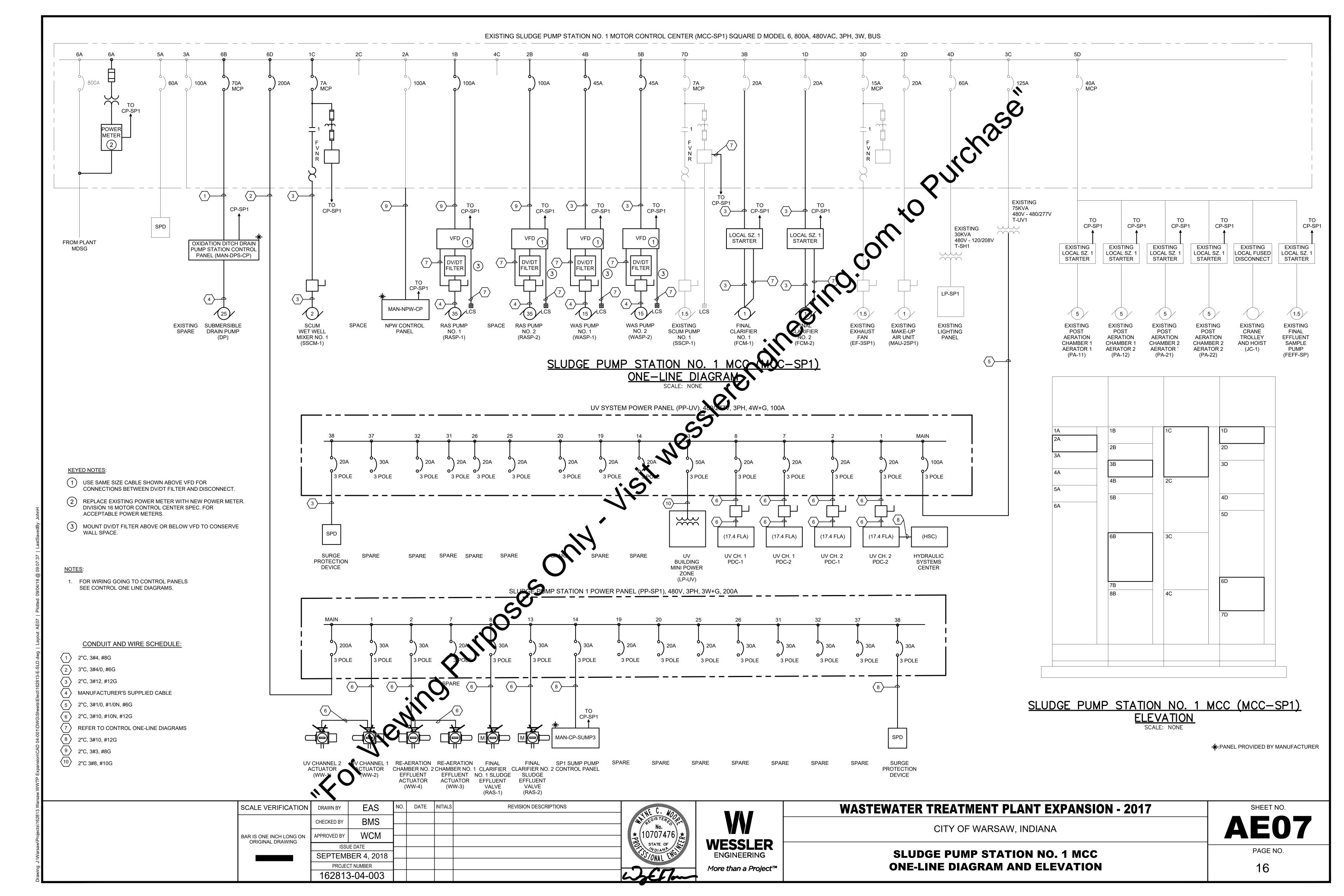


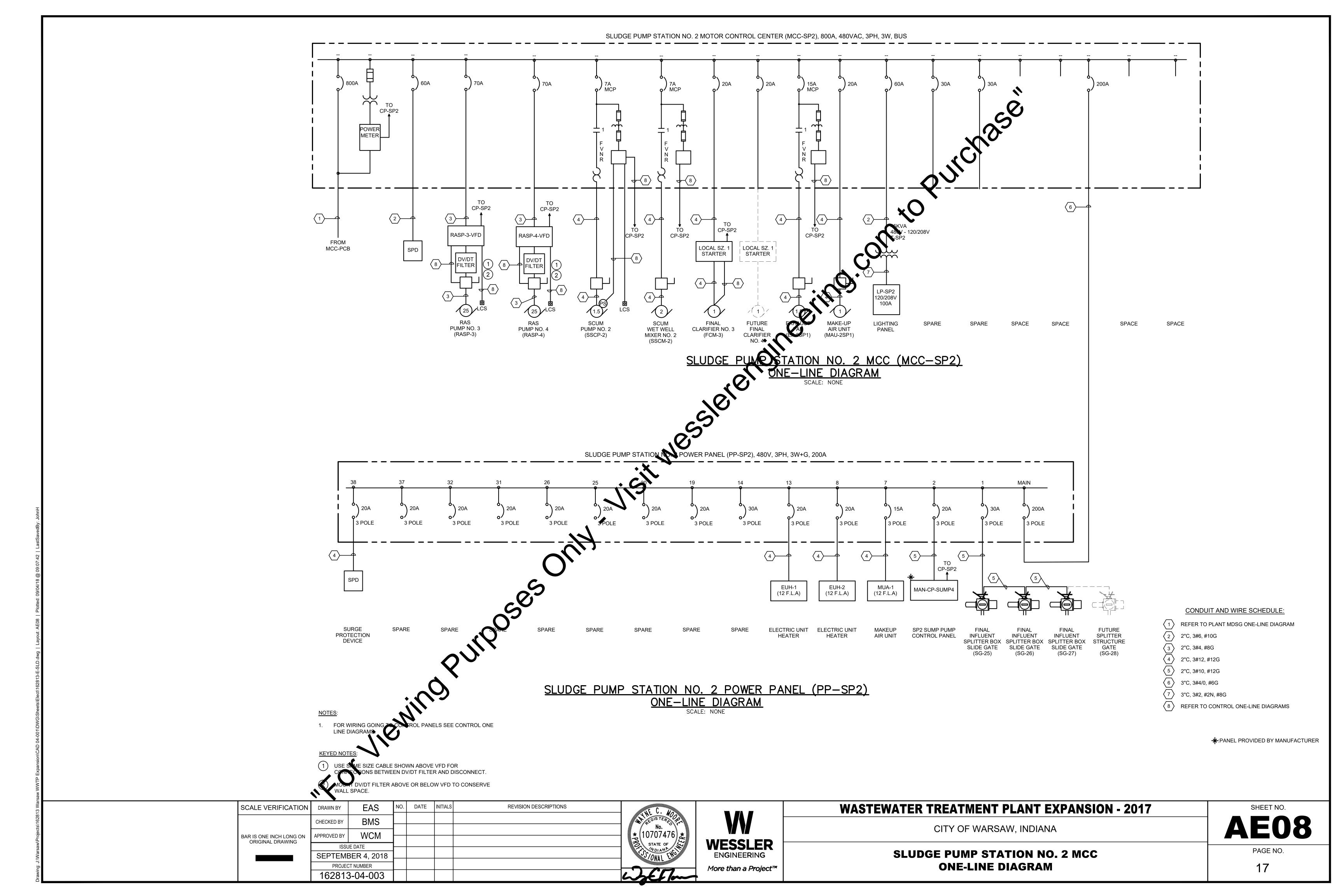


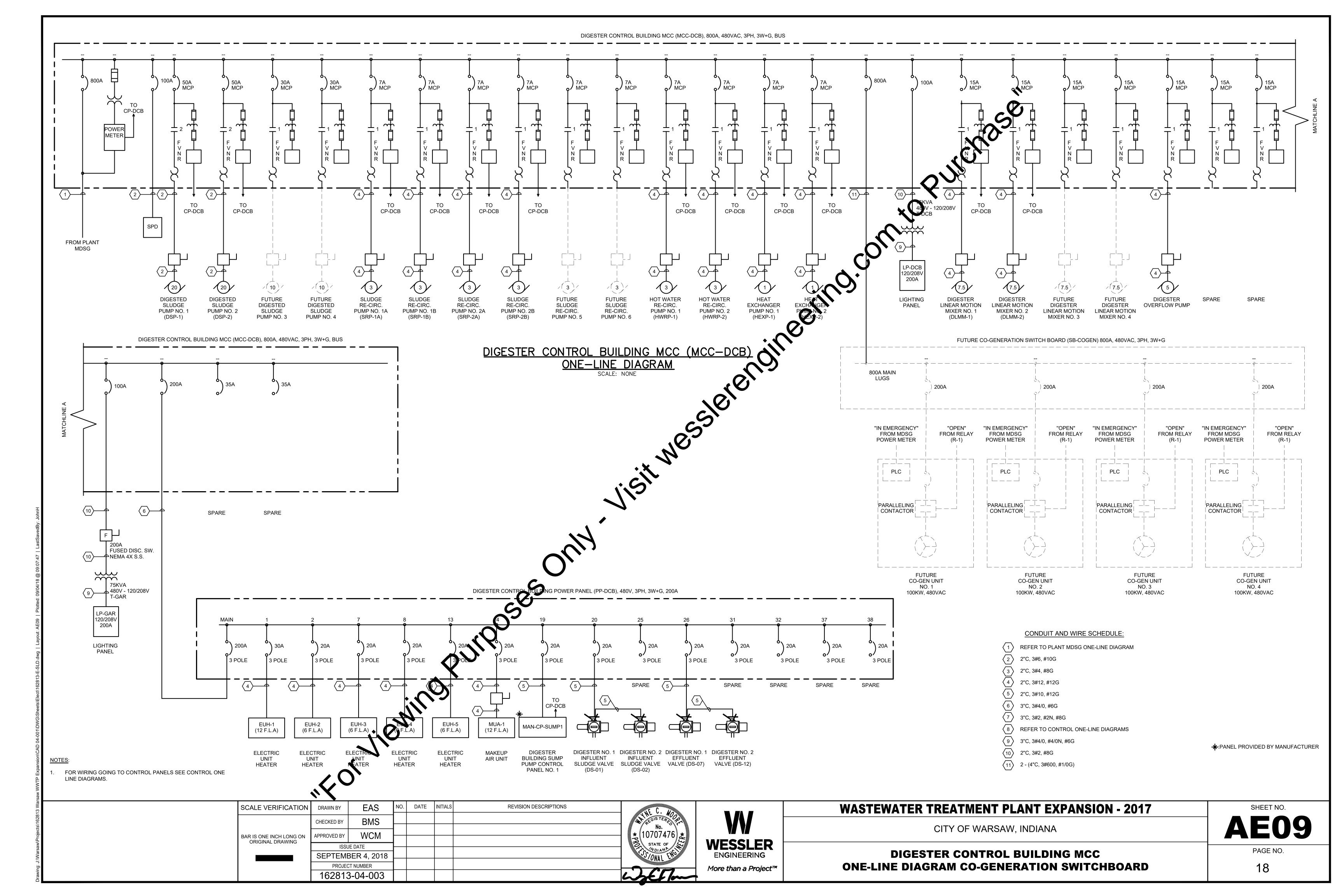


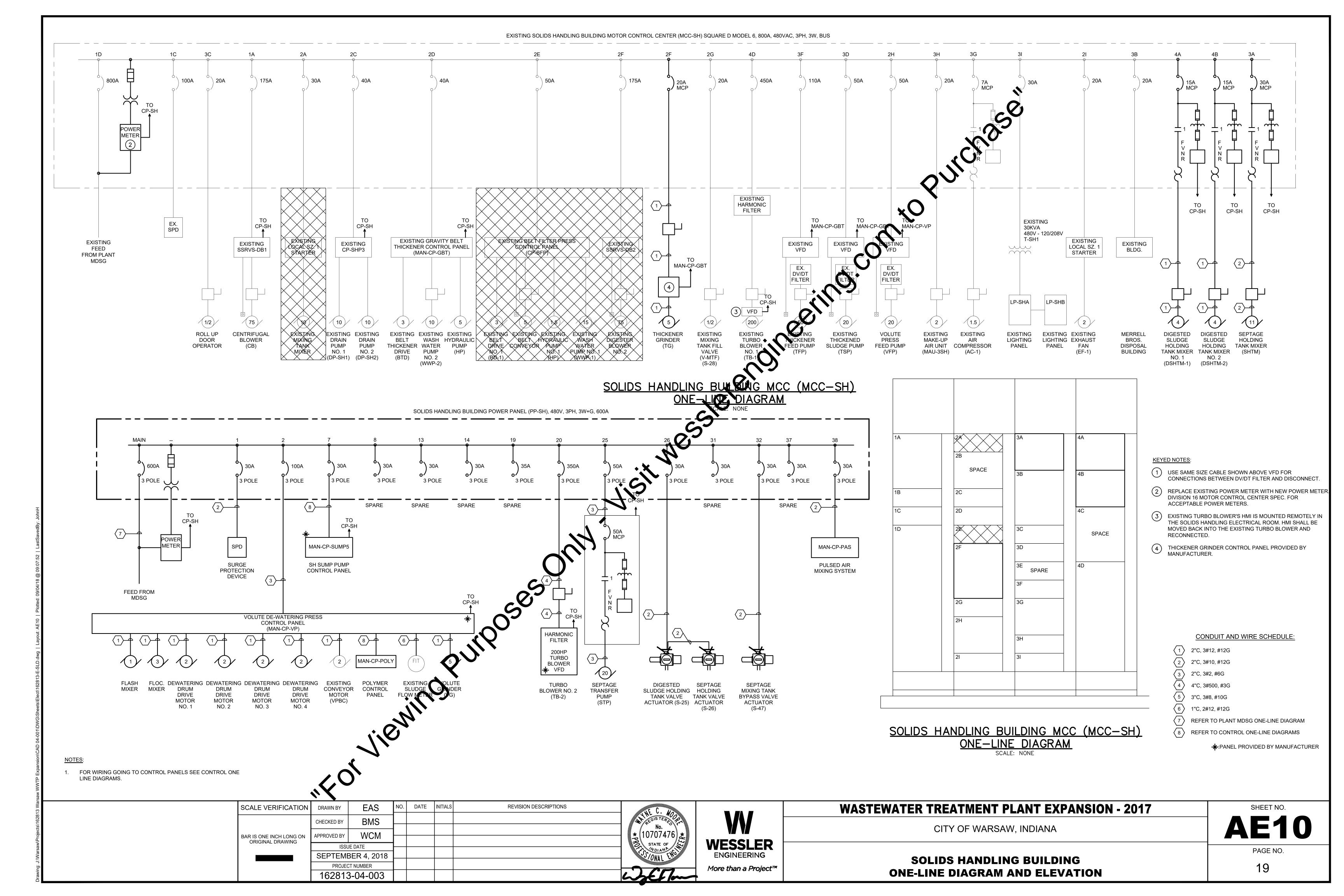


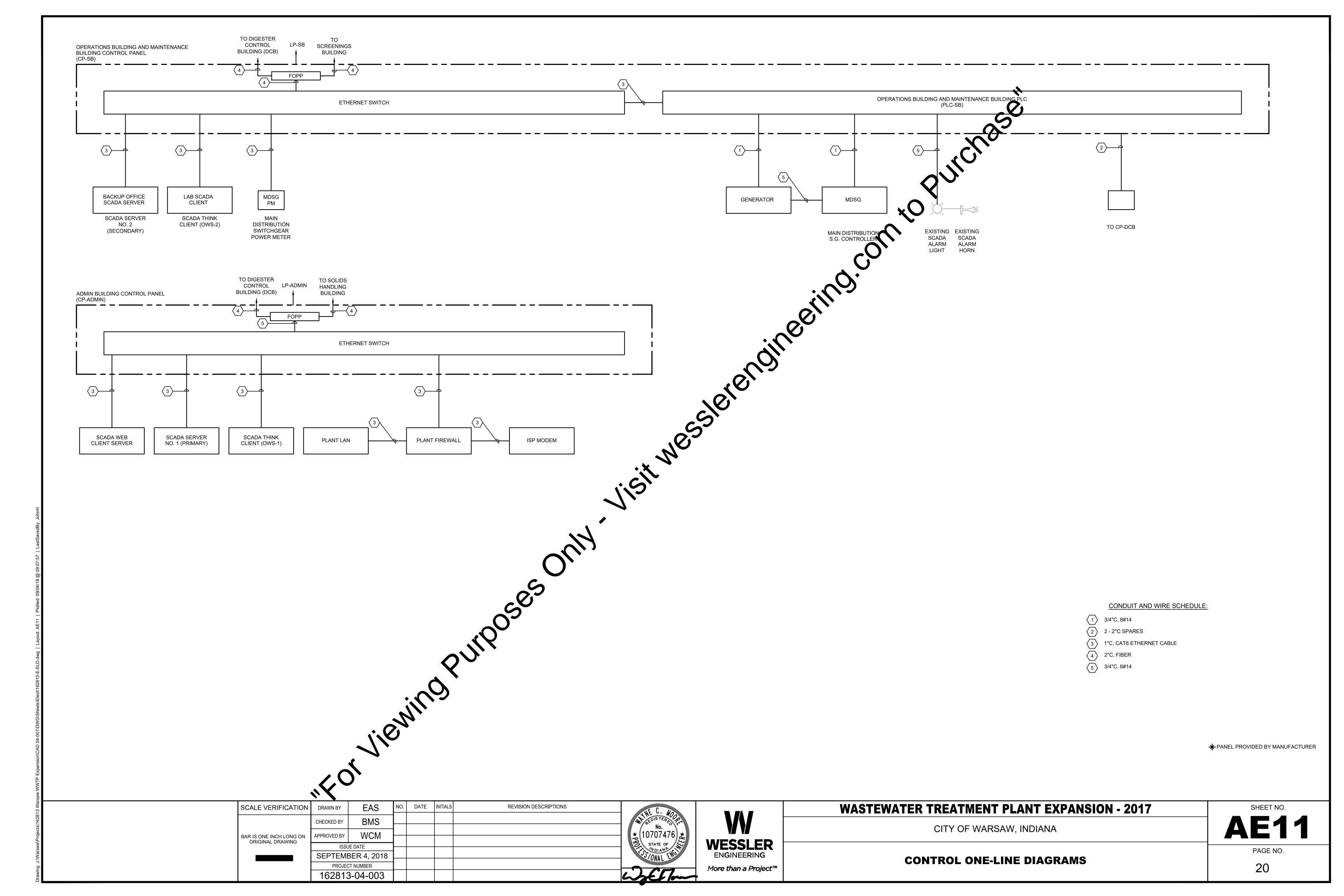


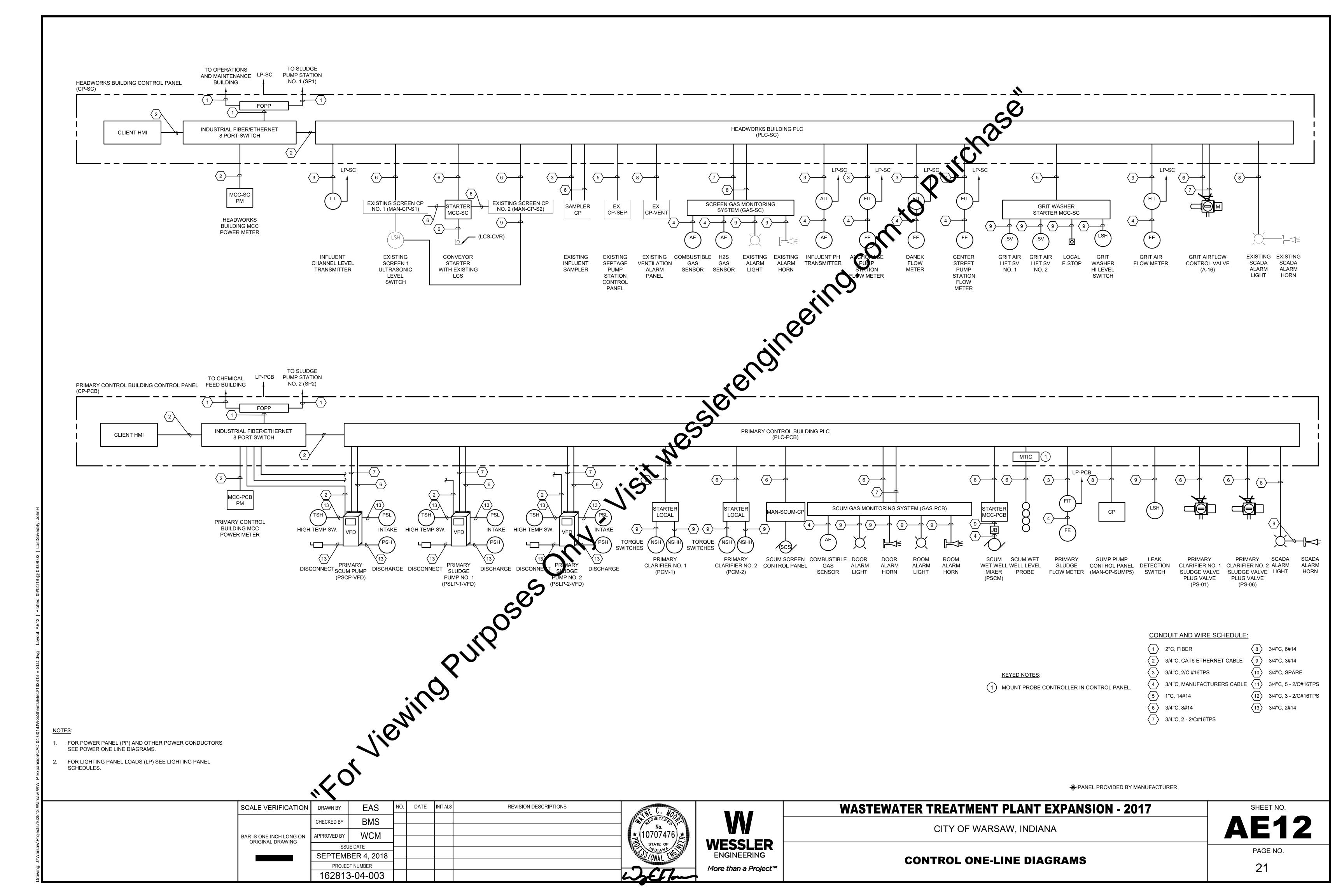


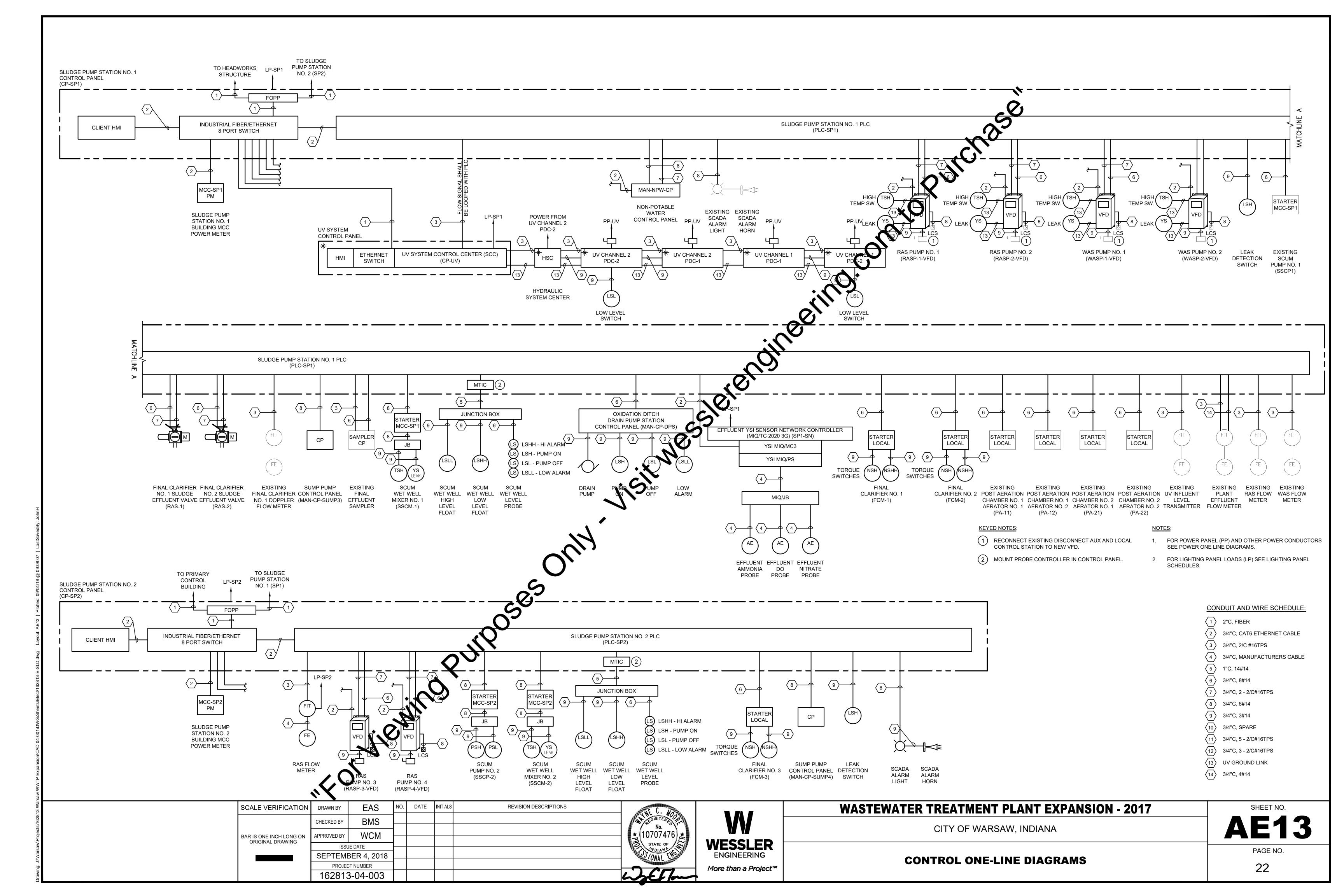


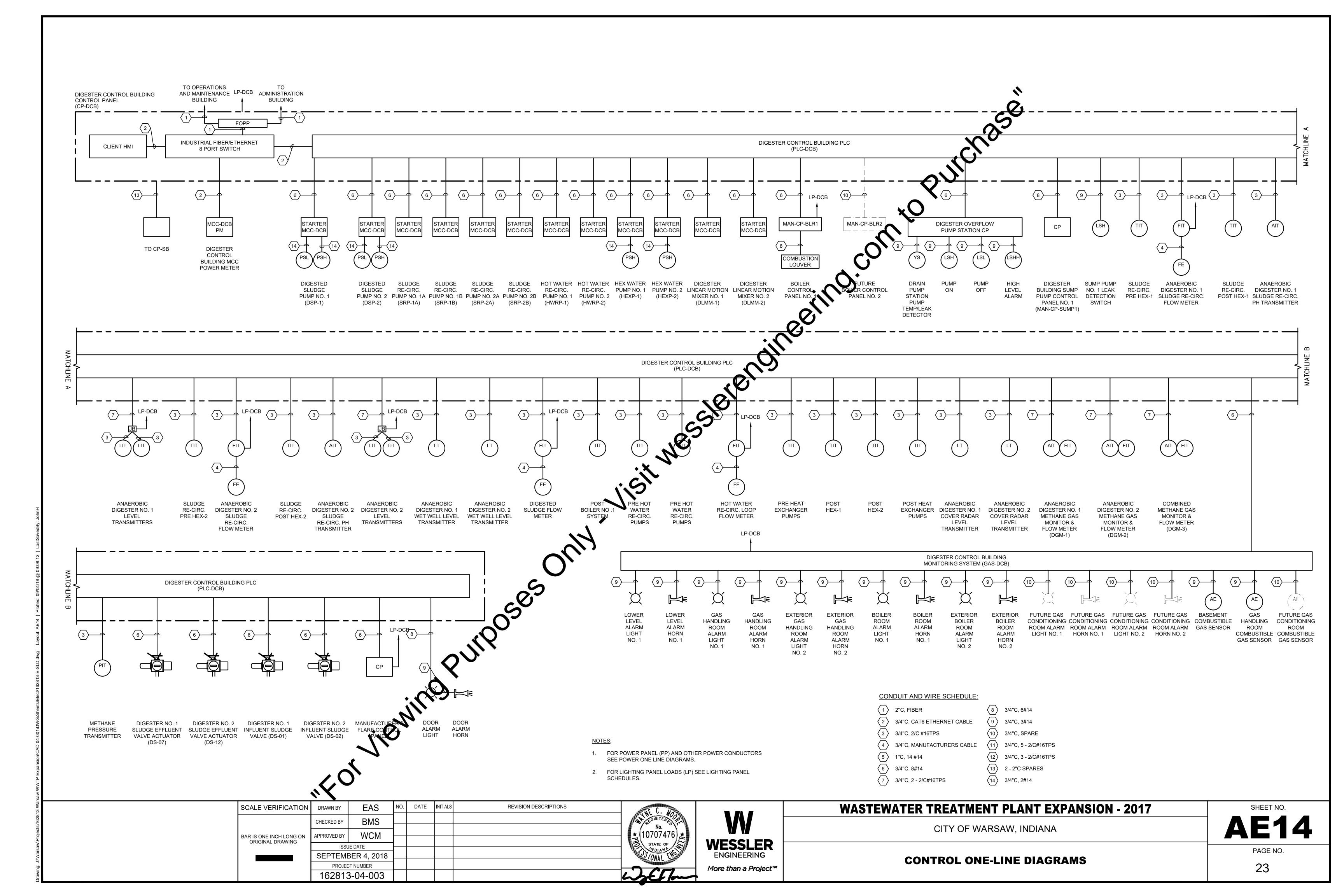


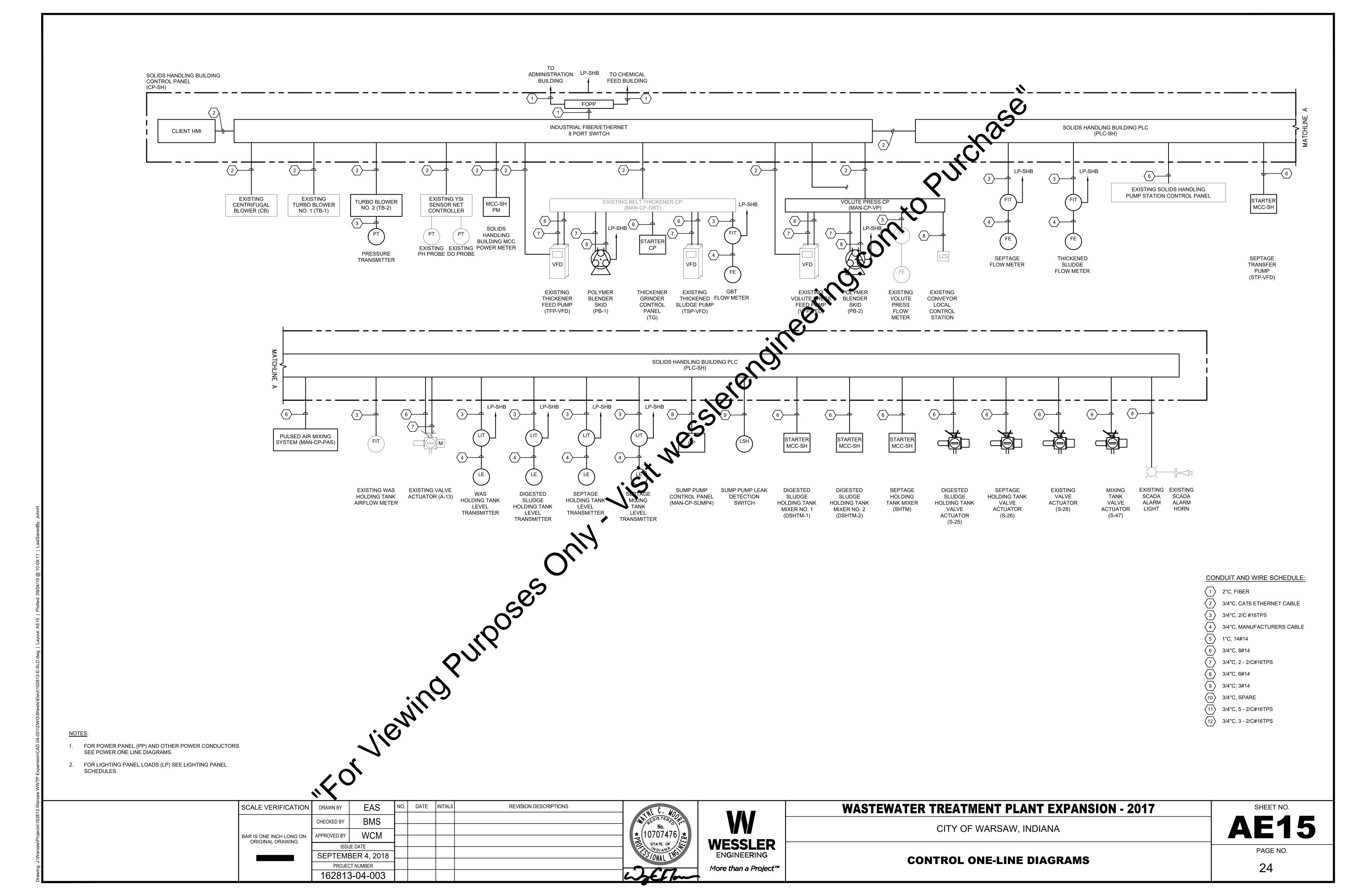


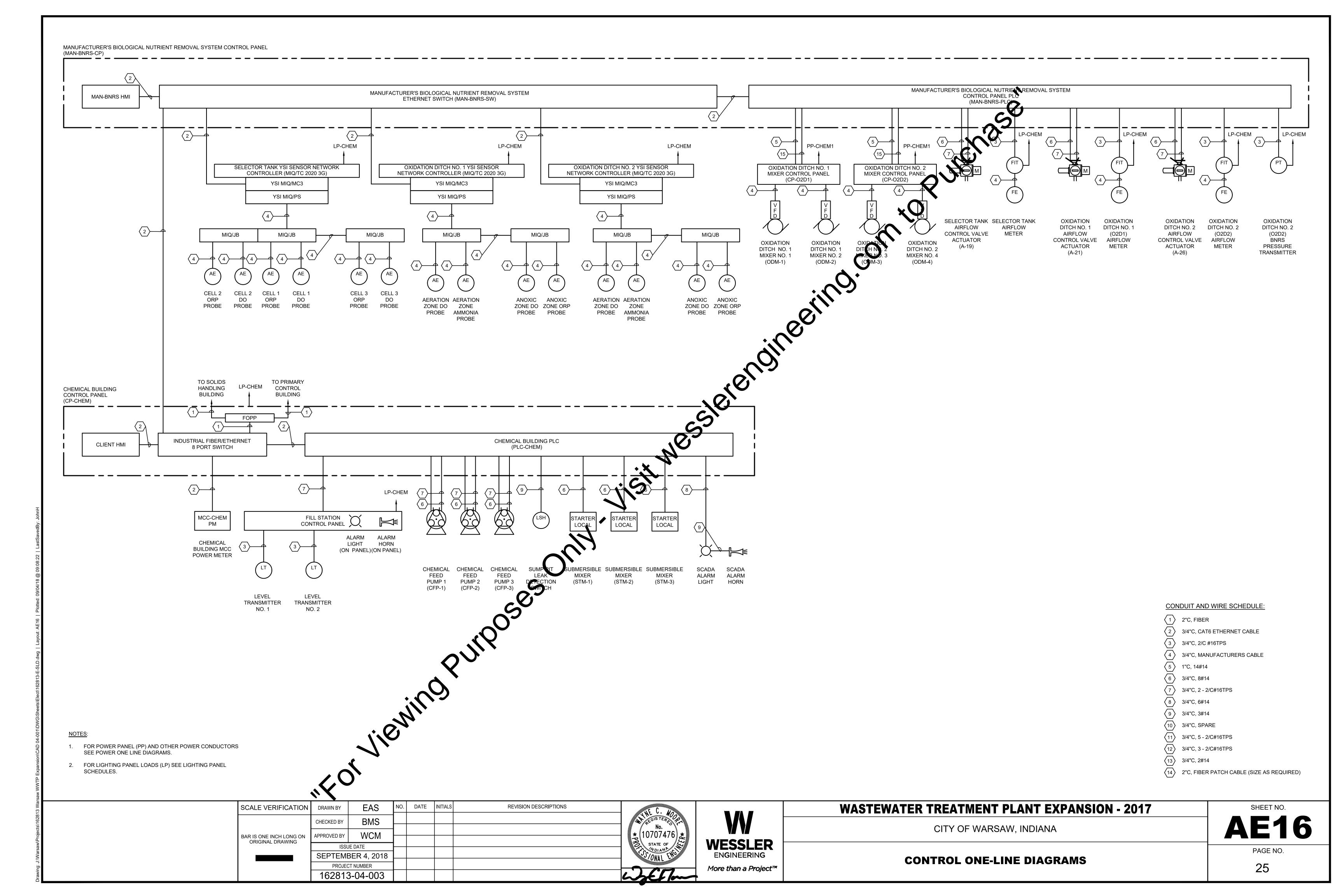


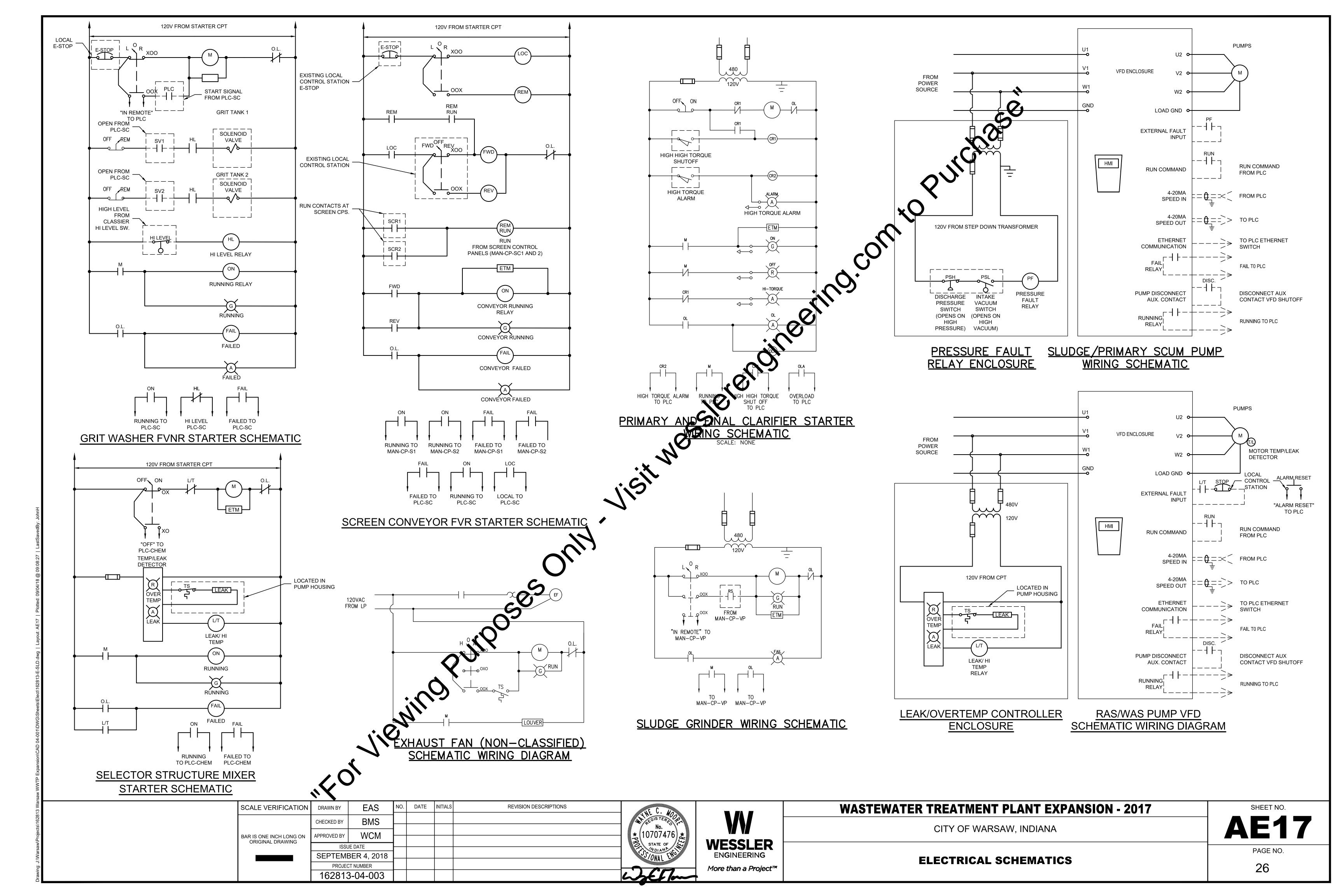


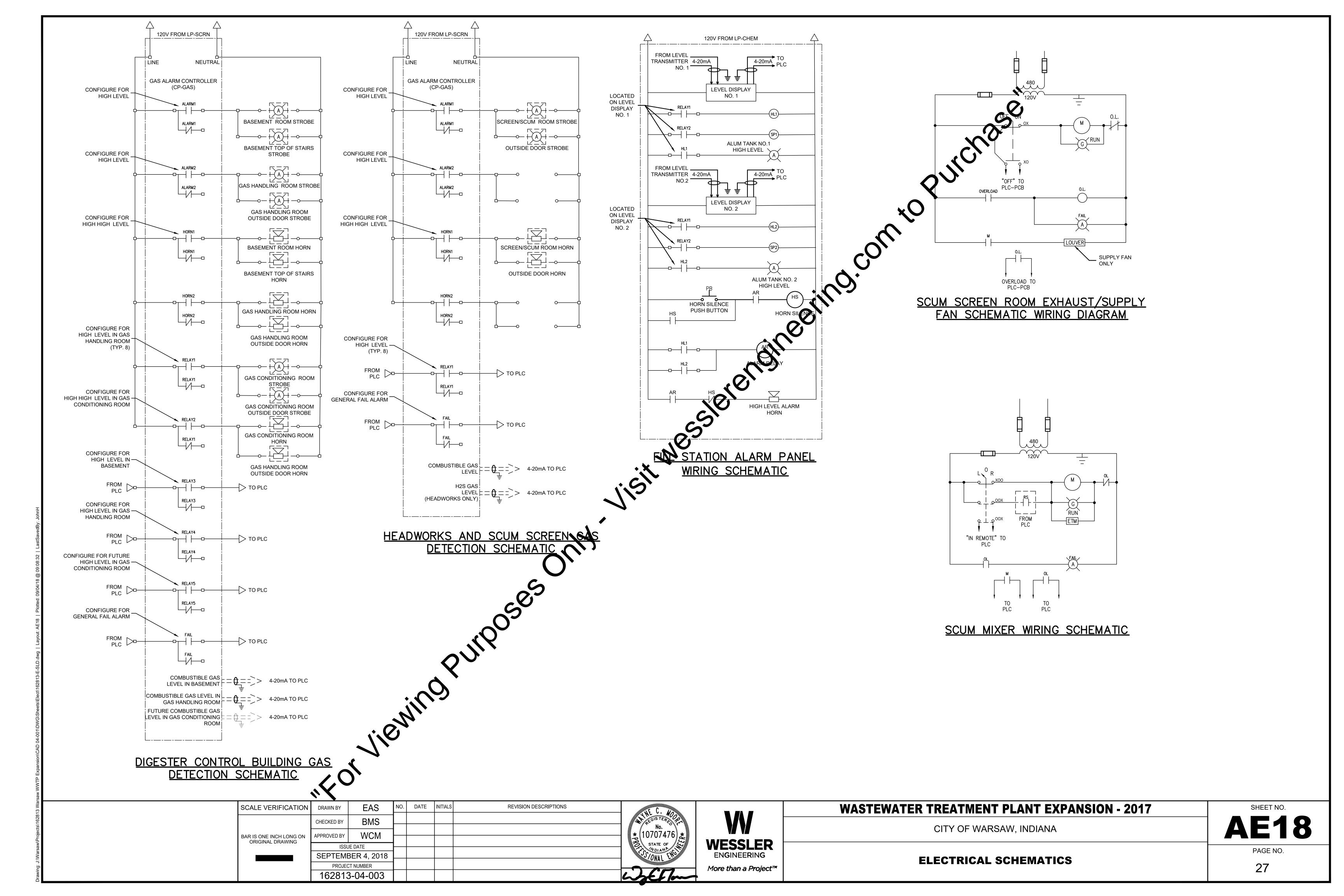


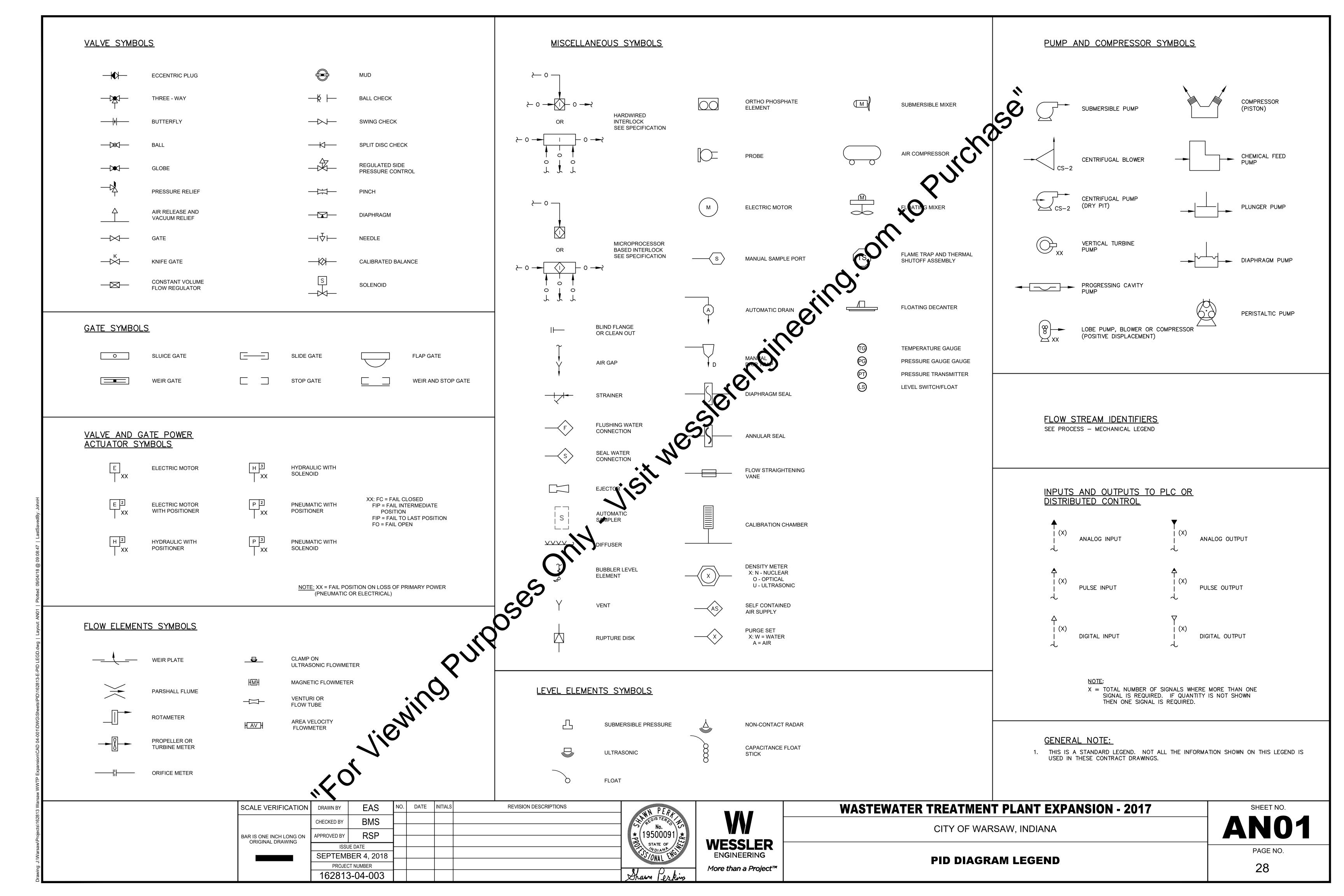


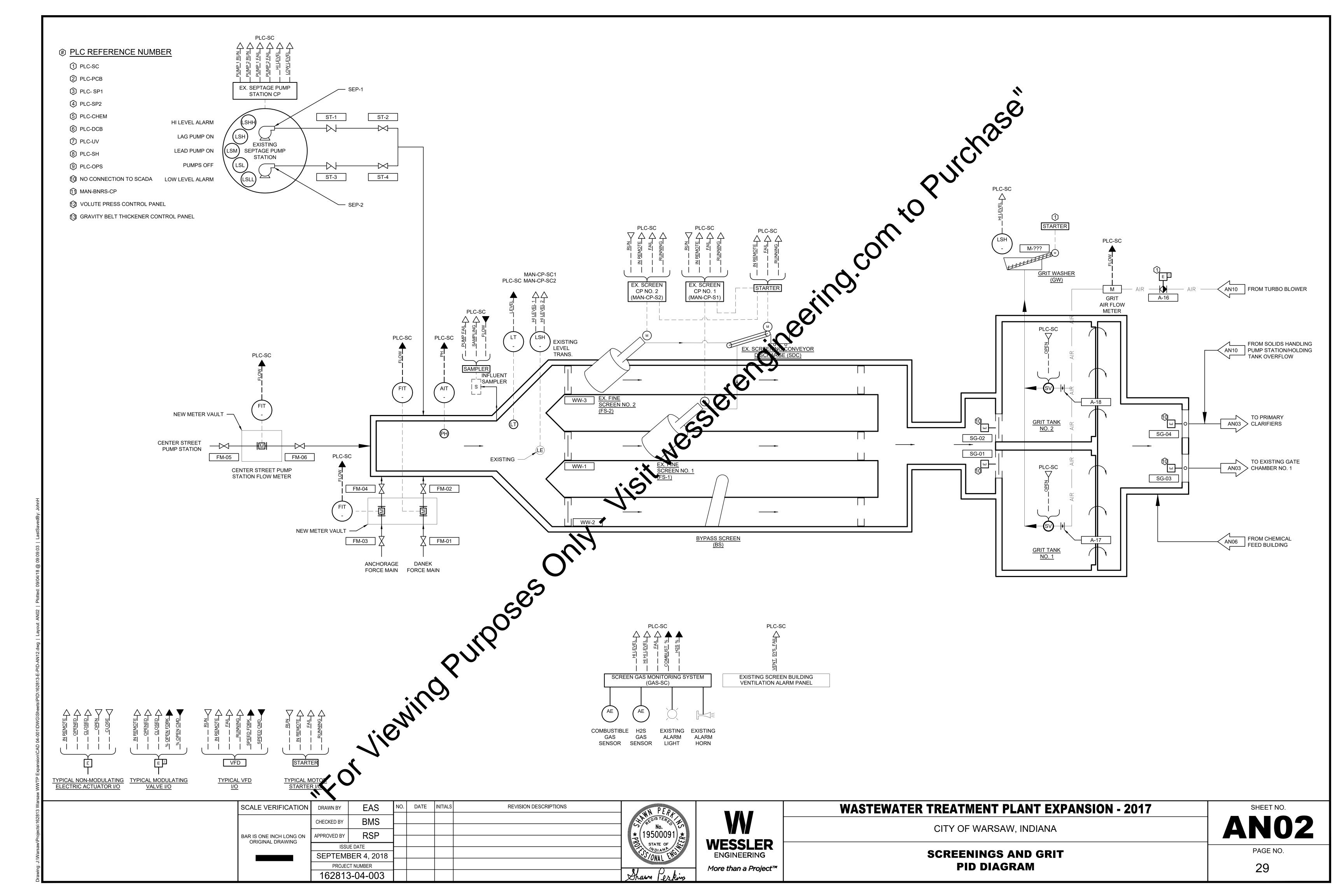


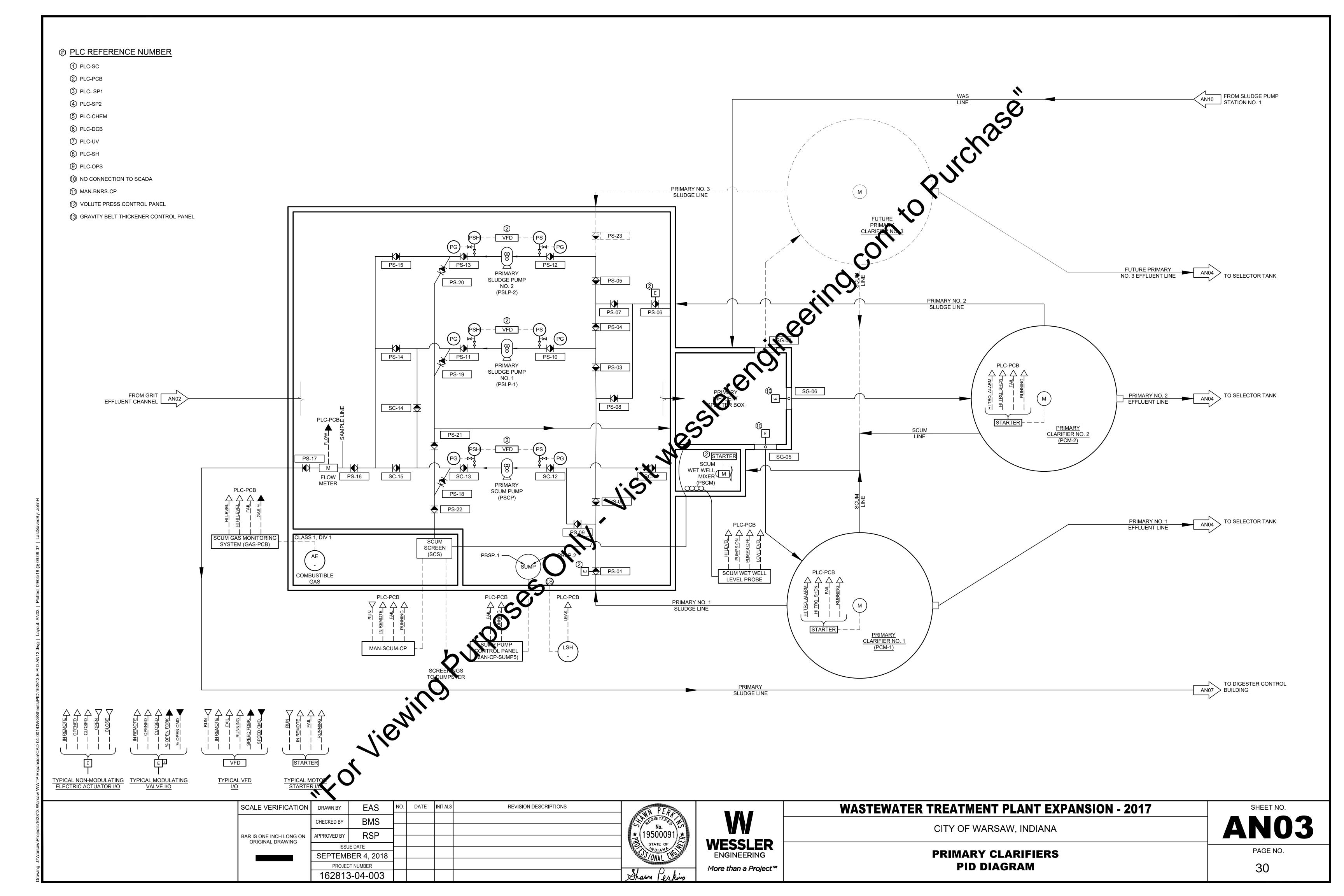


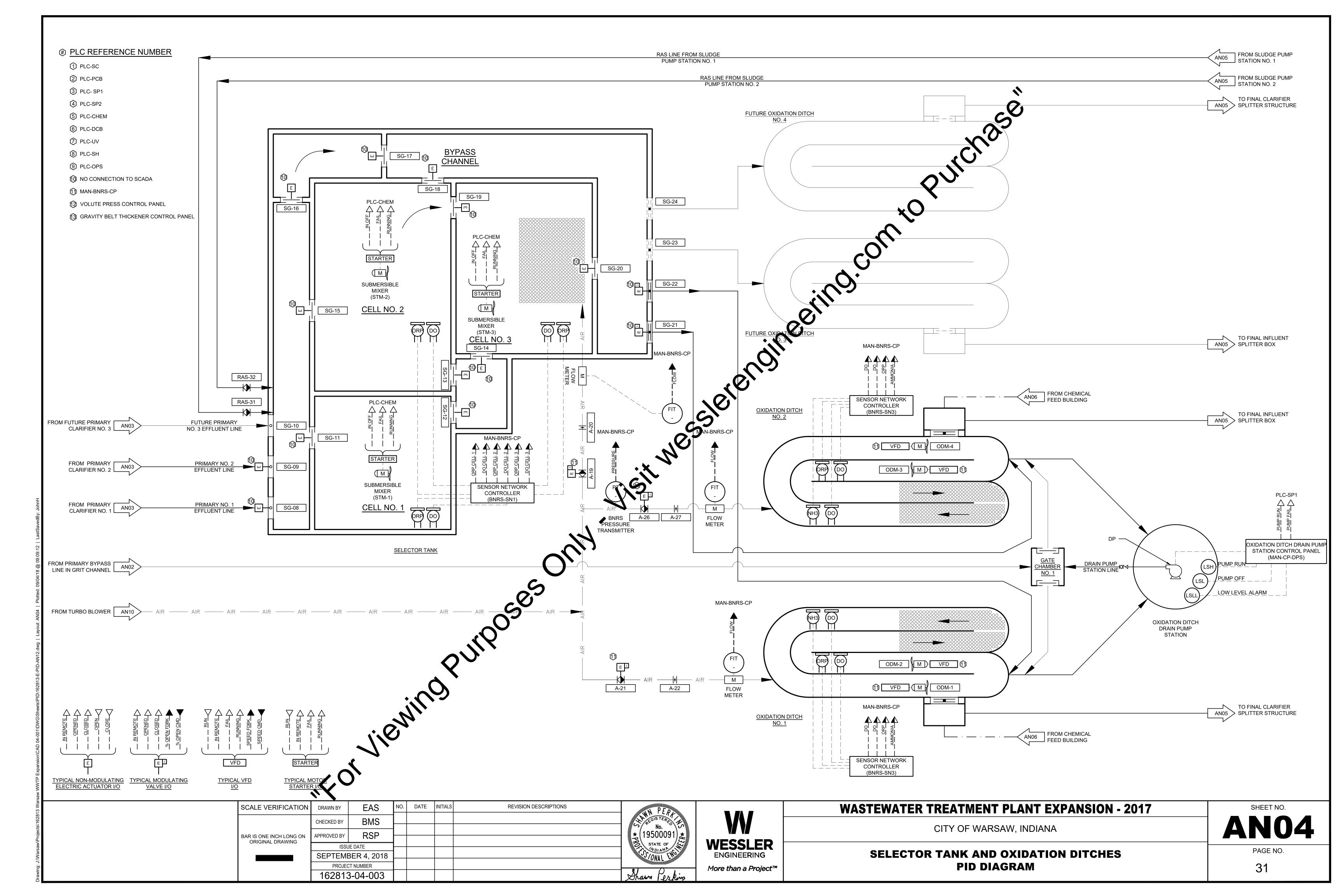


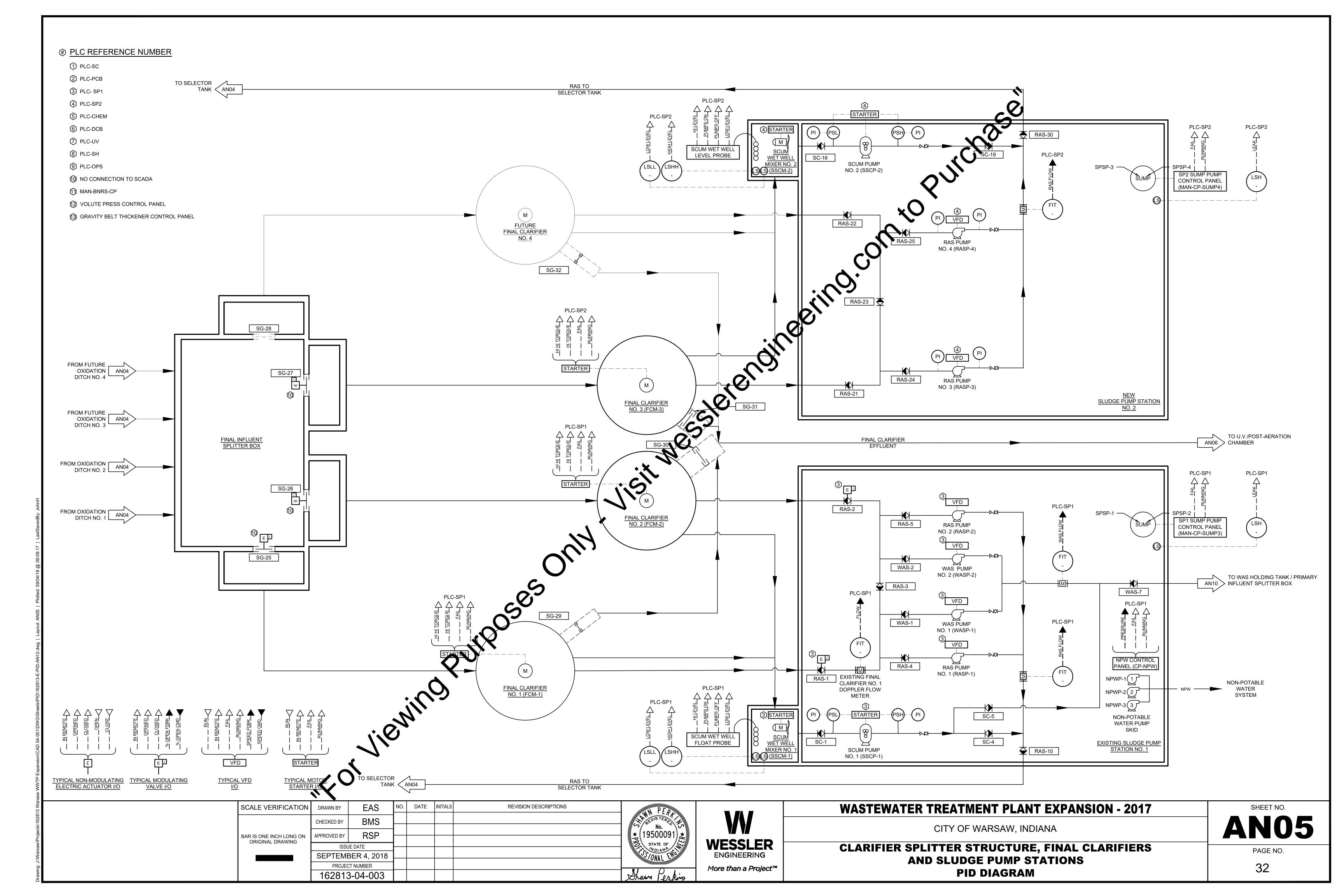


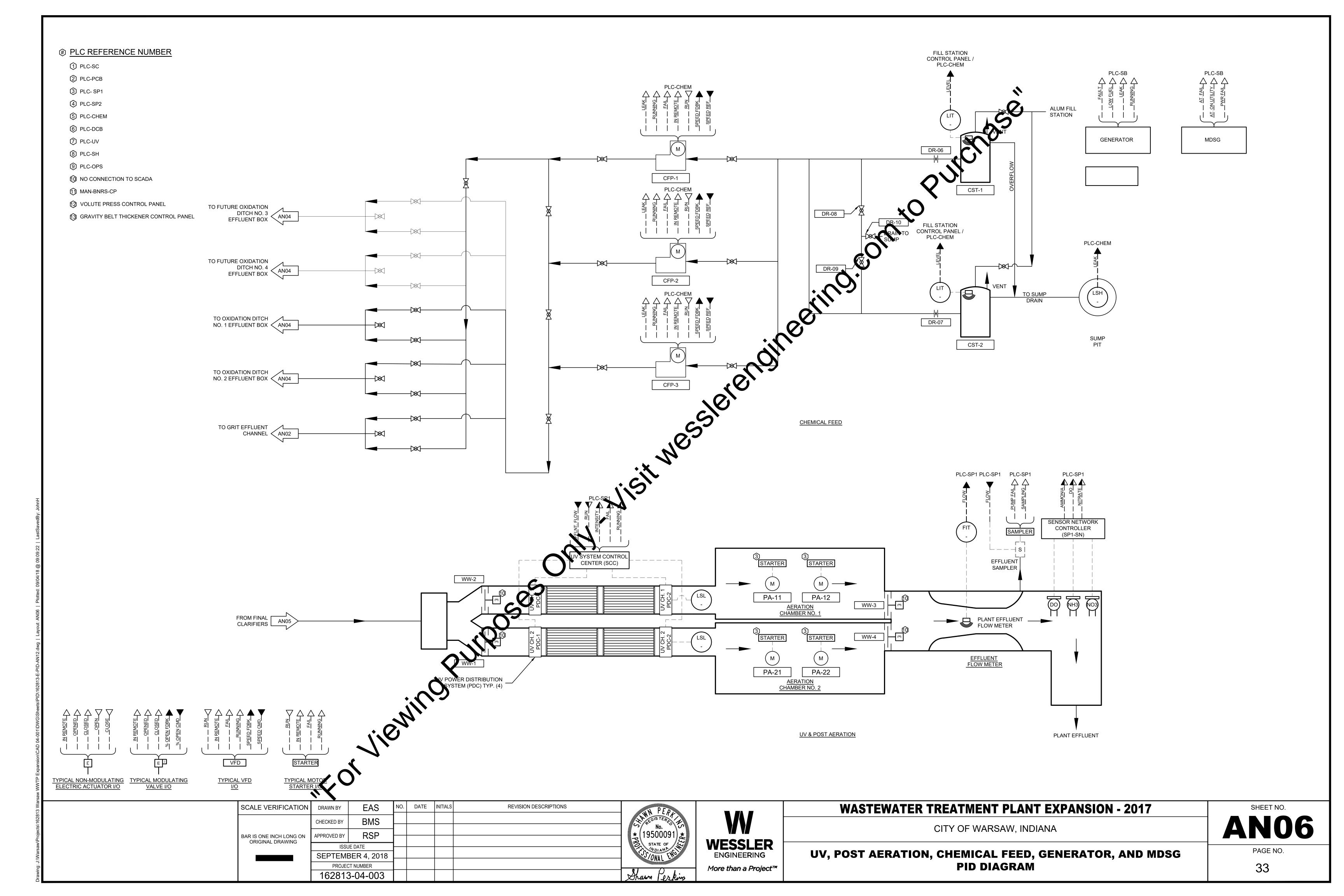


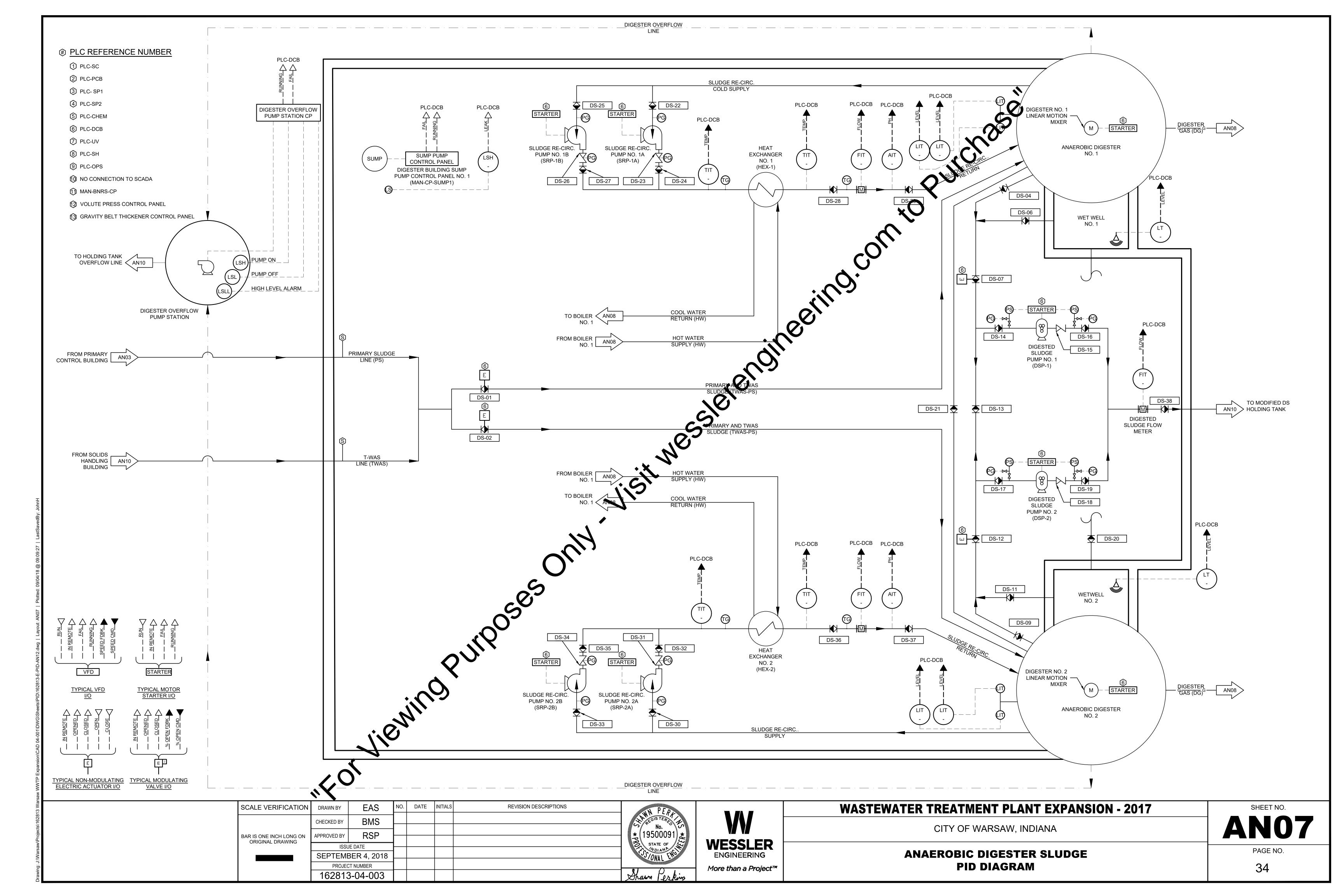


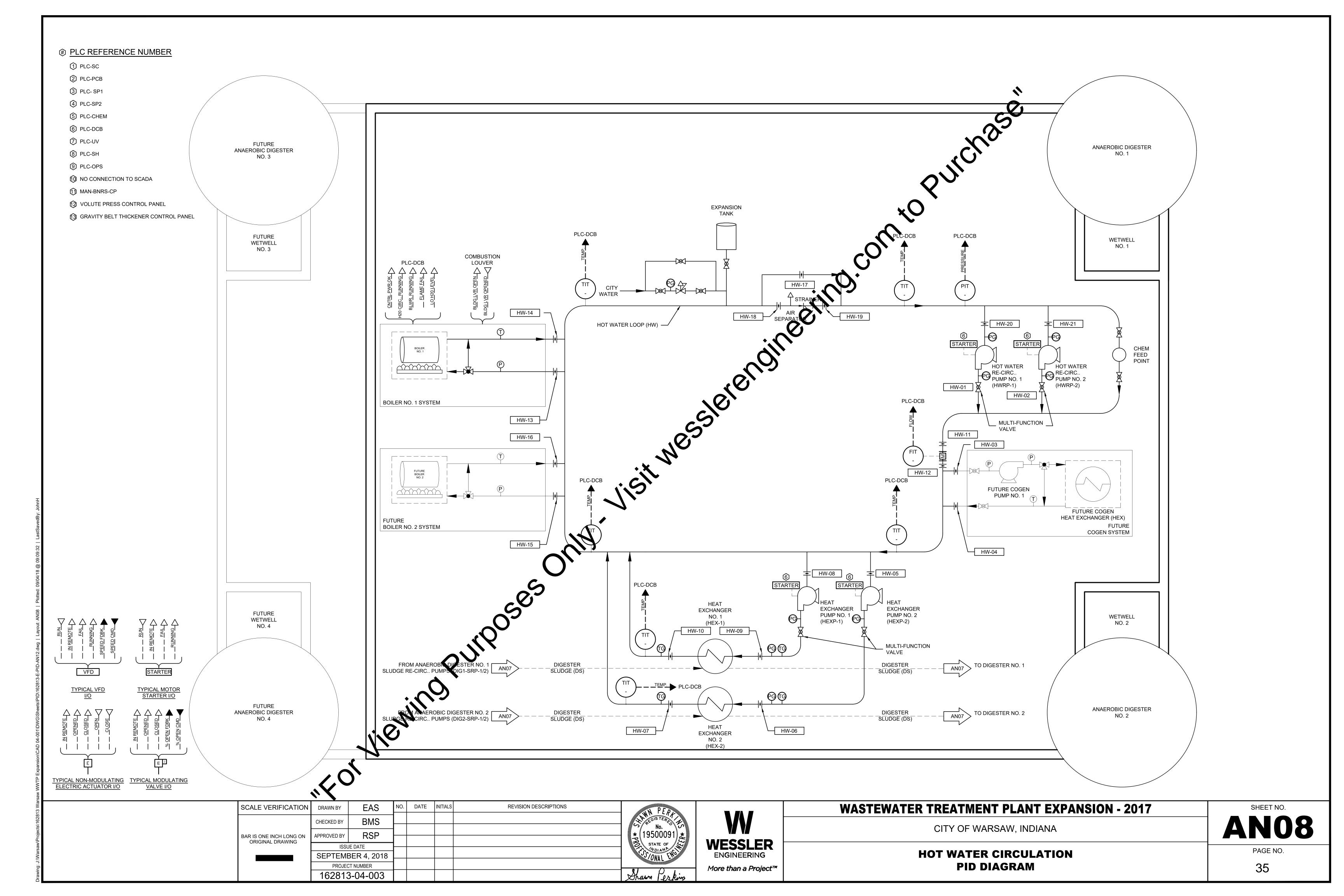


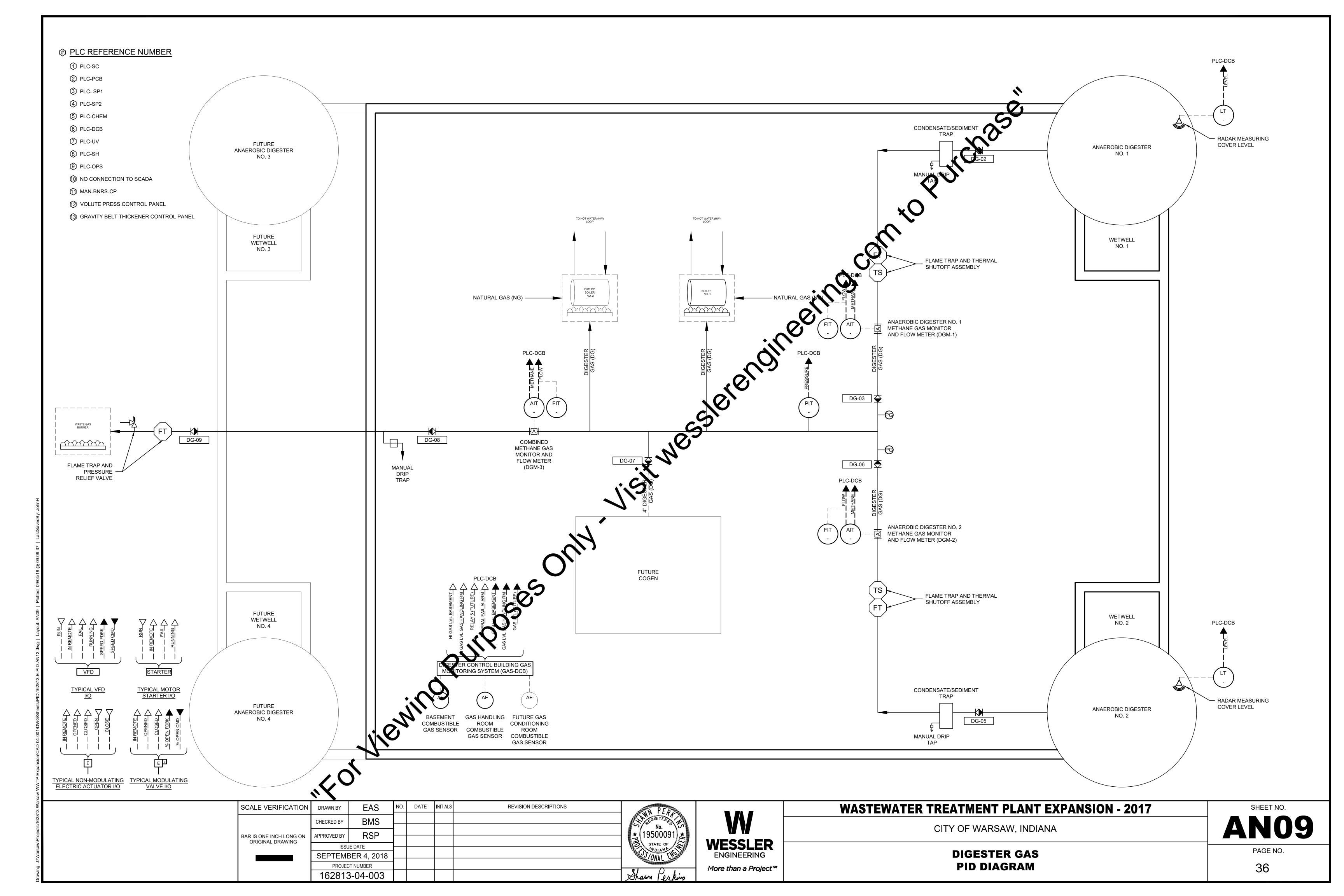


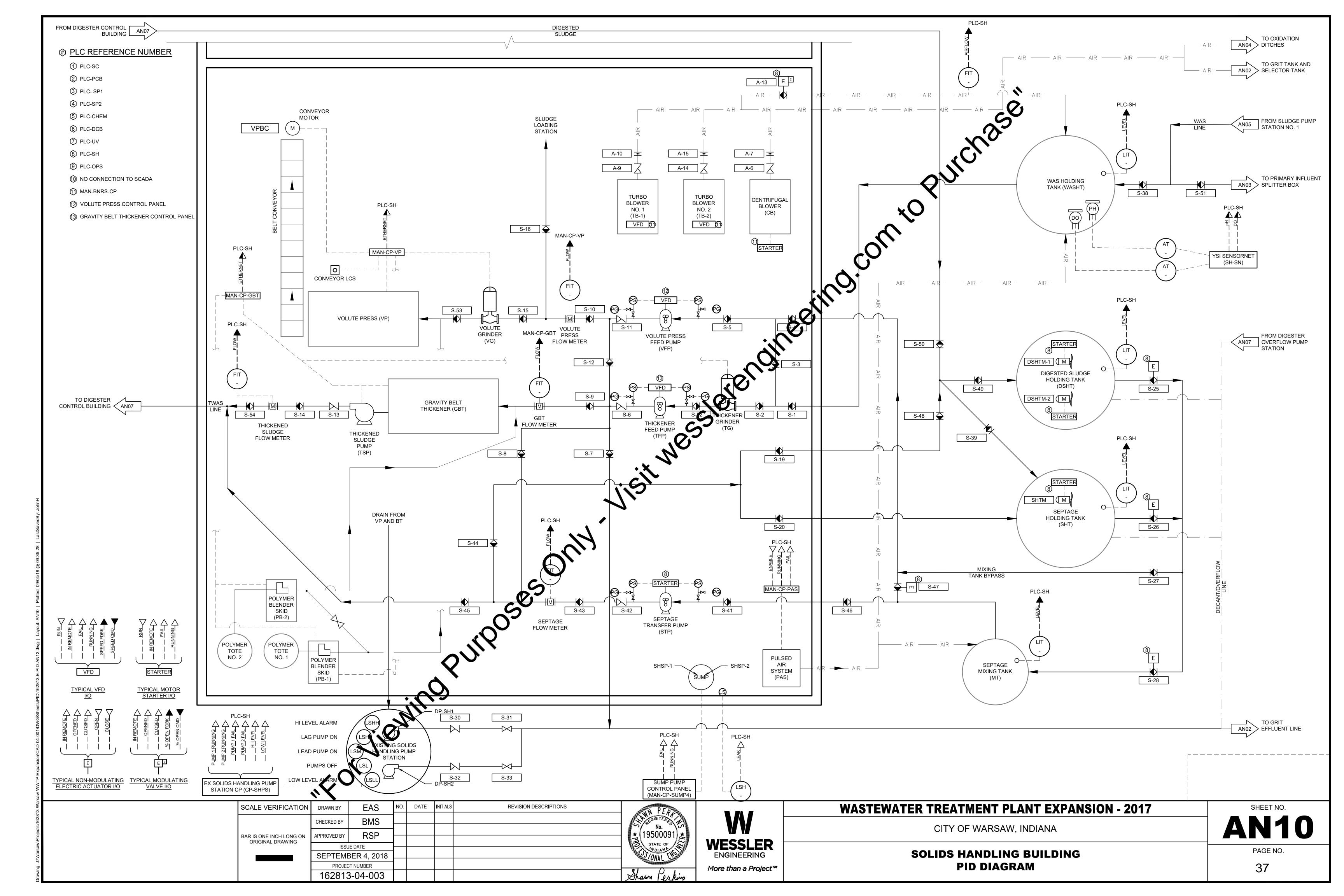


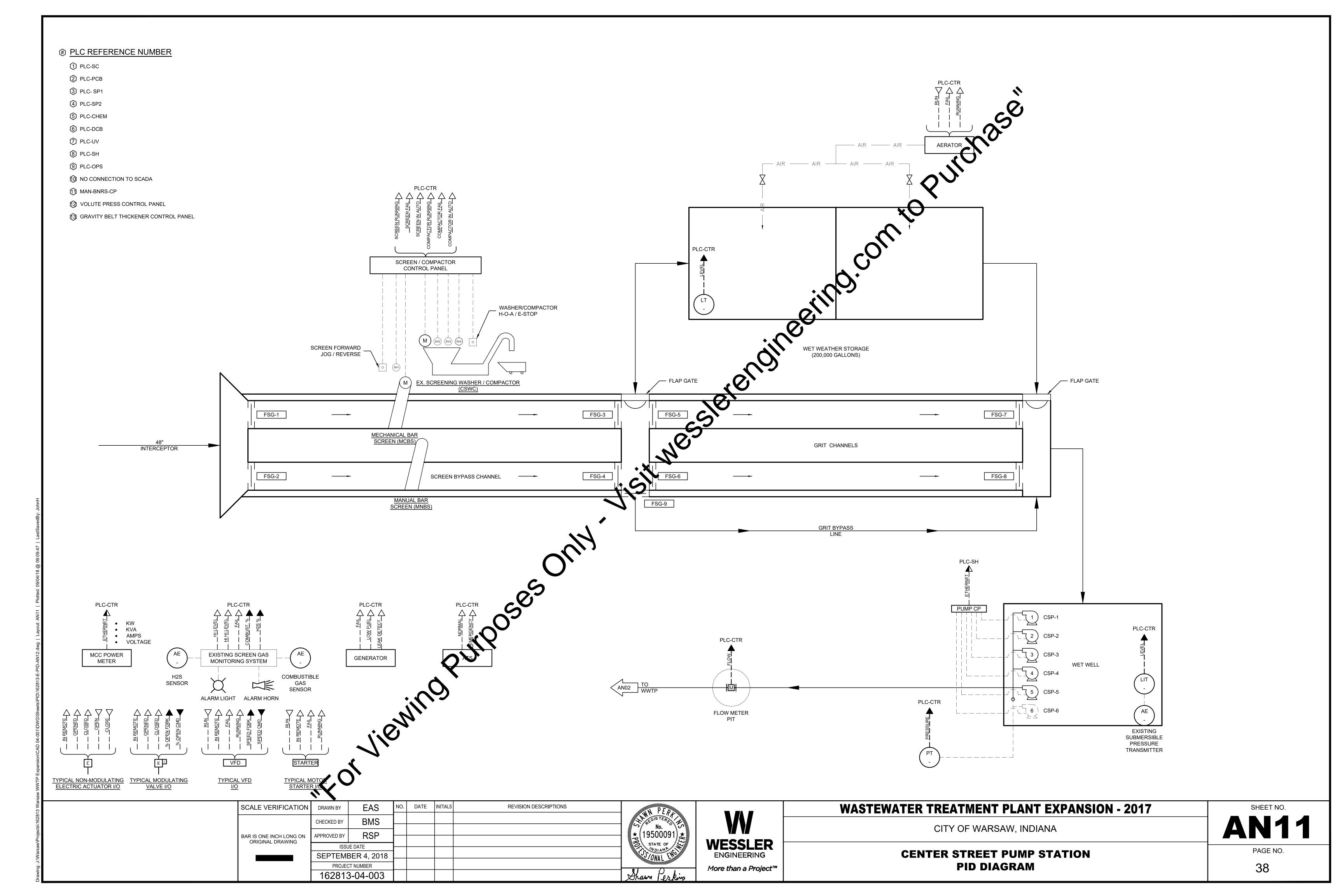


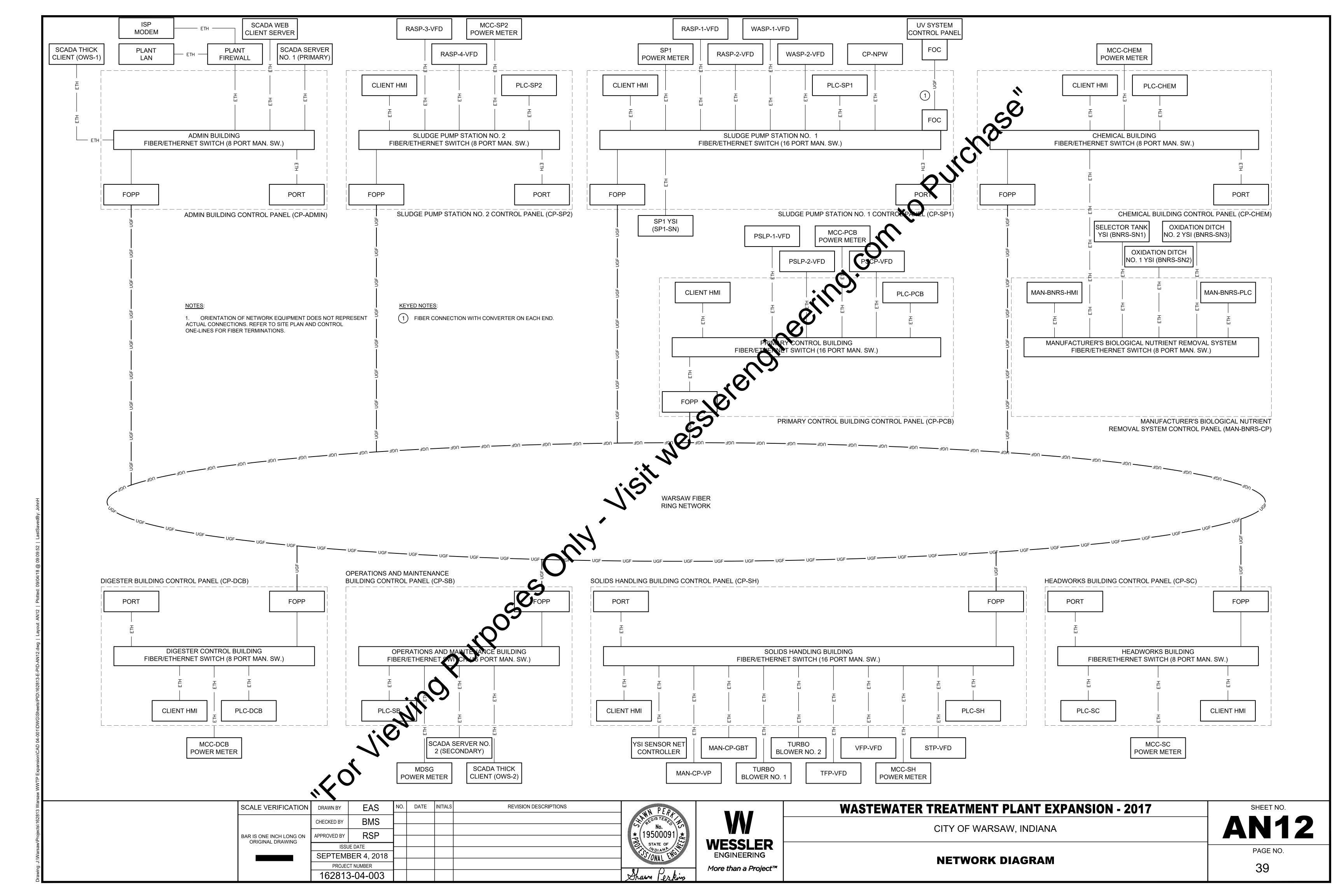


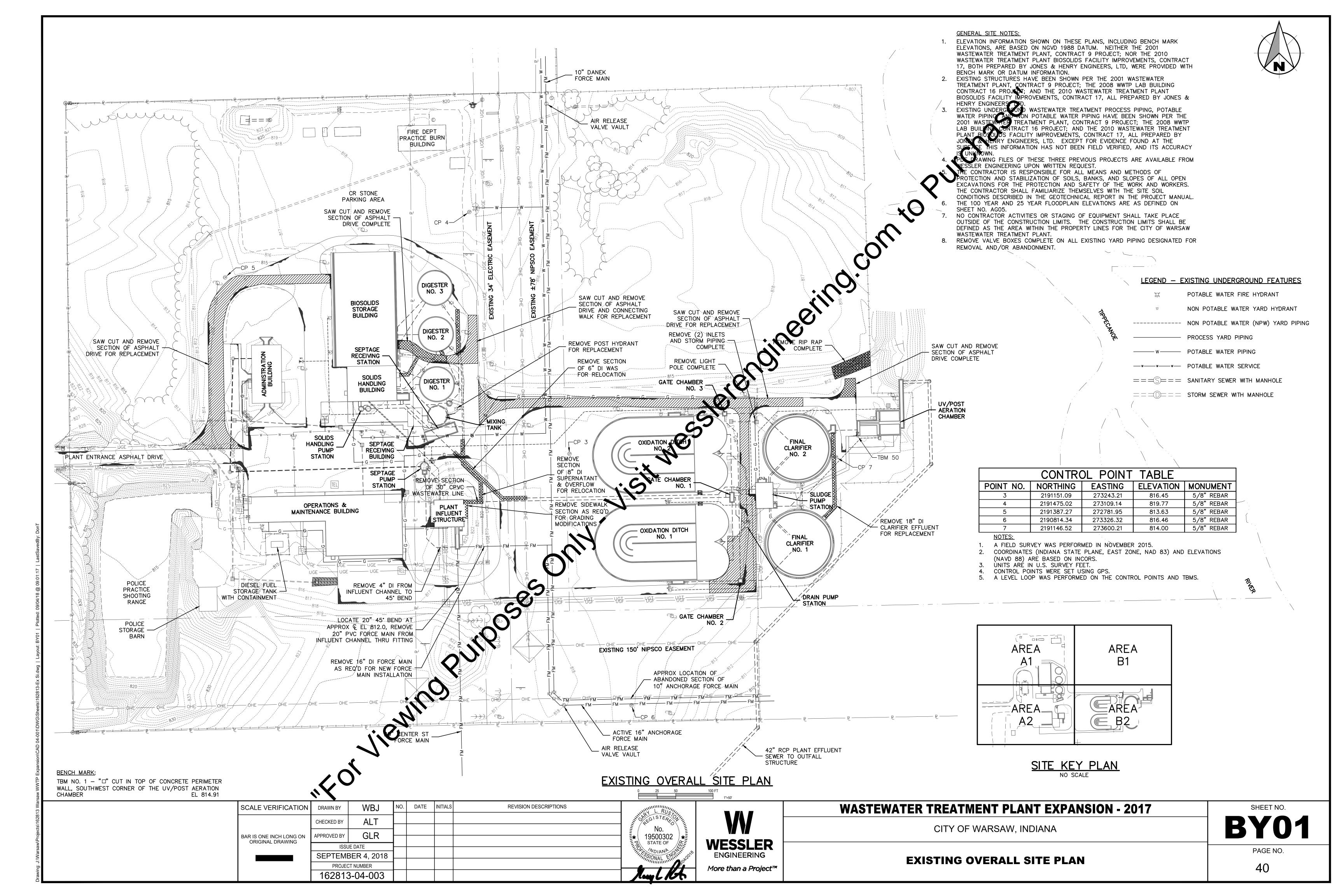


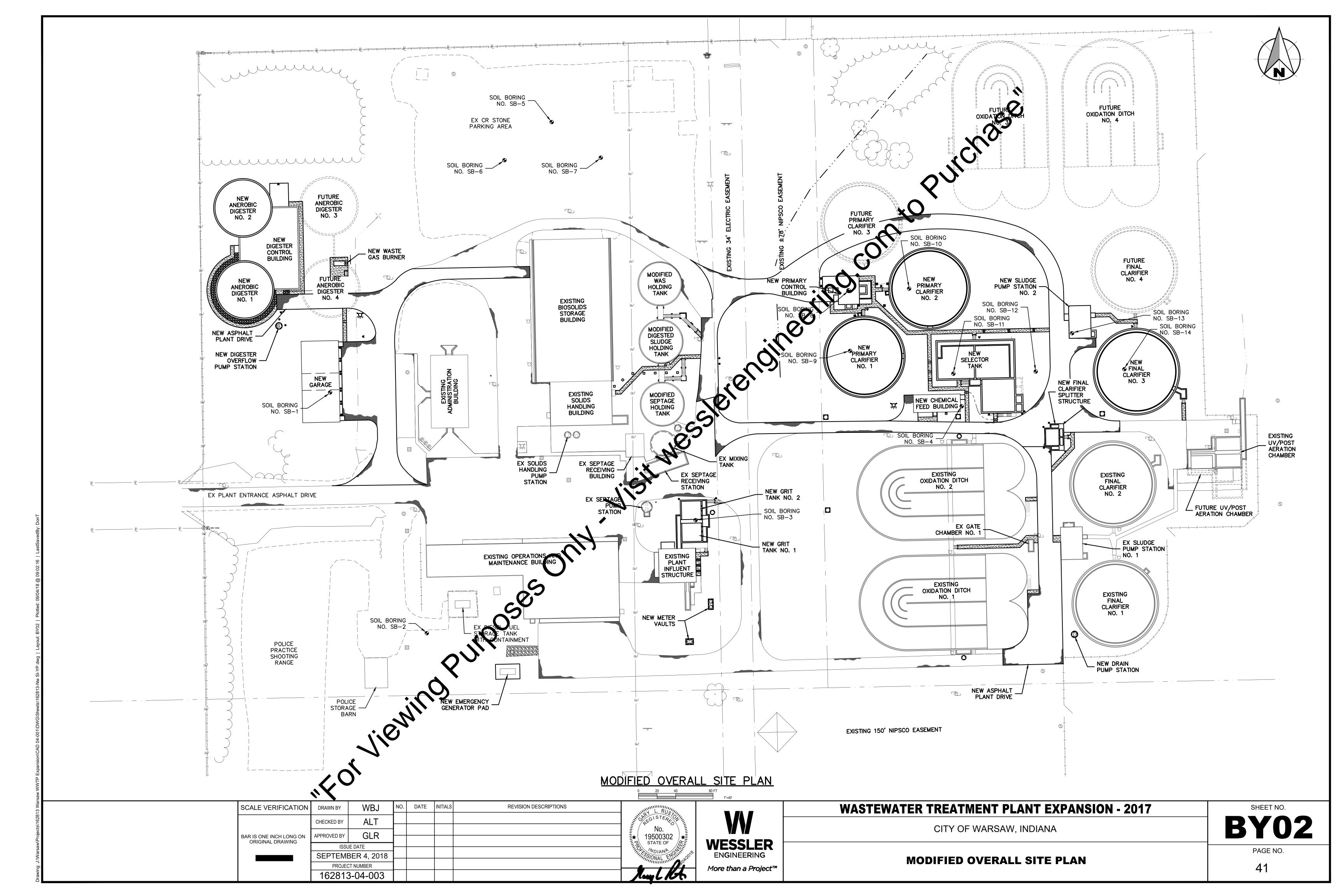


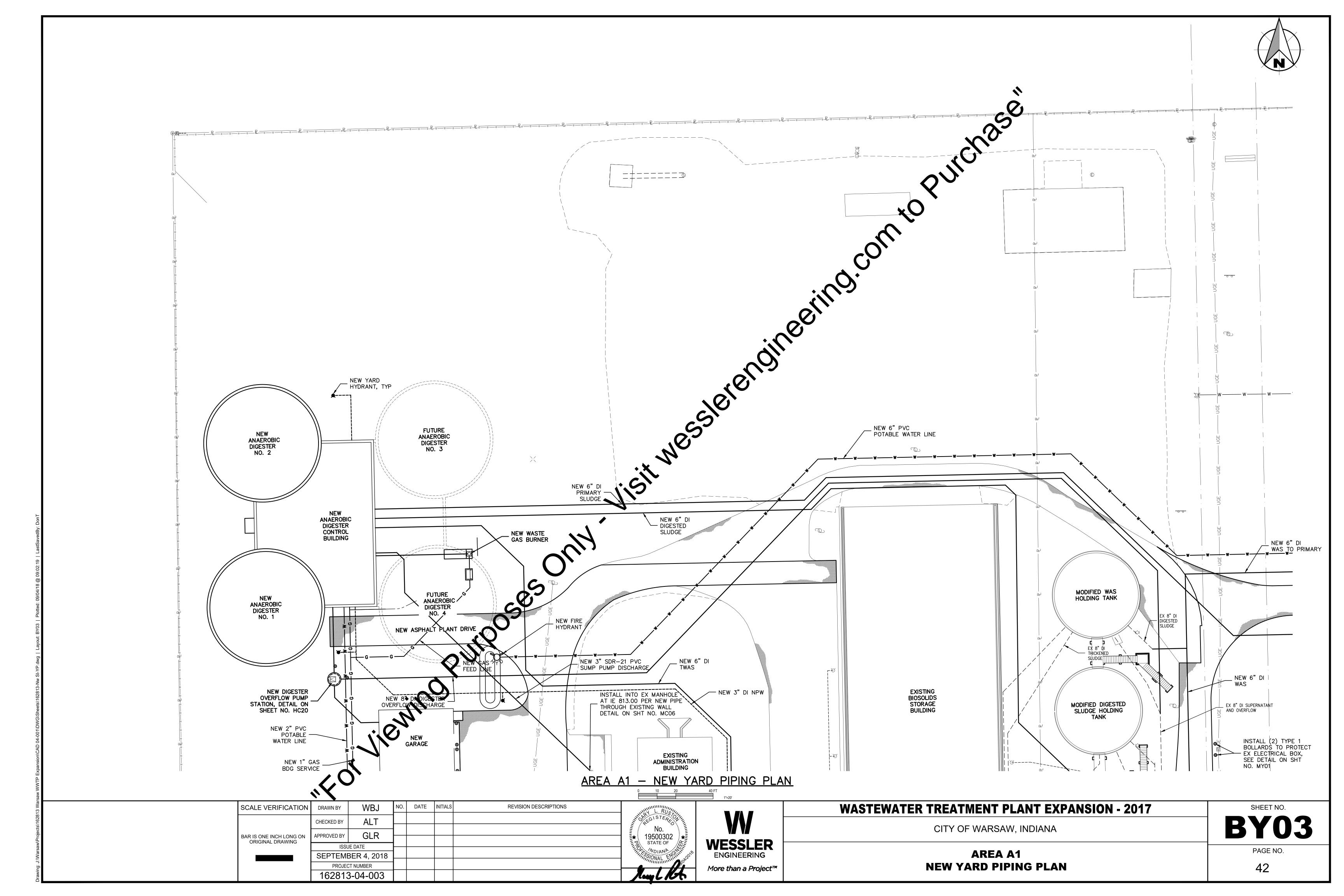


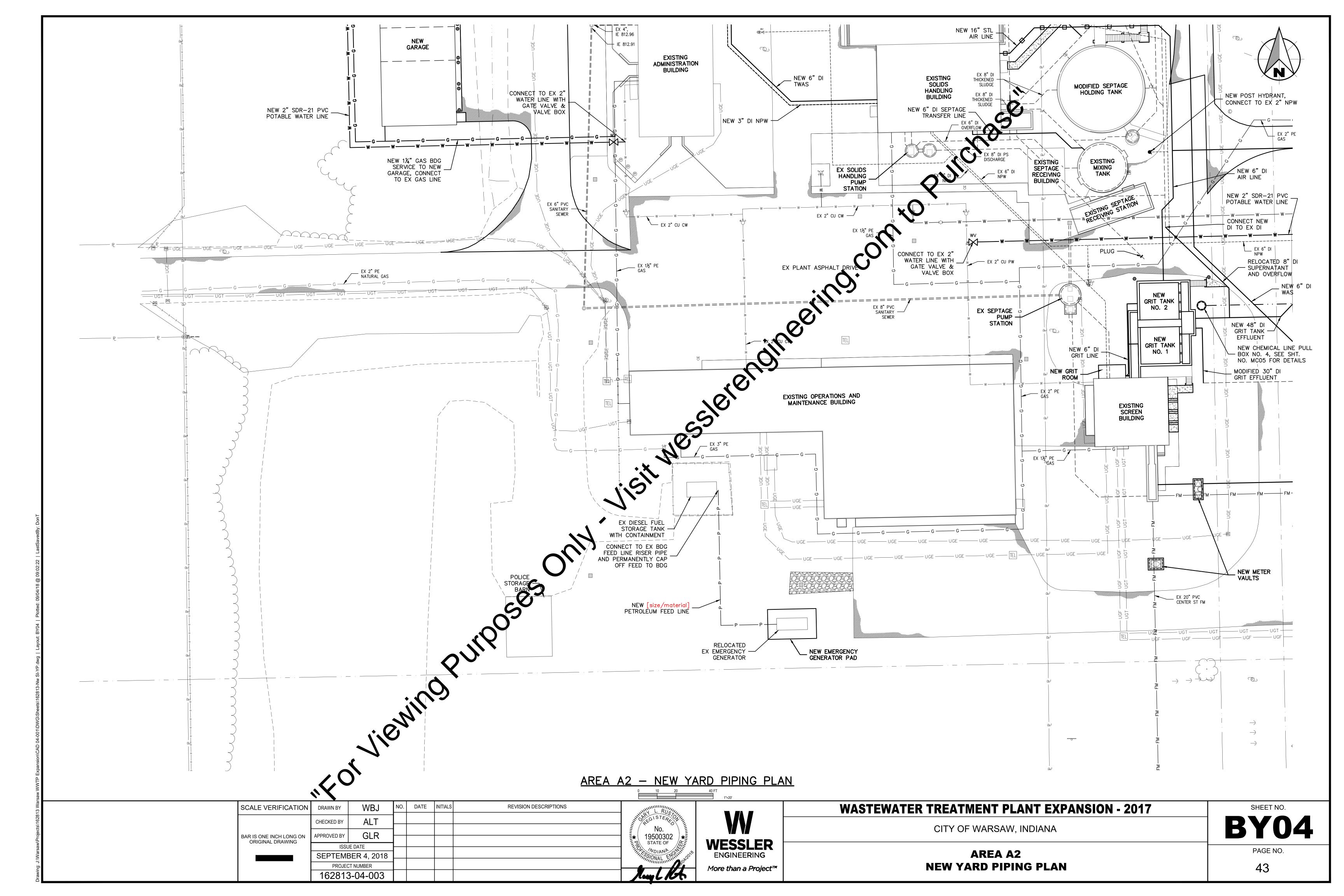


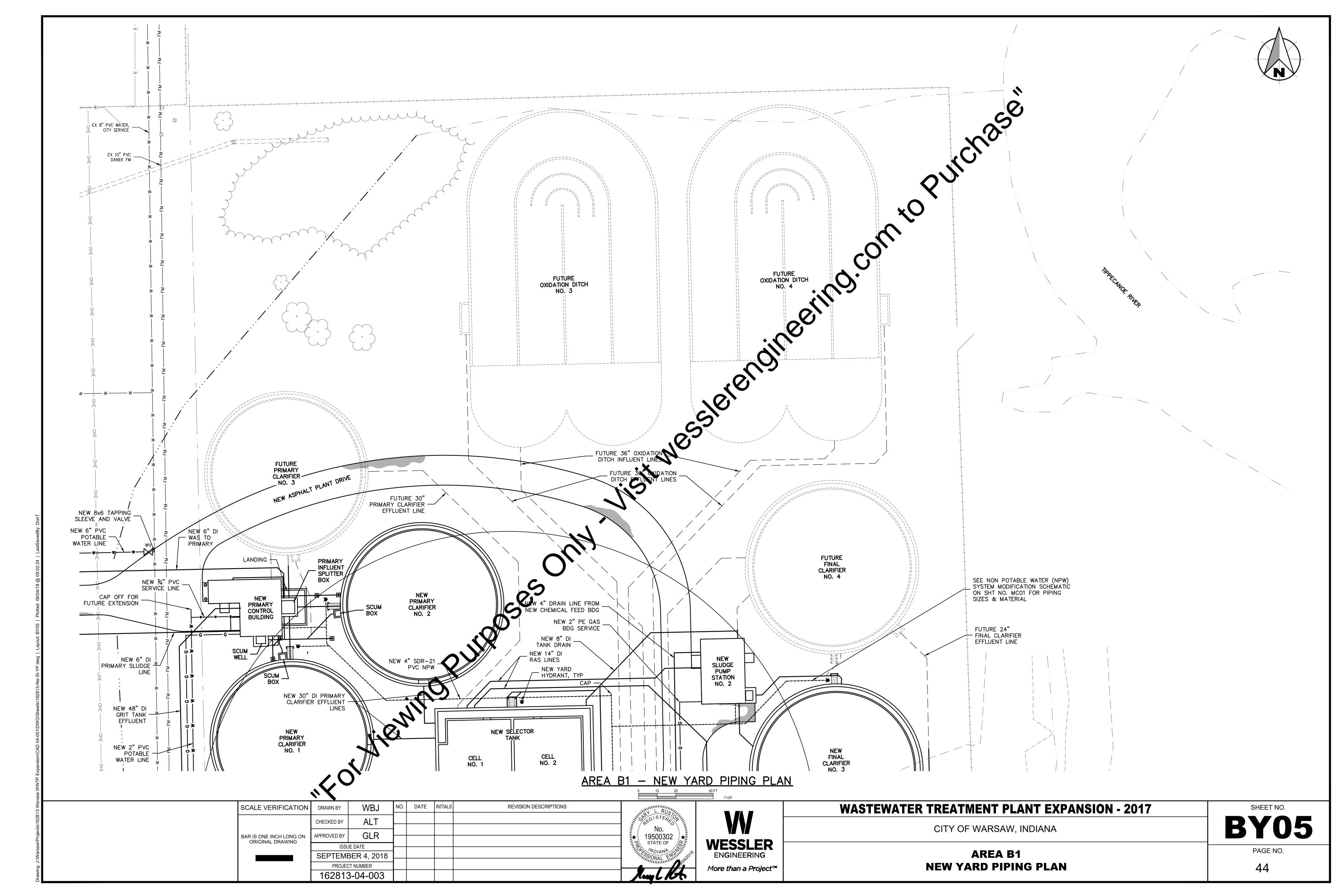


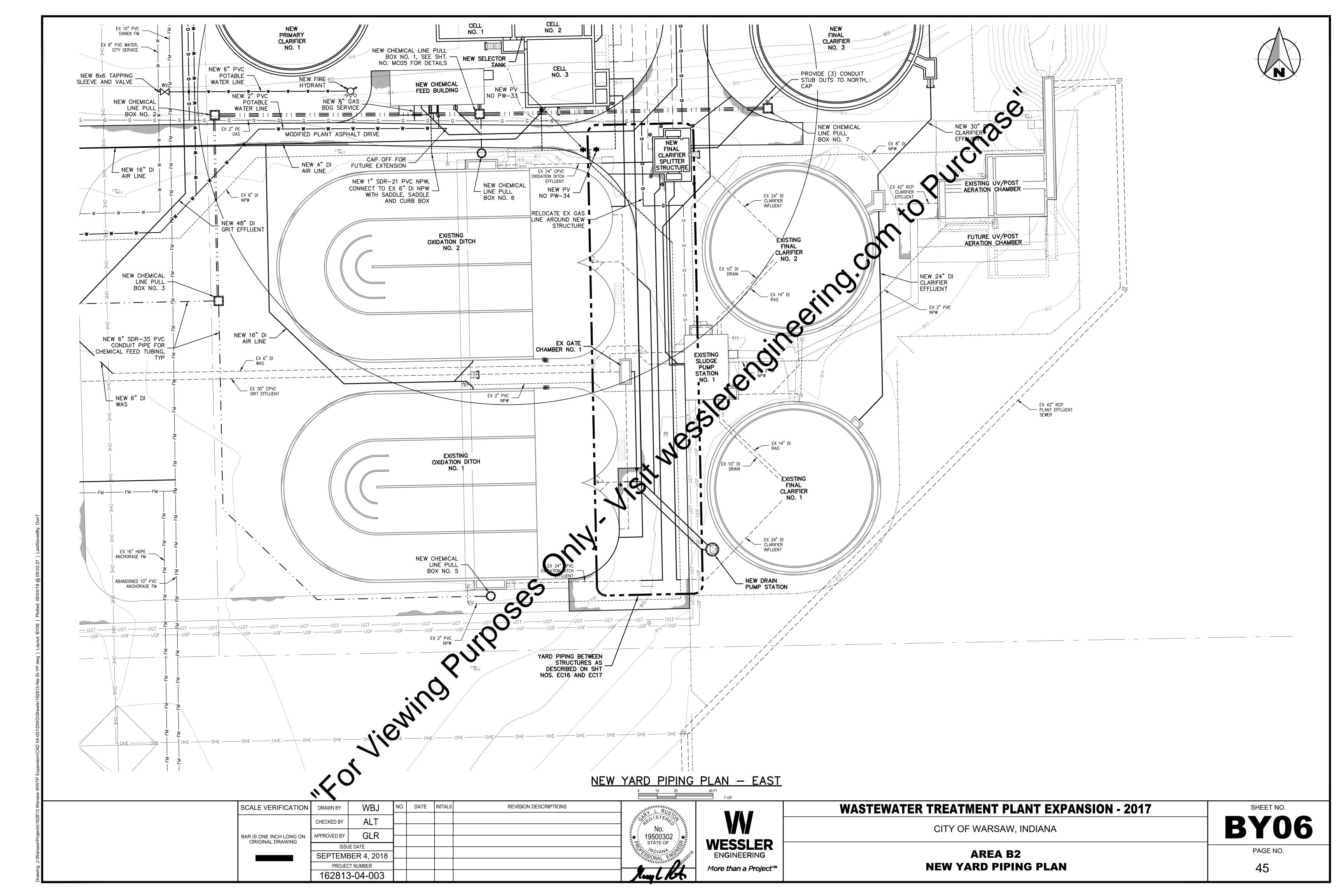


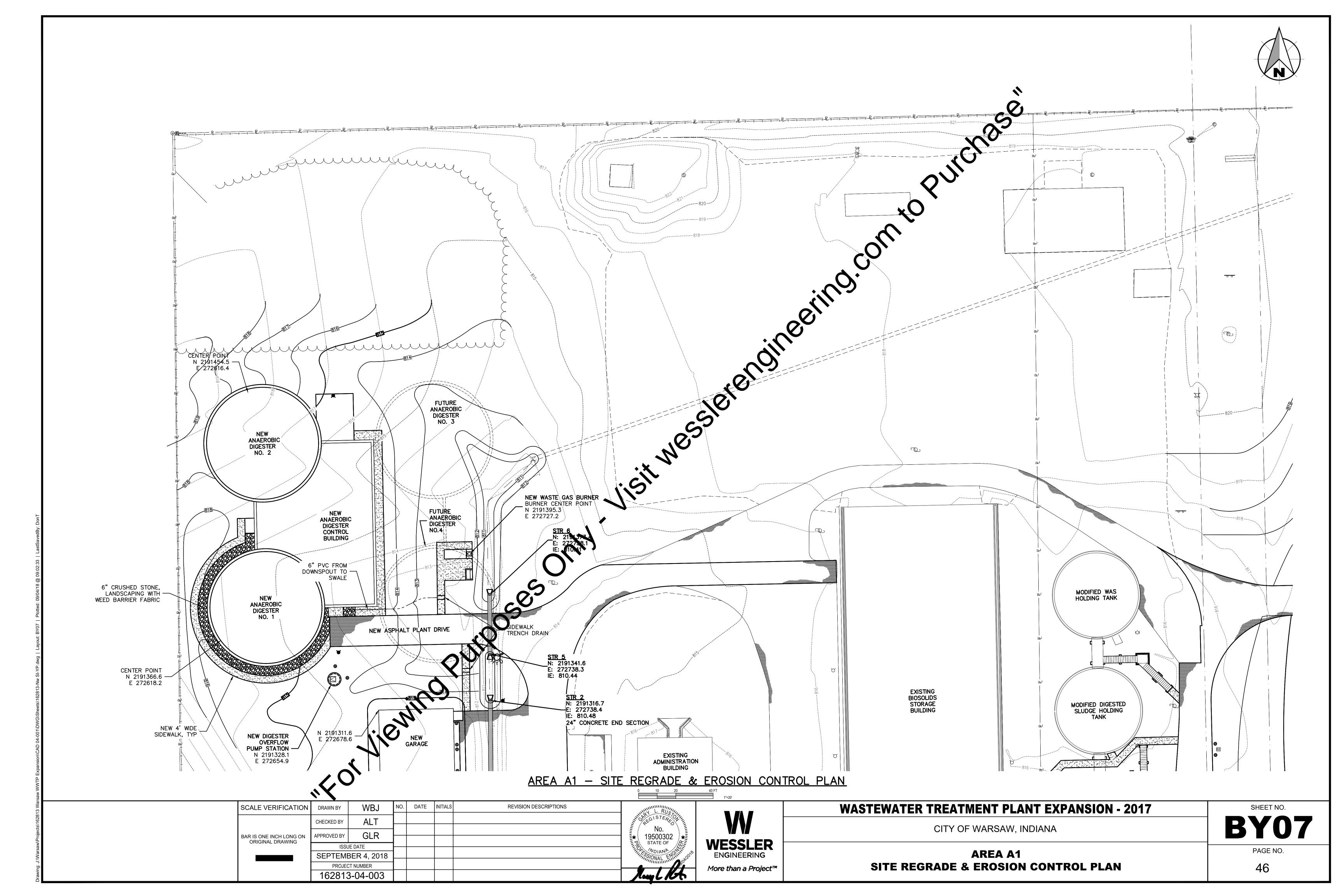


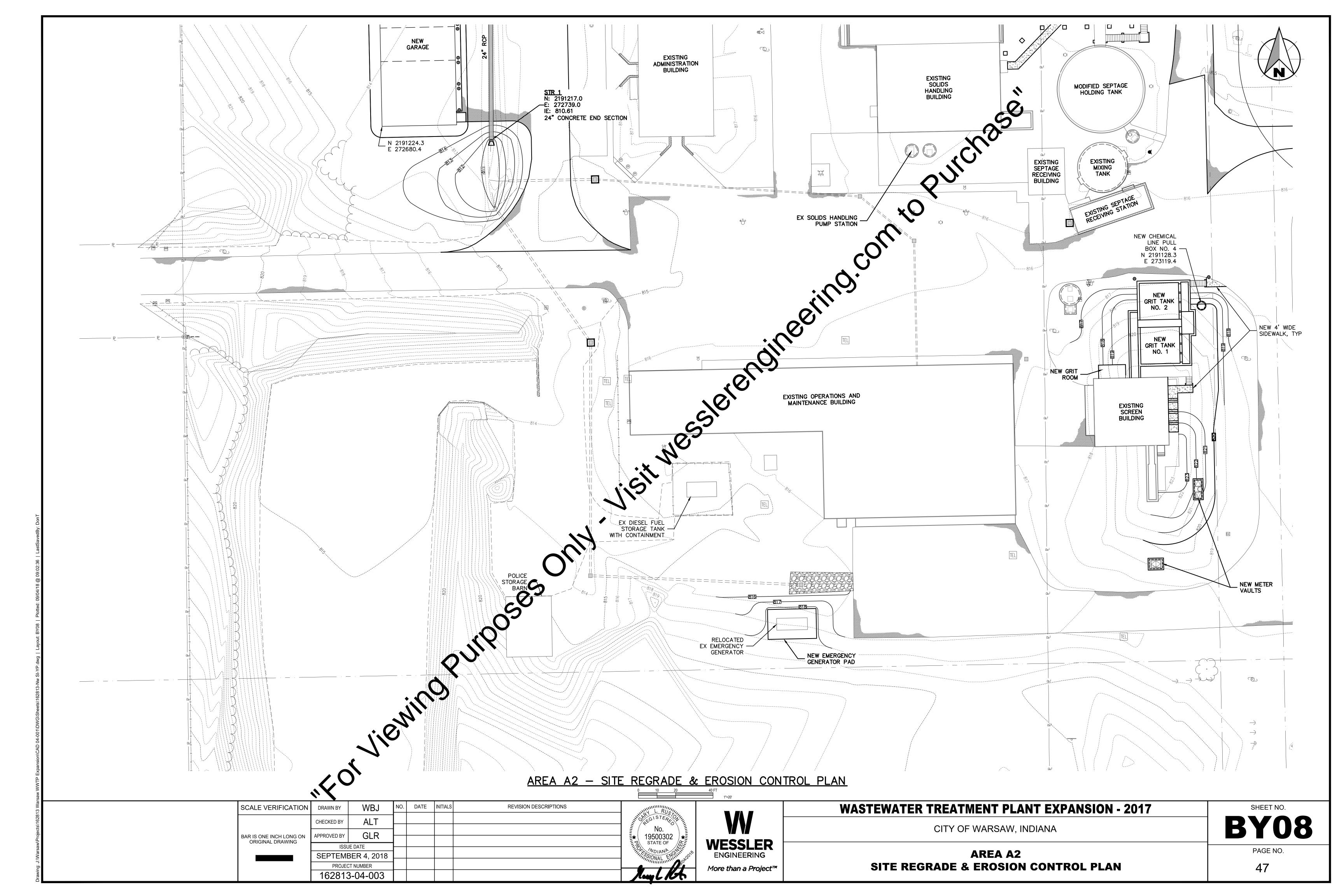


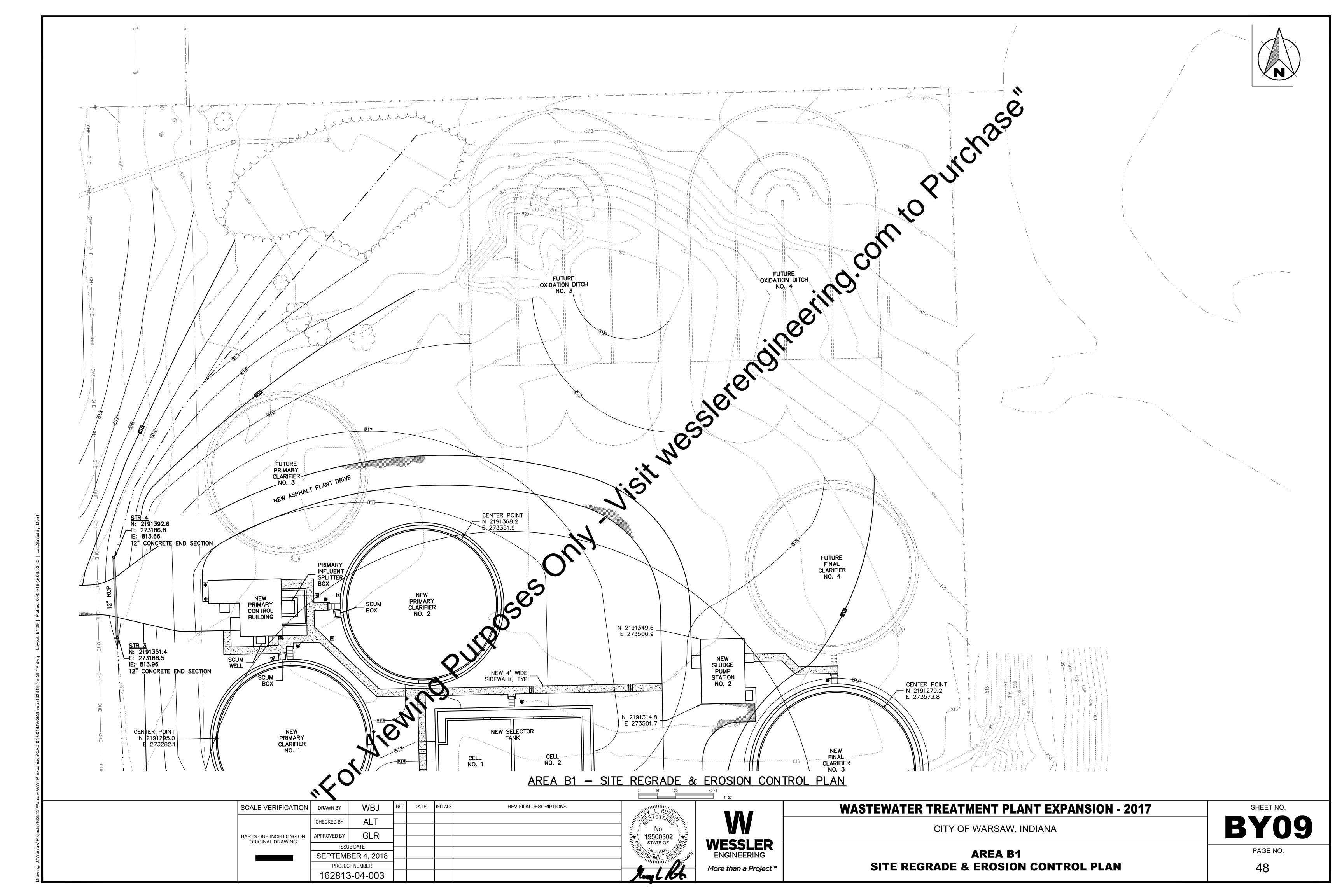


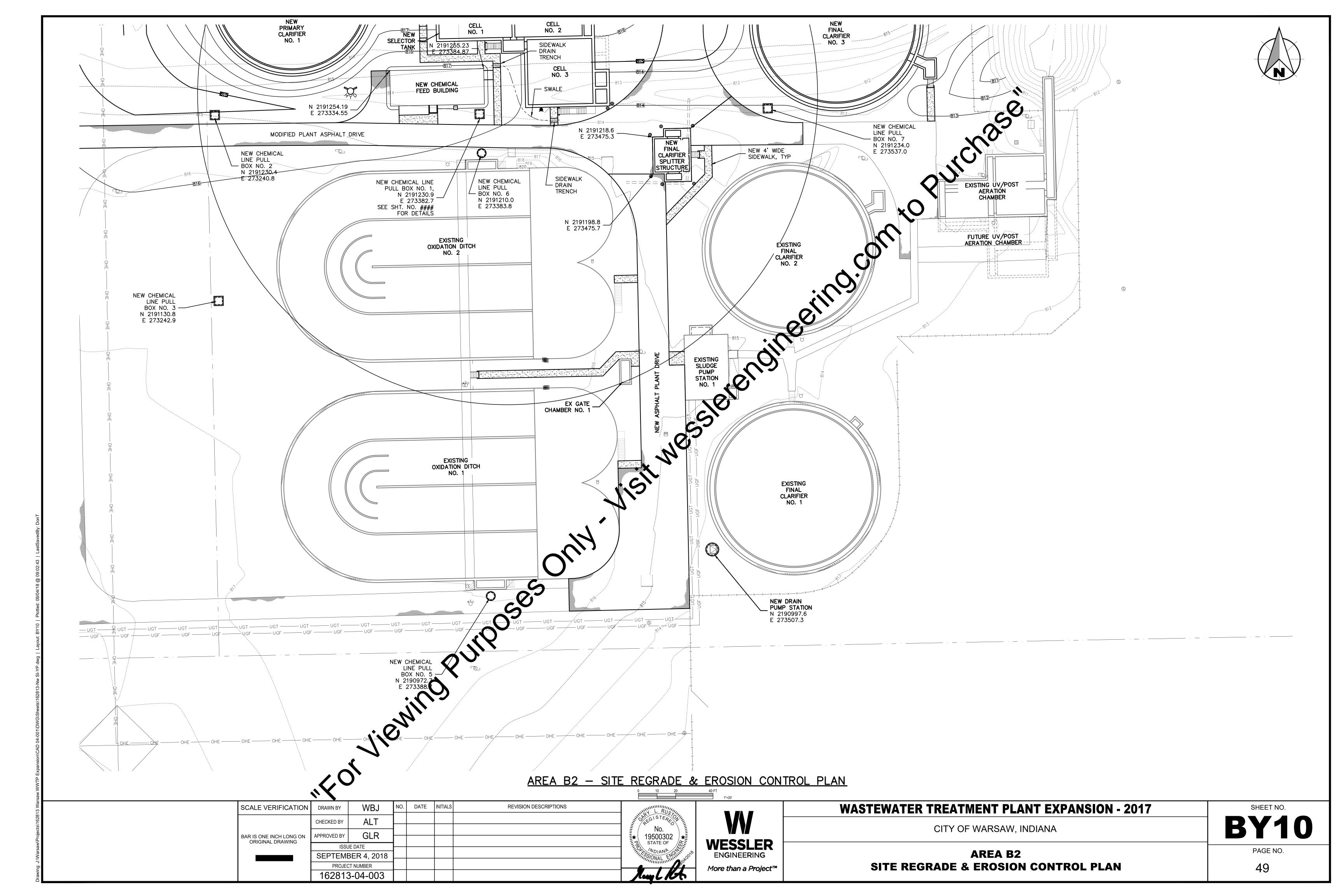


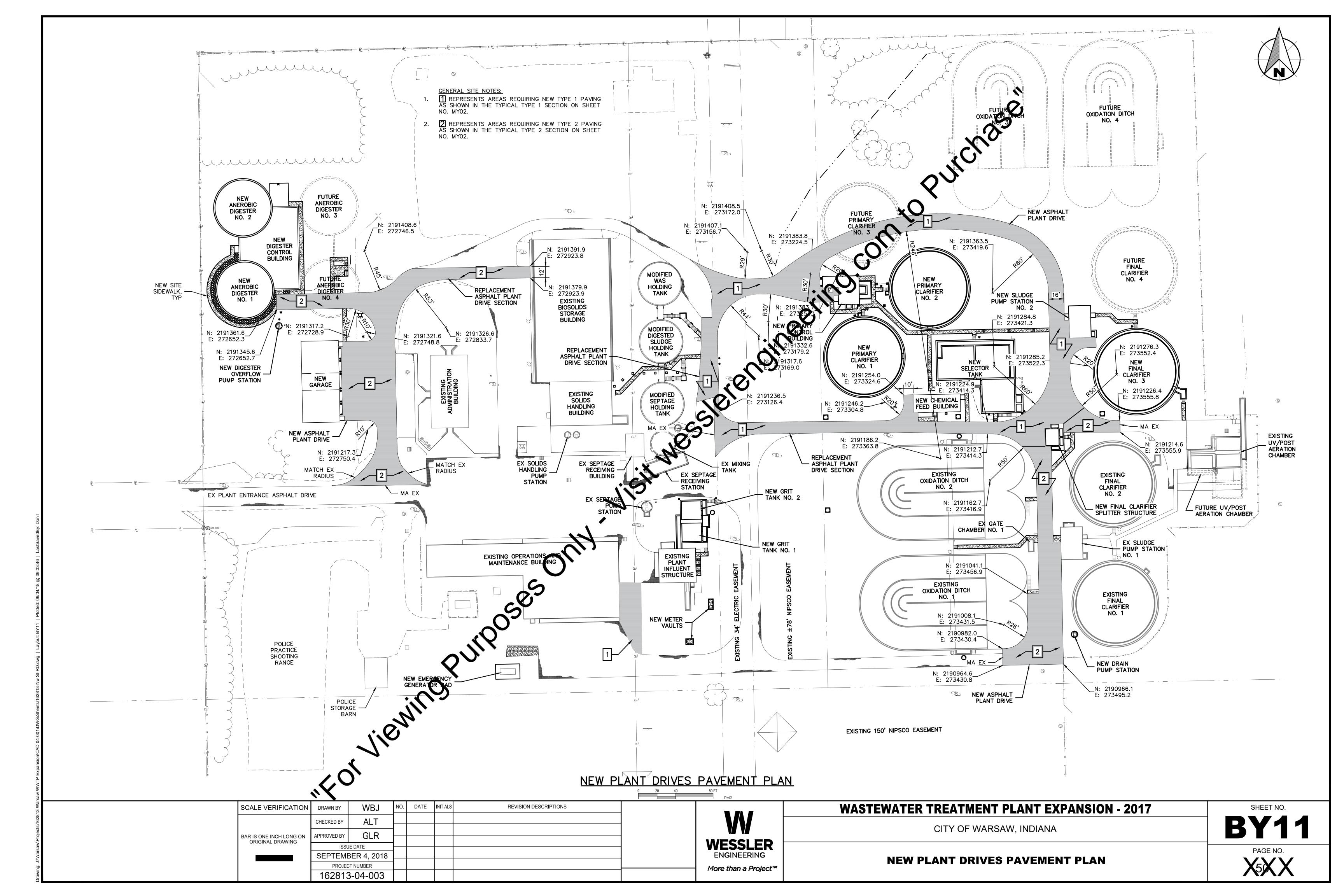


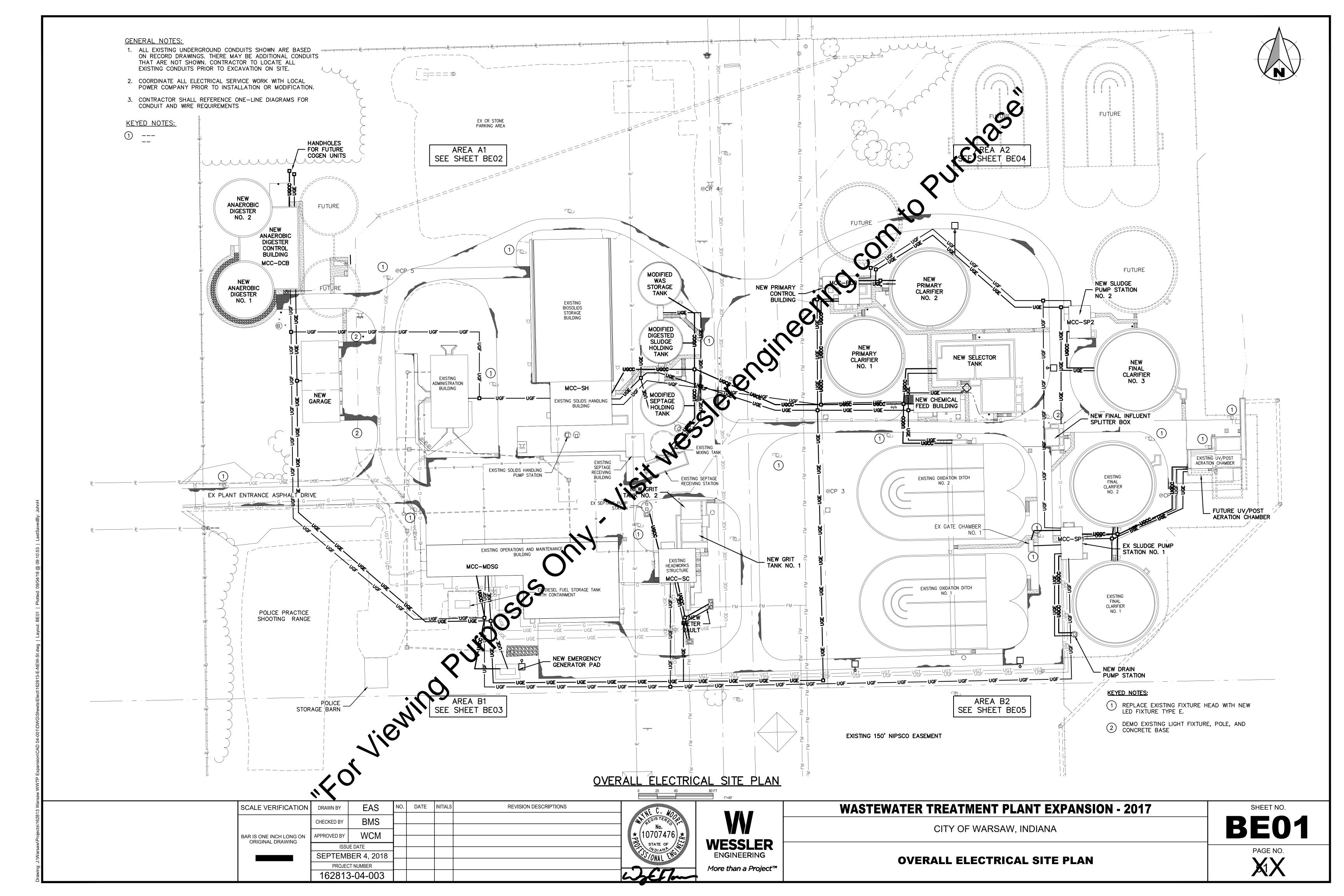


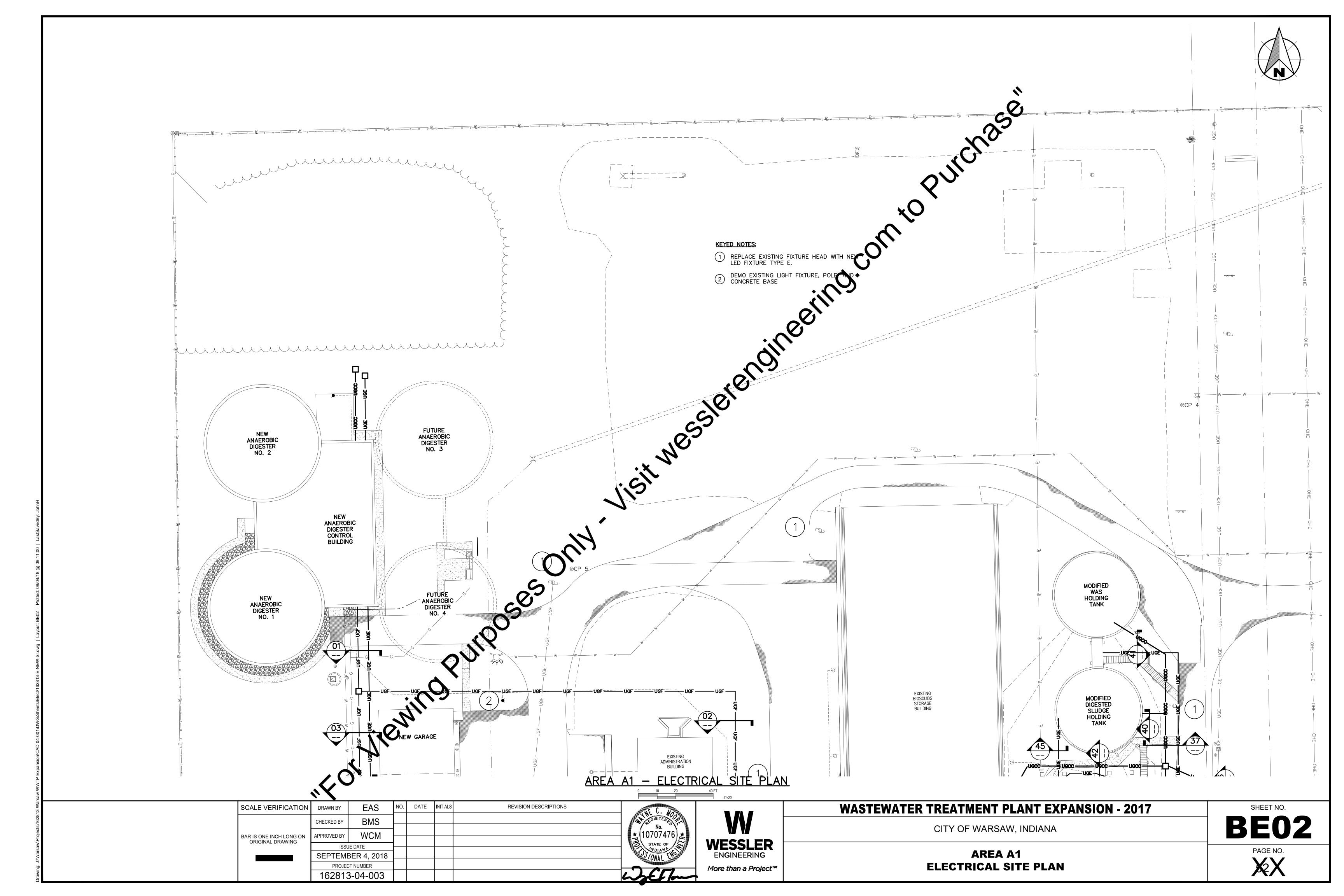


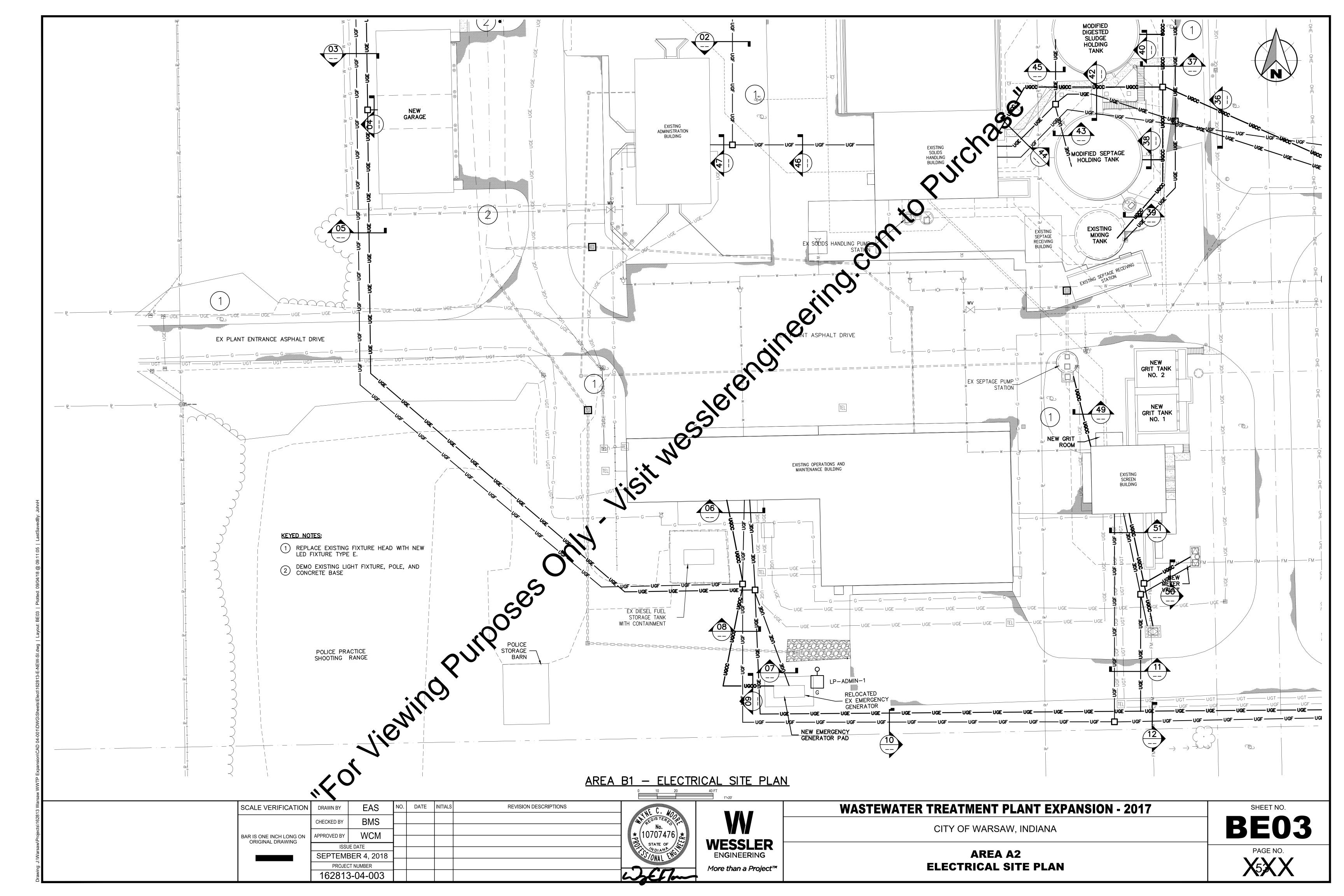


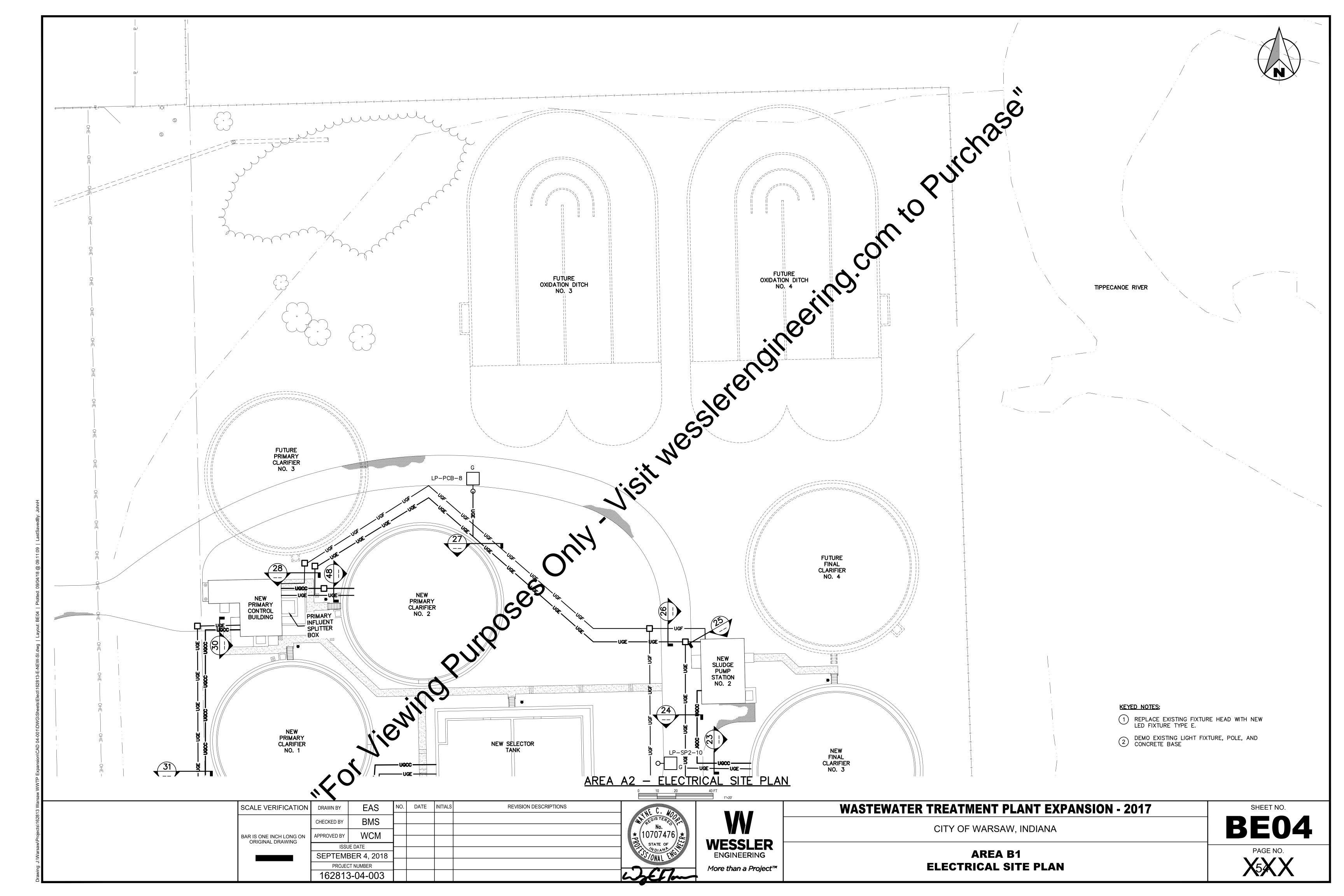


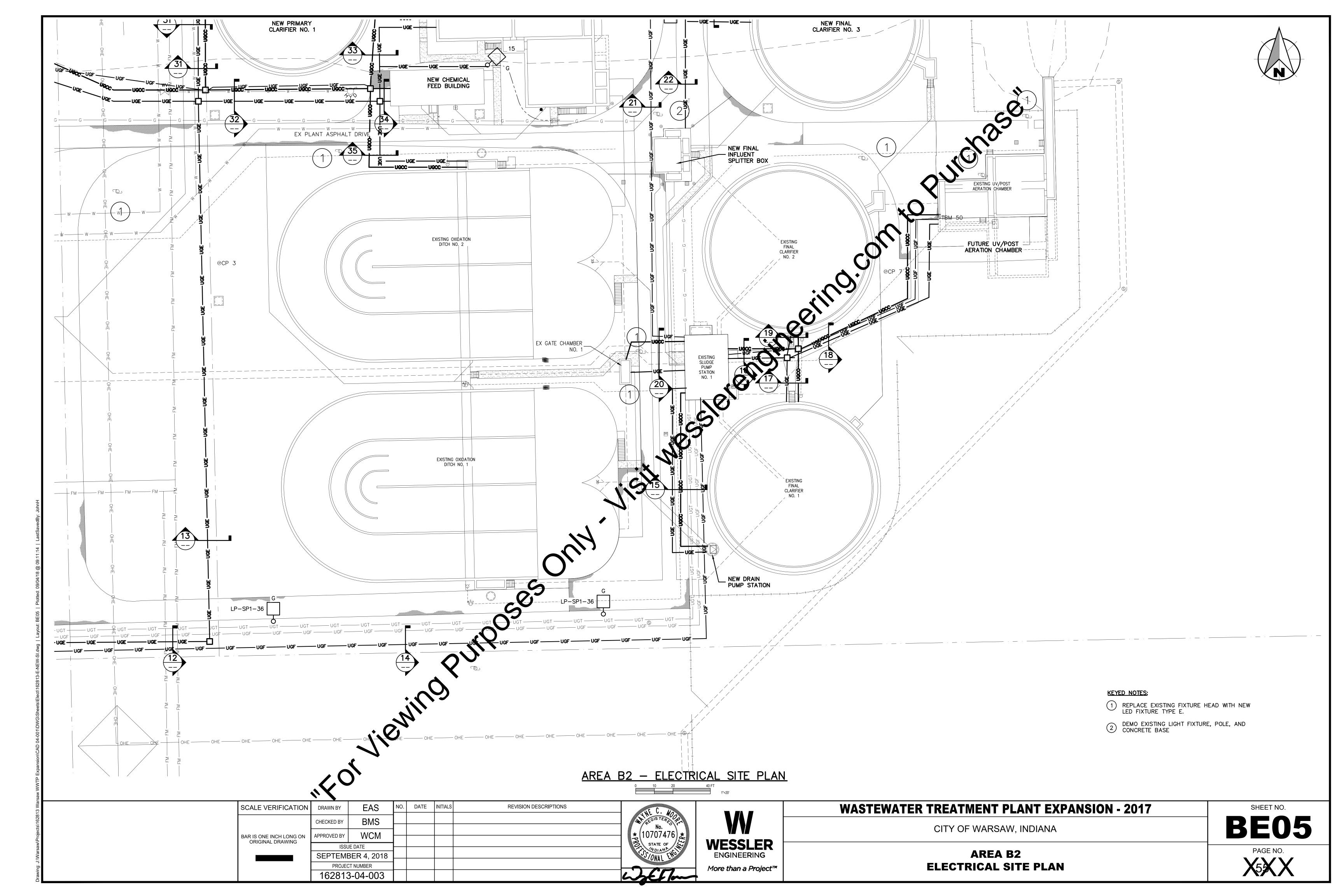


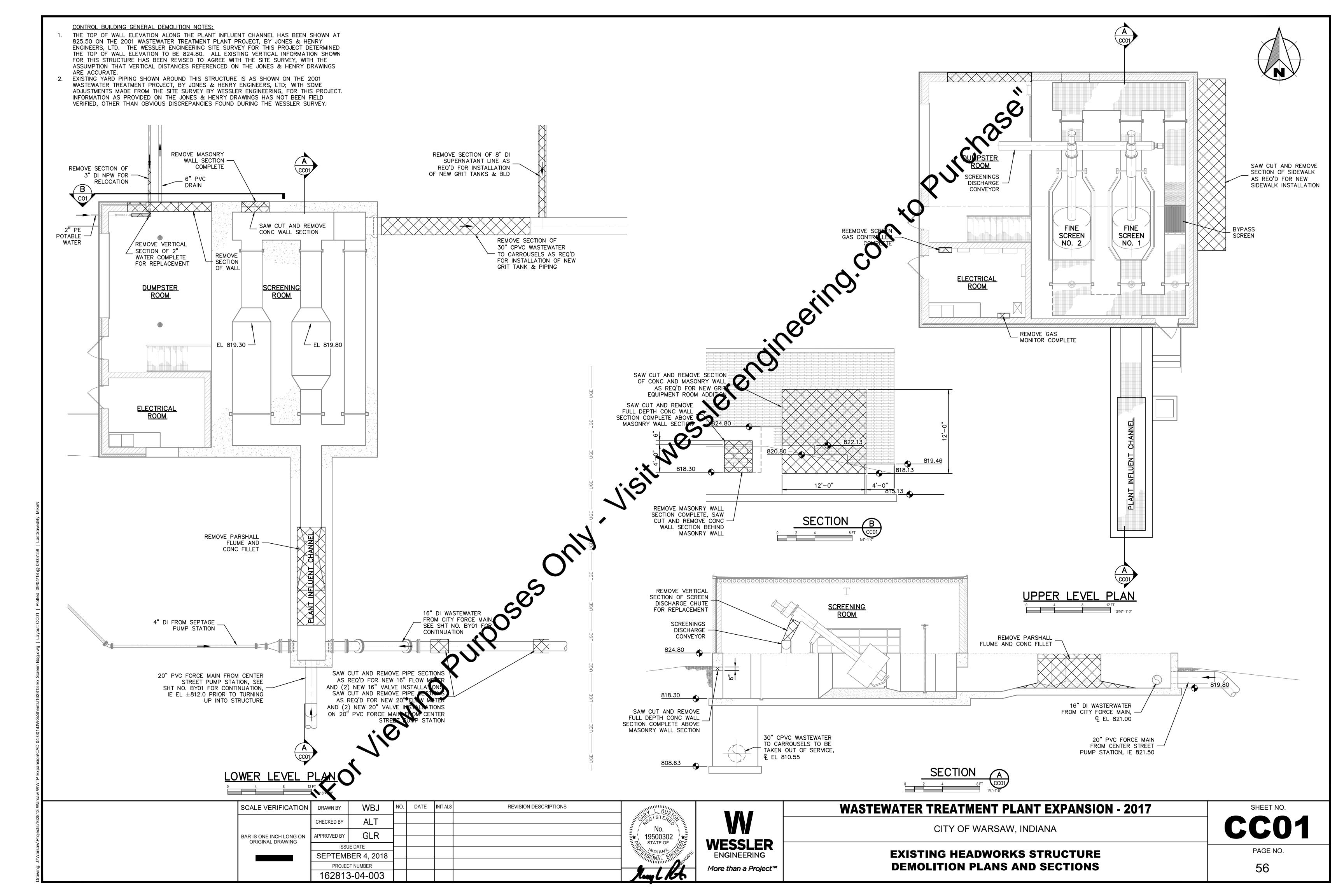


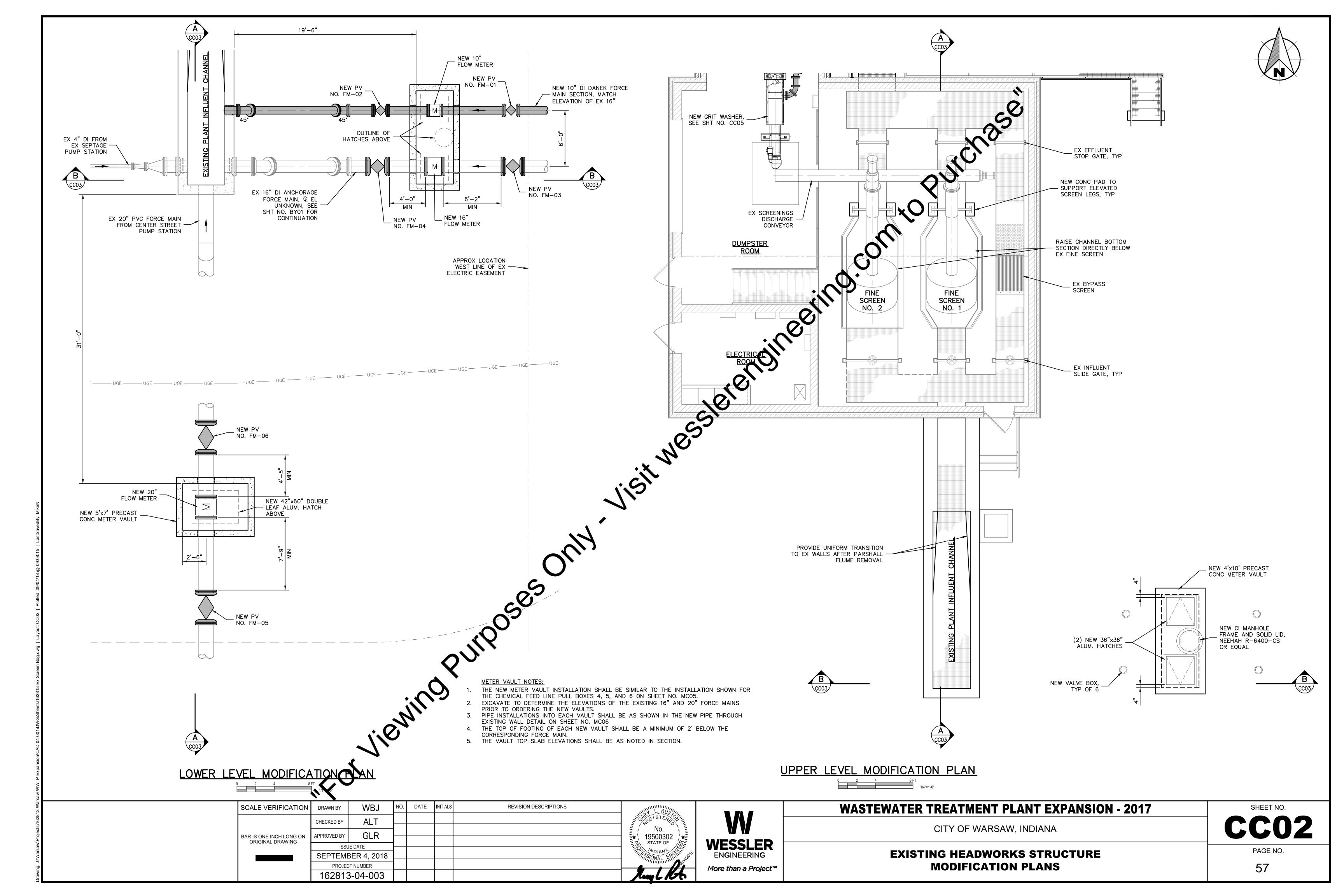


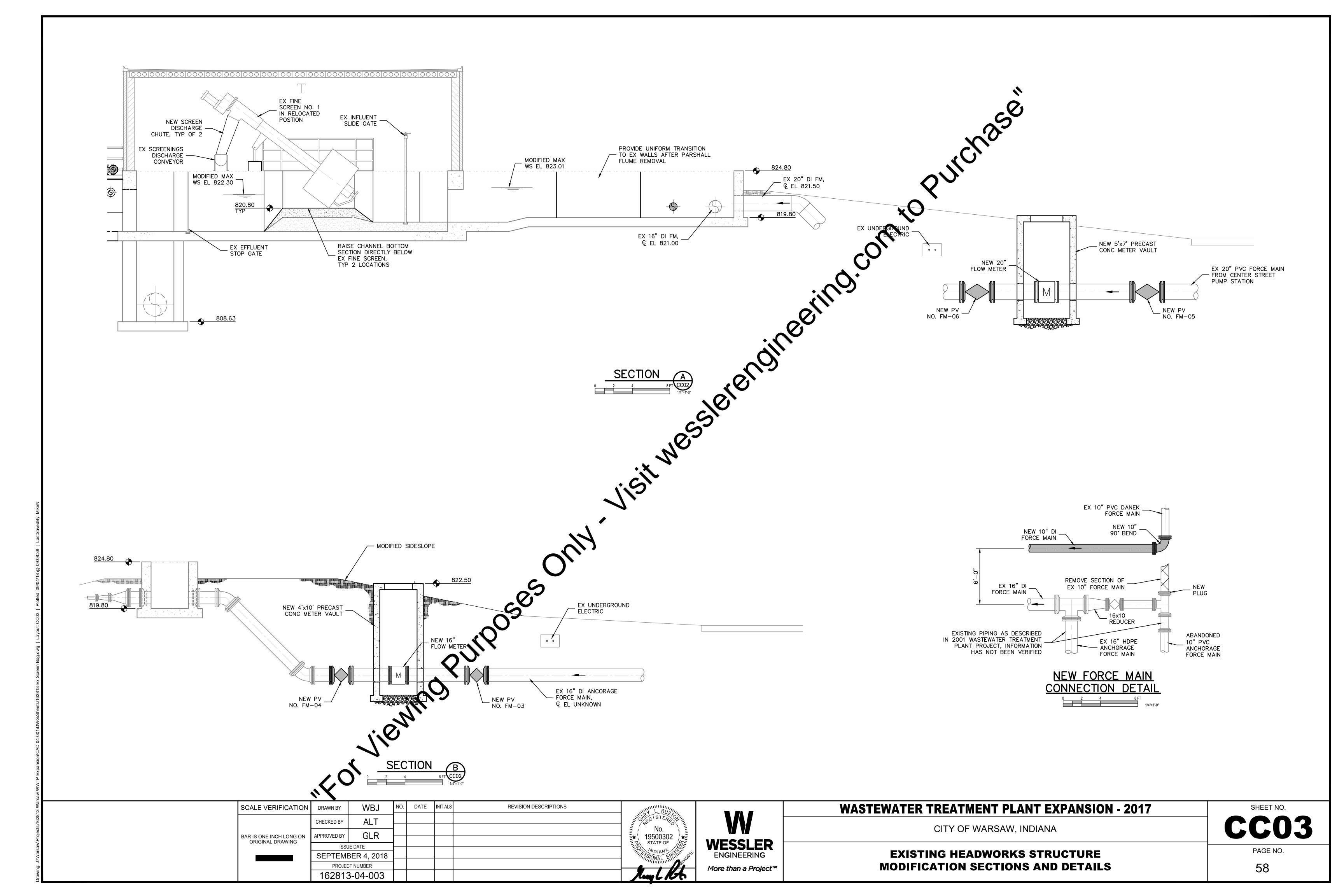


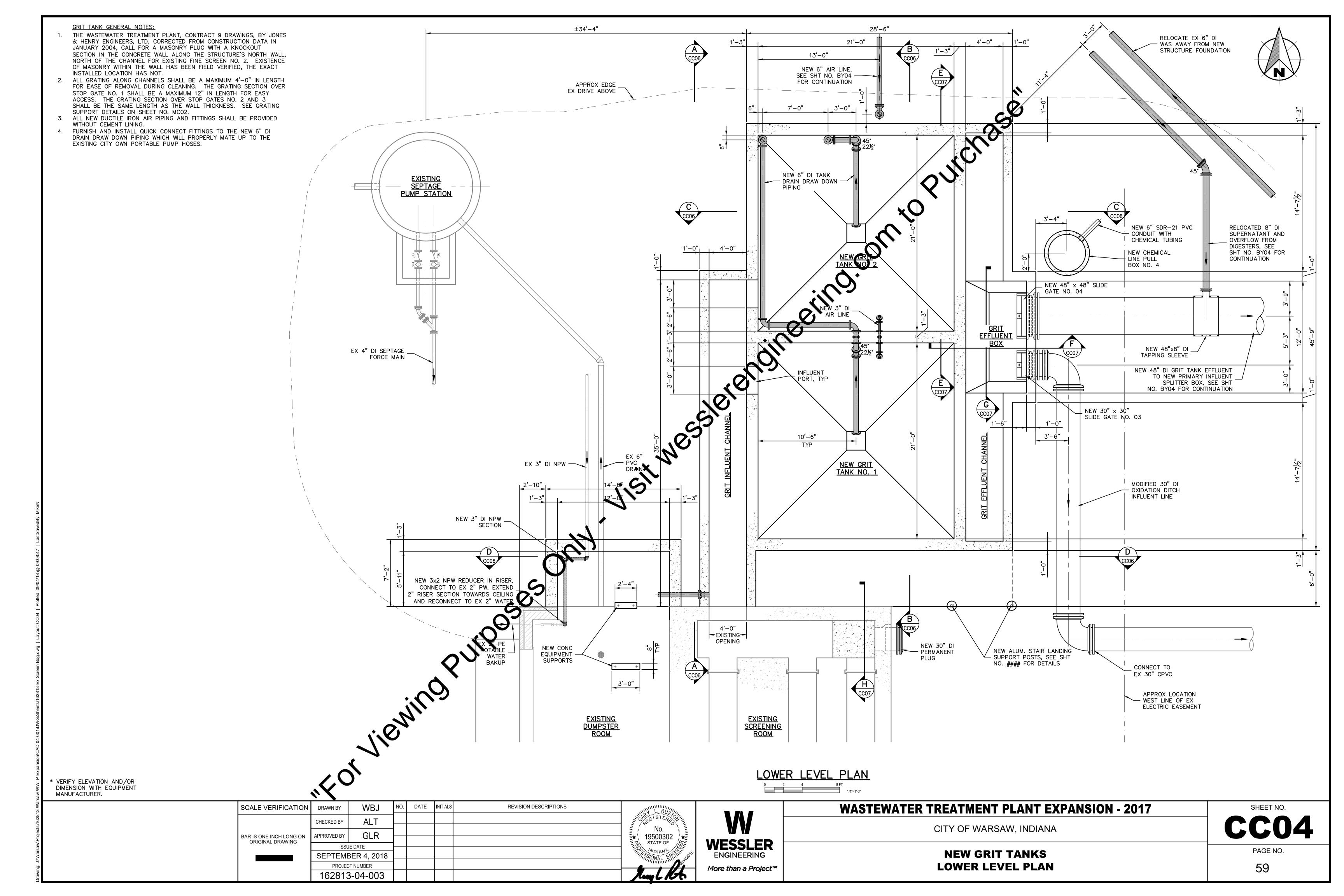


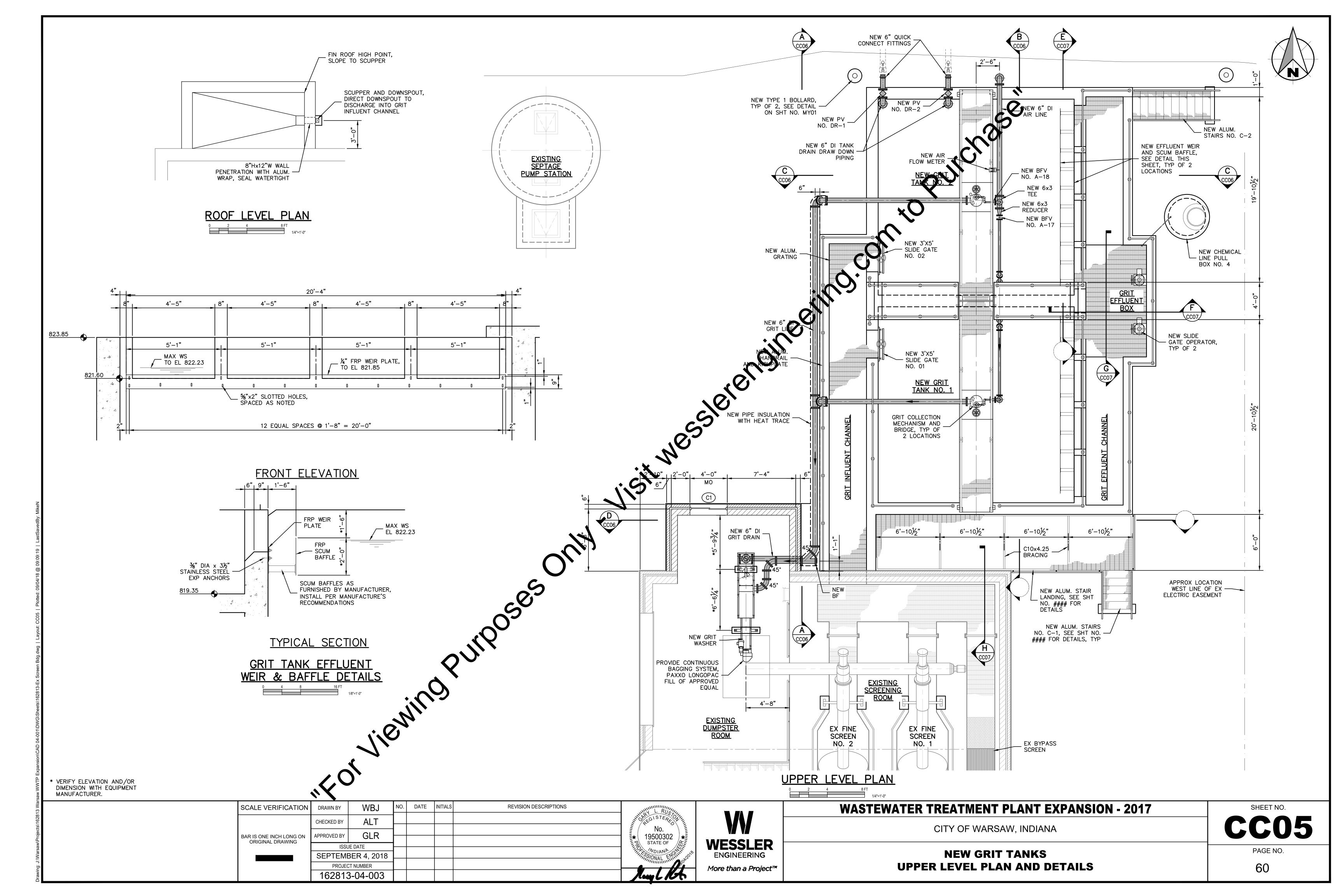


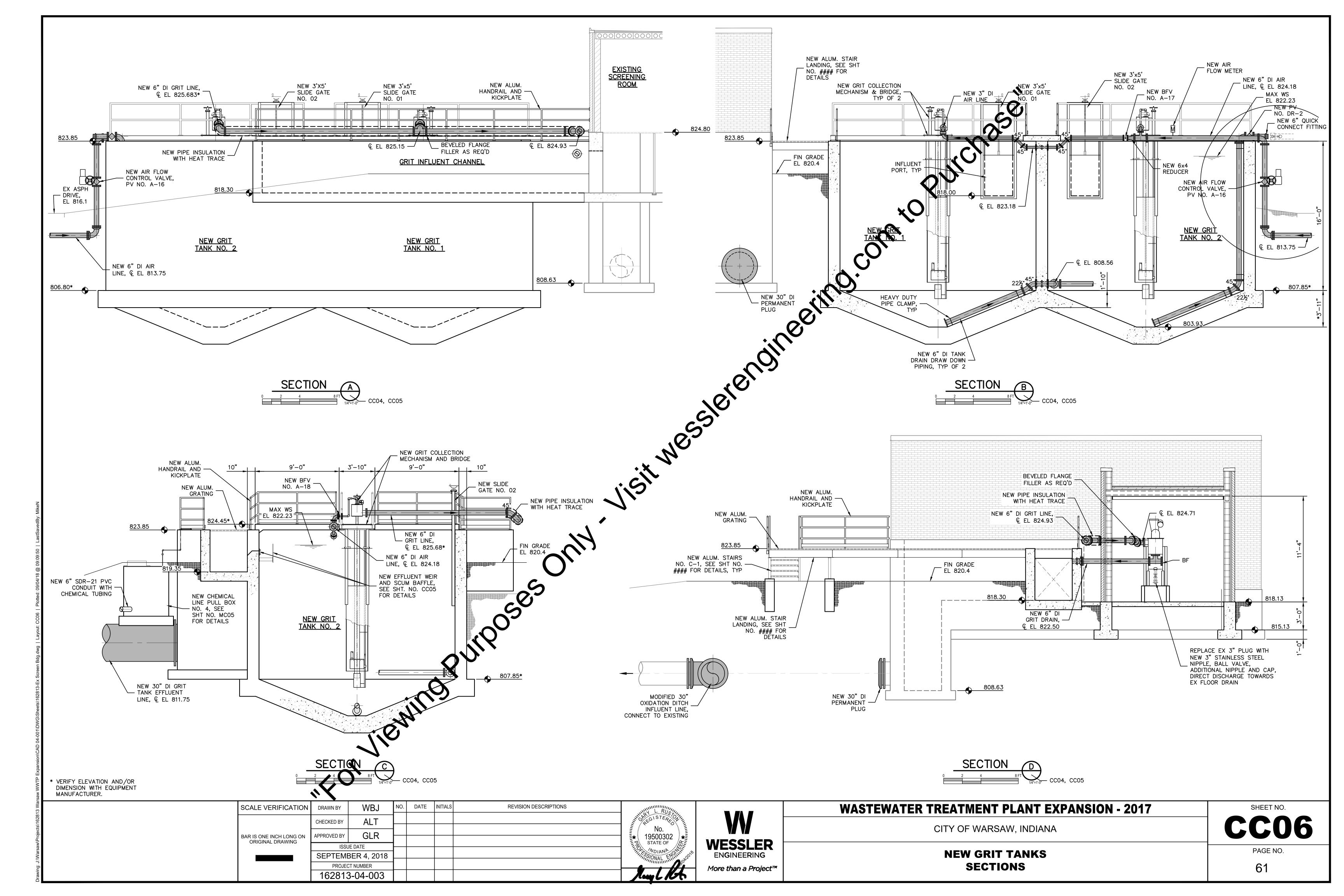


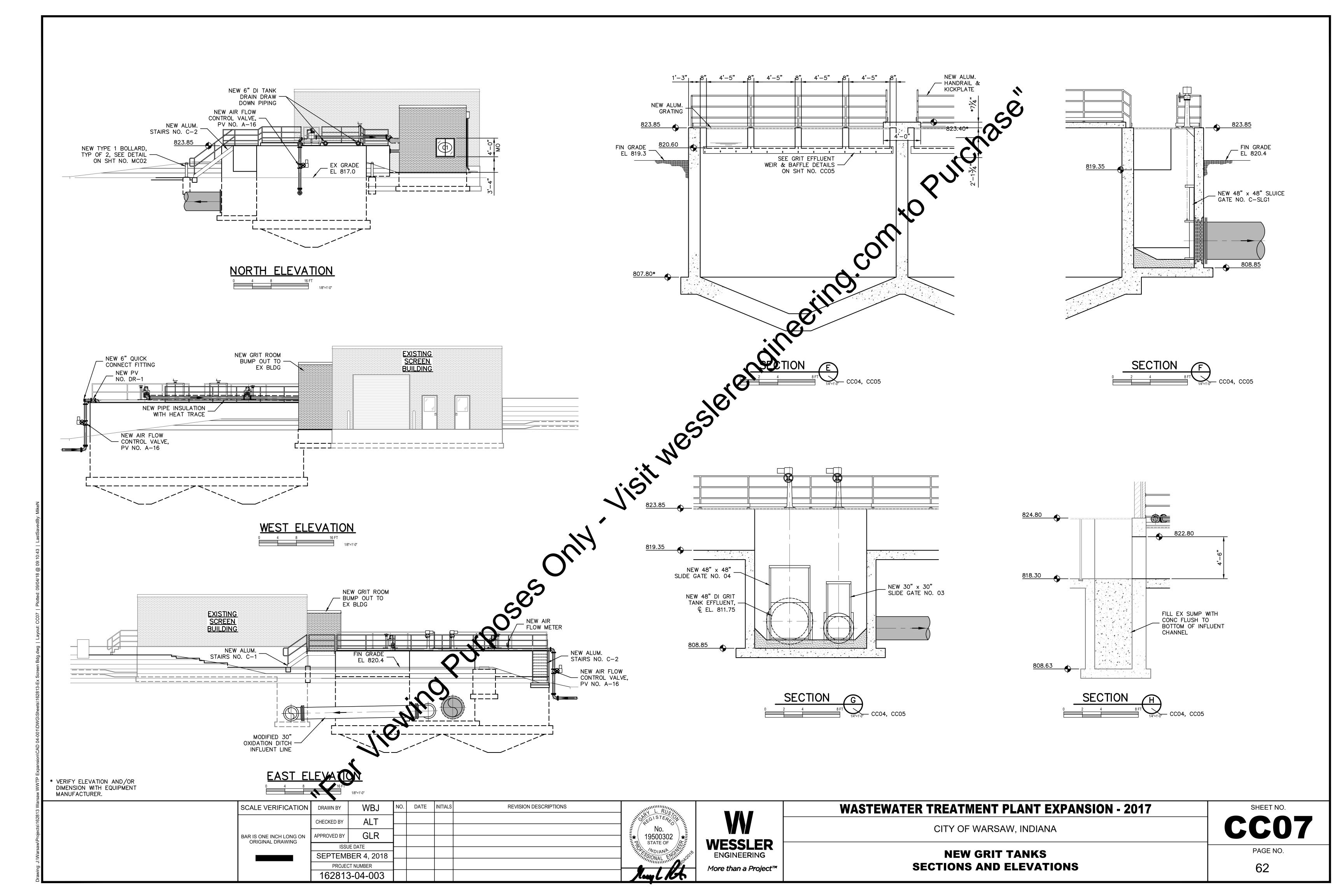


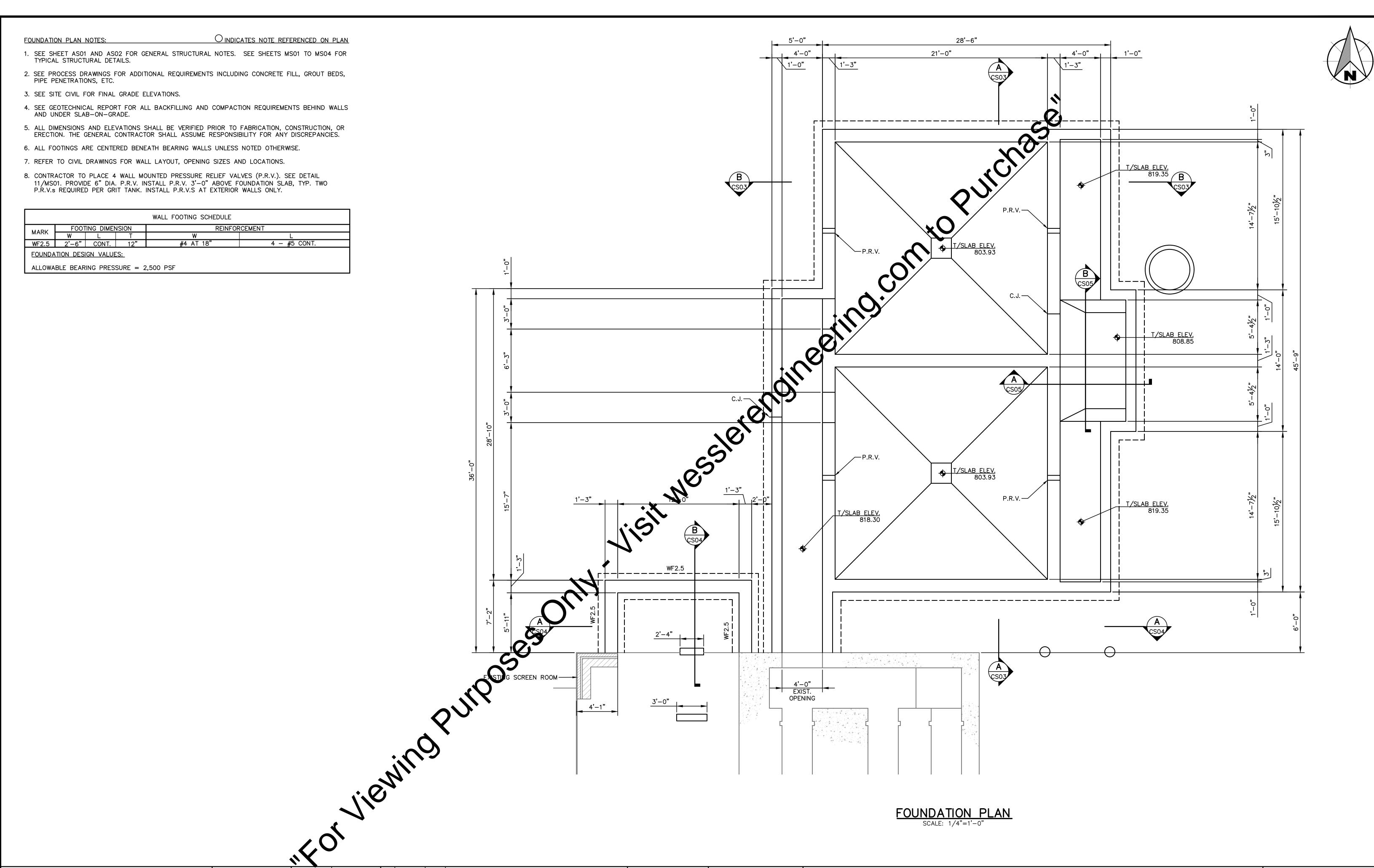












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NOIANA CEB BAR IS ONE INCH LONG ON ORIGINAL DRAWING APPROVED BY ISSUE DATE SEPTEMBER 4, 2018 PROJECT NUMBER Carle 162813-04-003



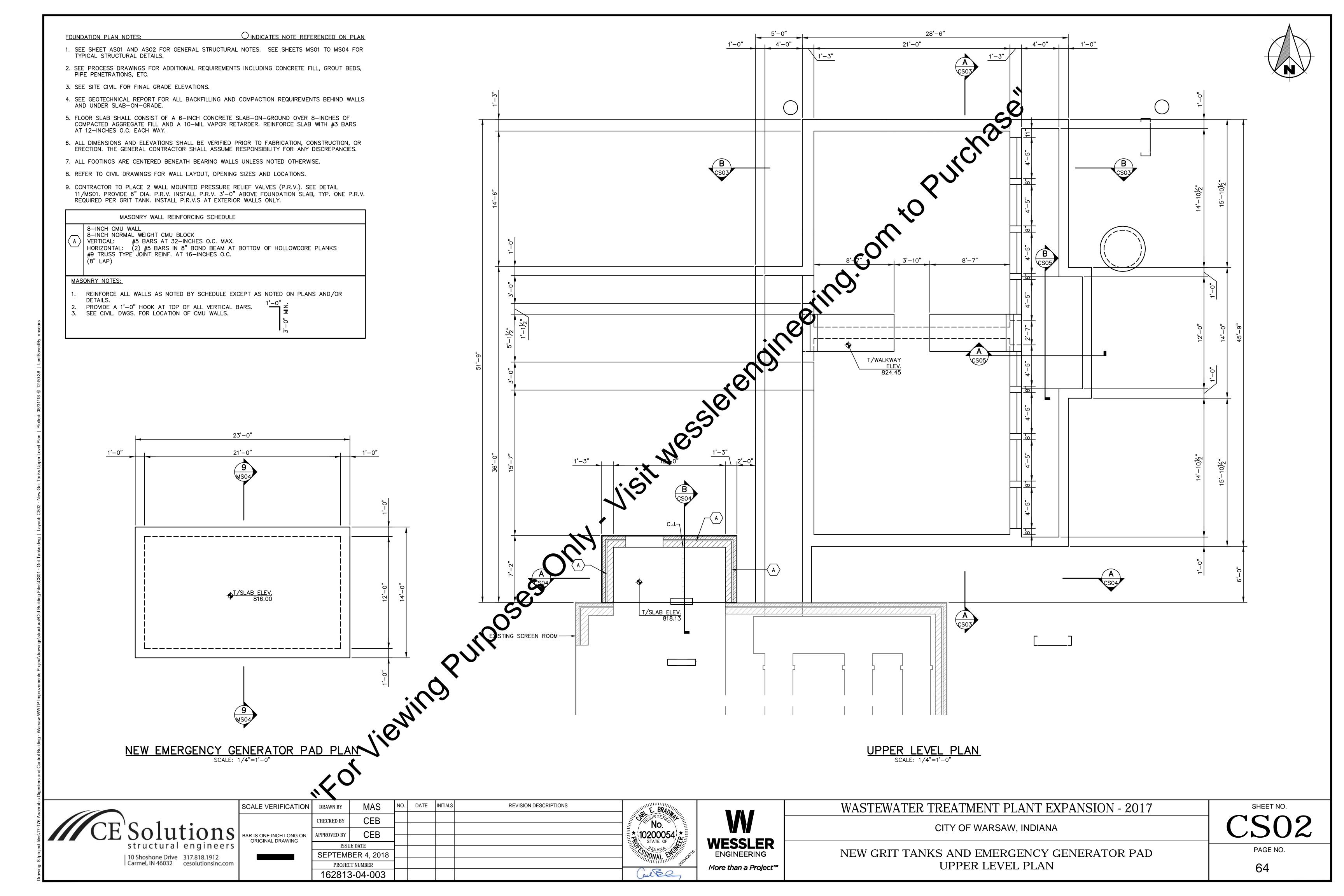
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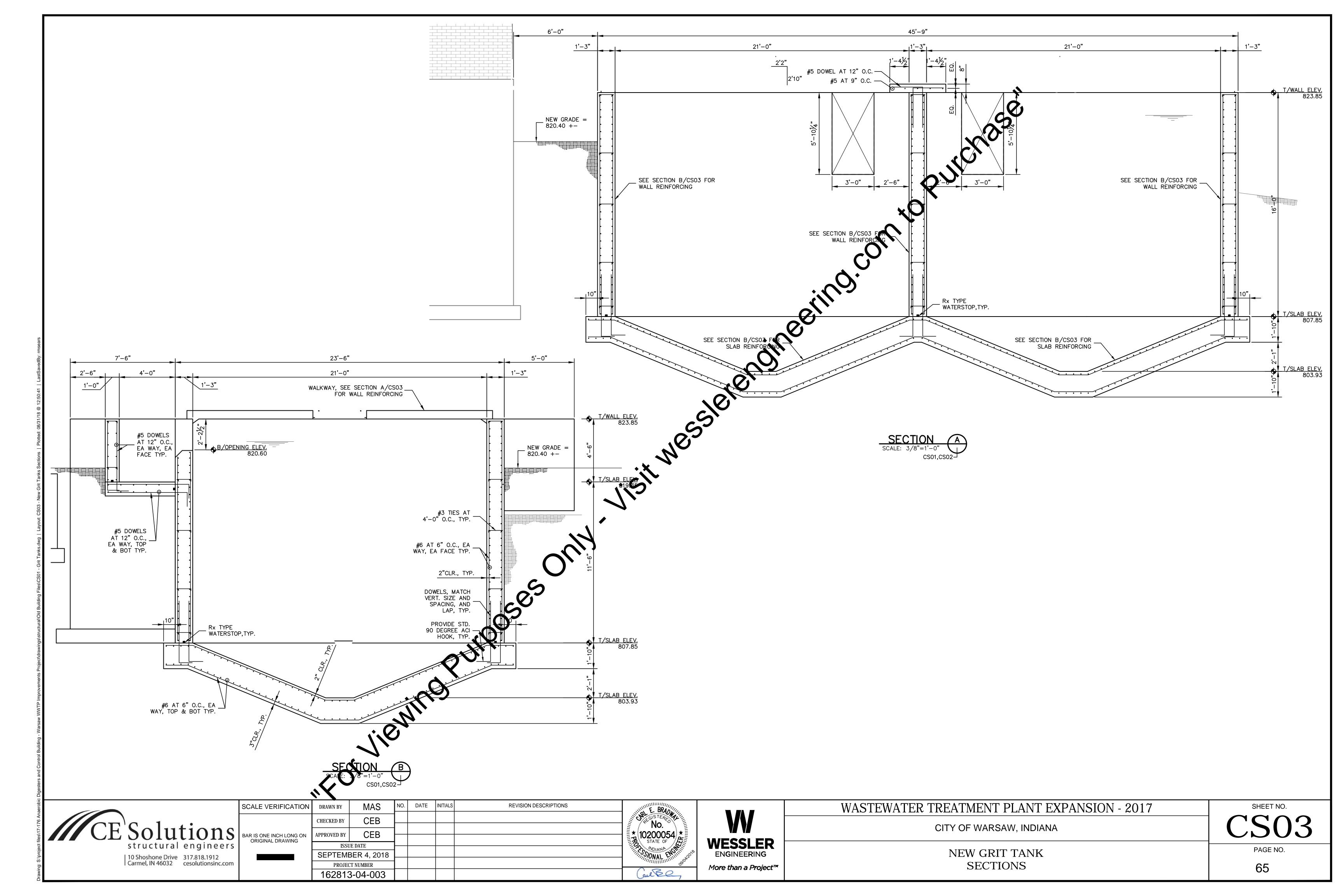
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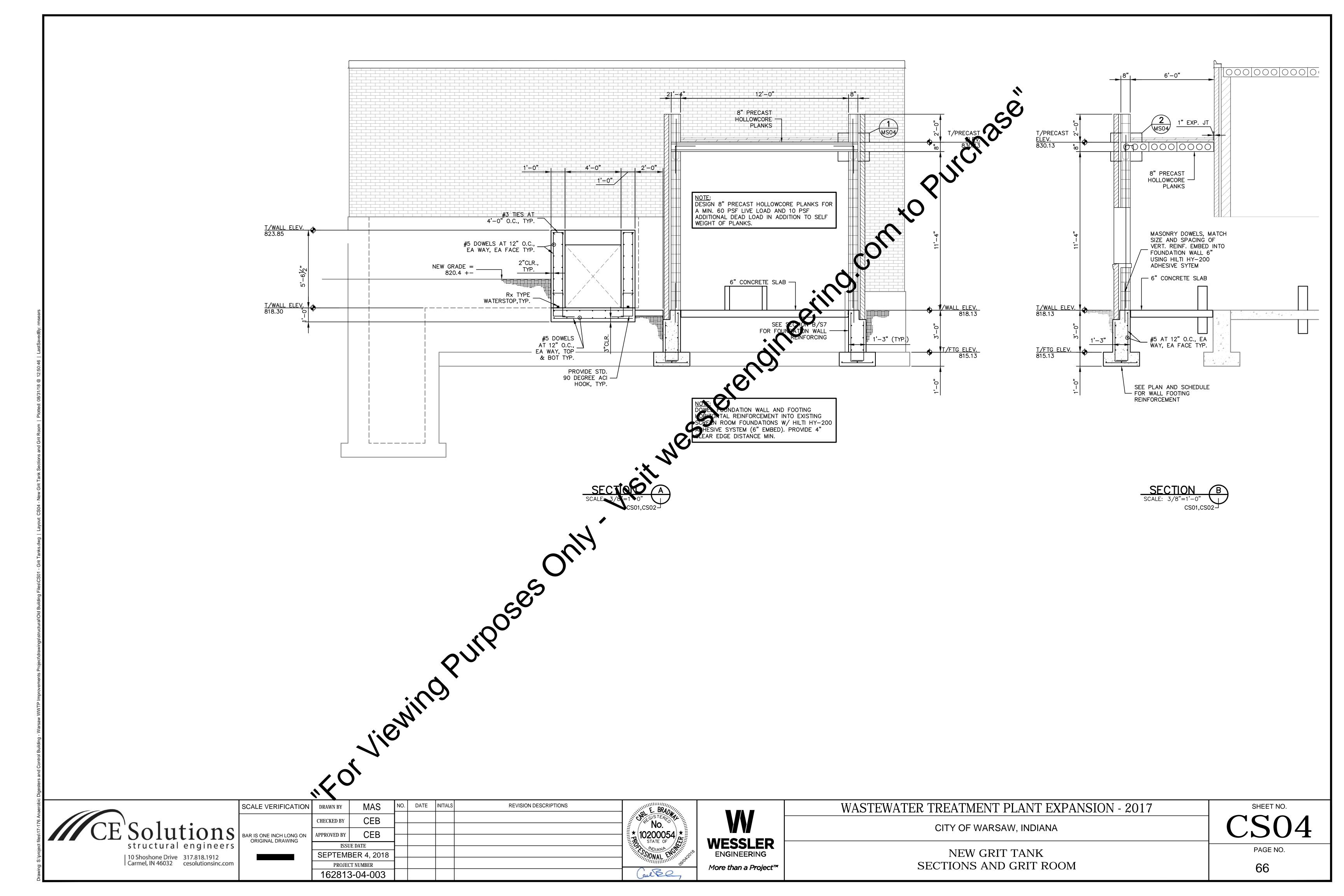
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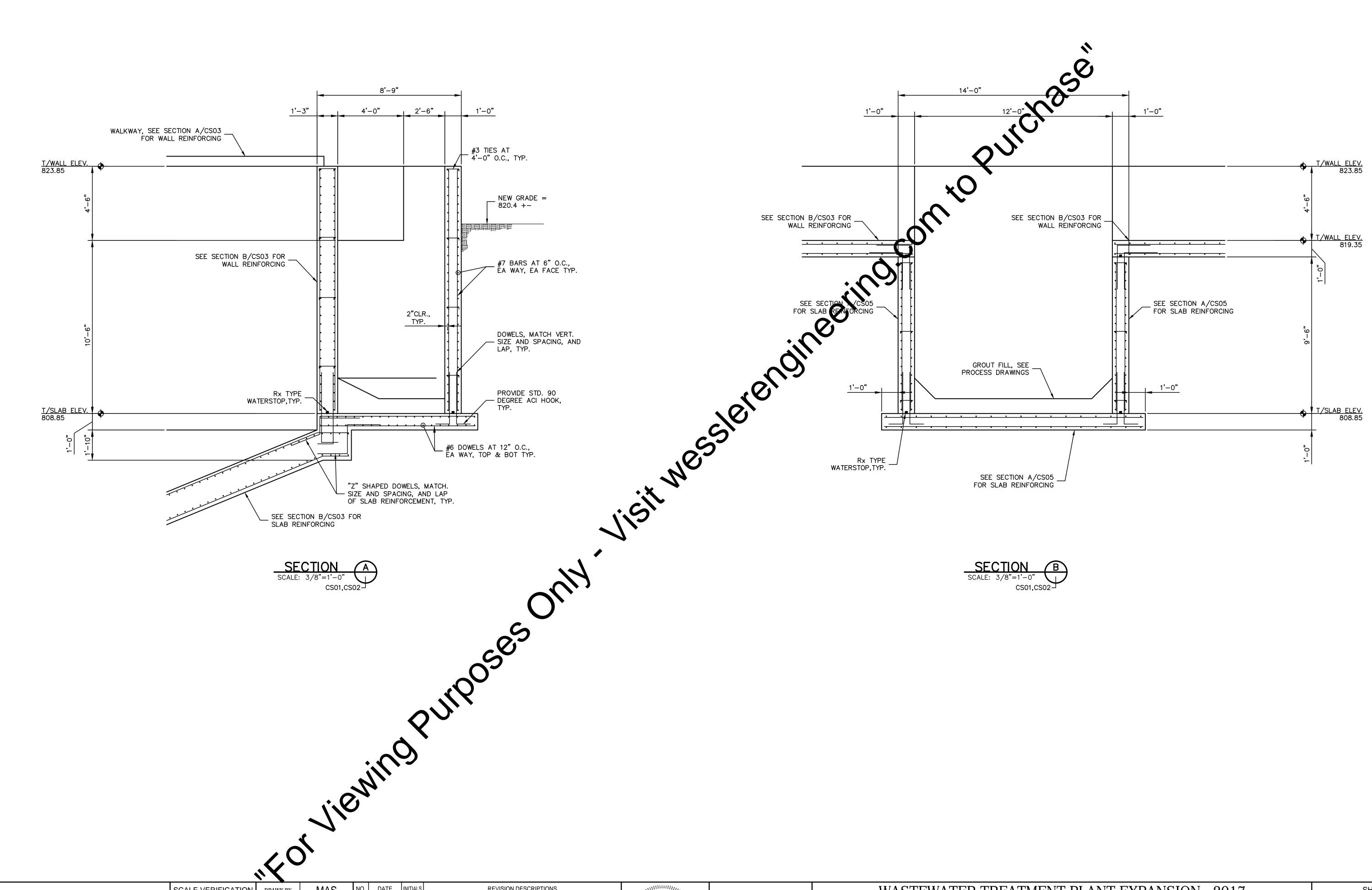
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PAGE NO.
63









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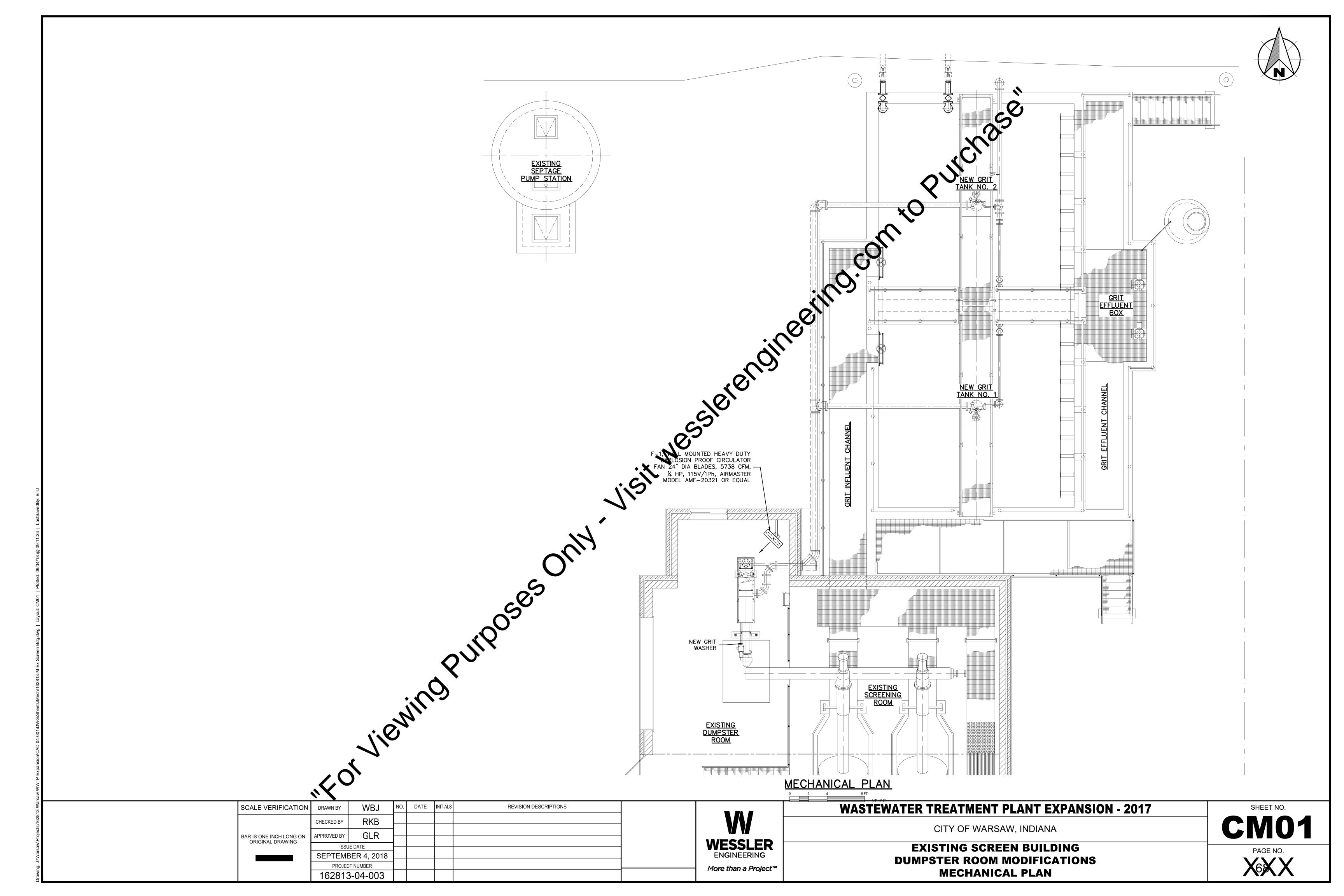
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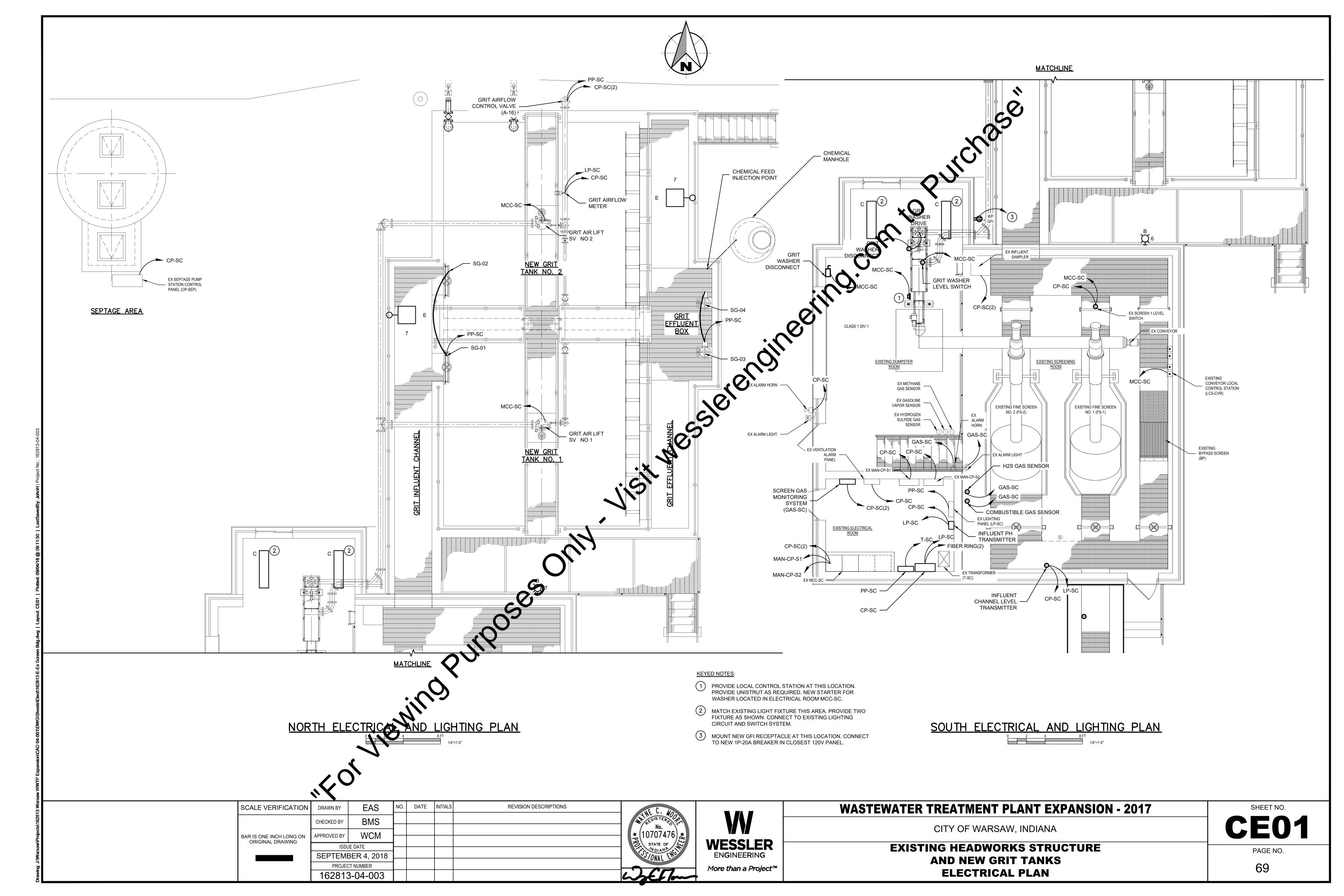


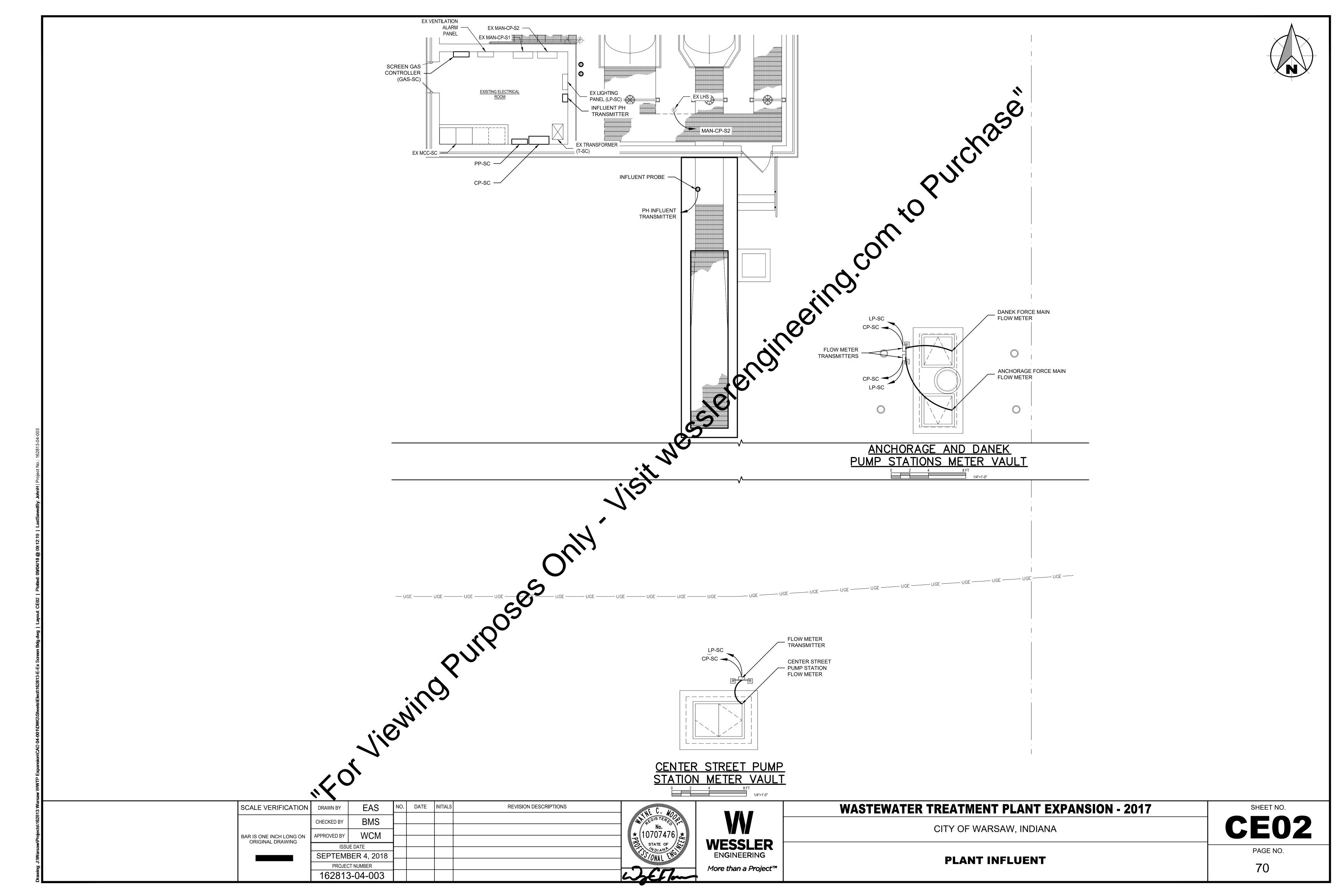
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ENGINEERING

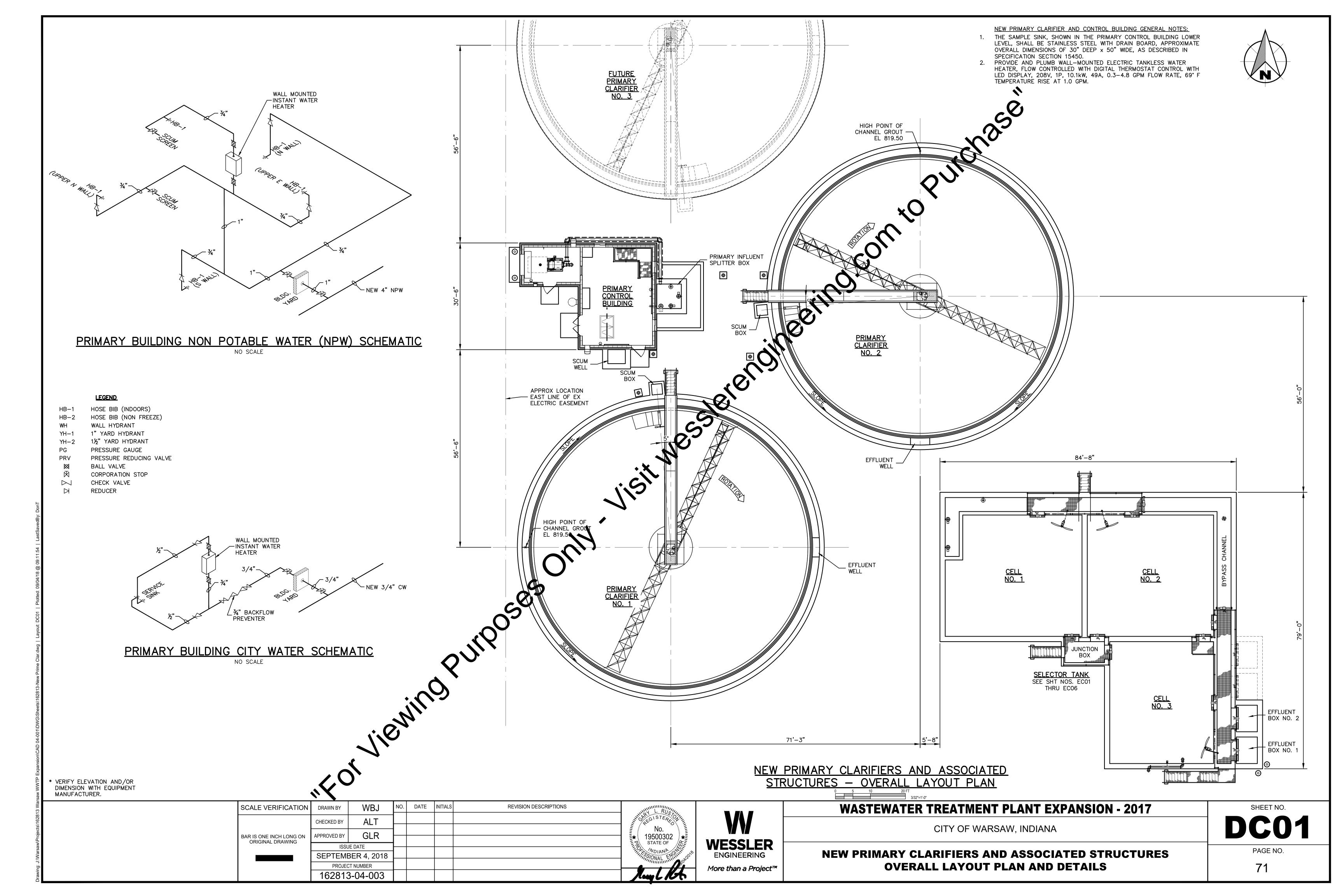
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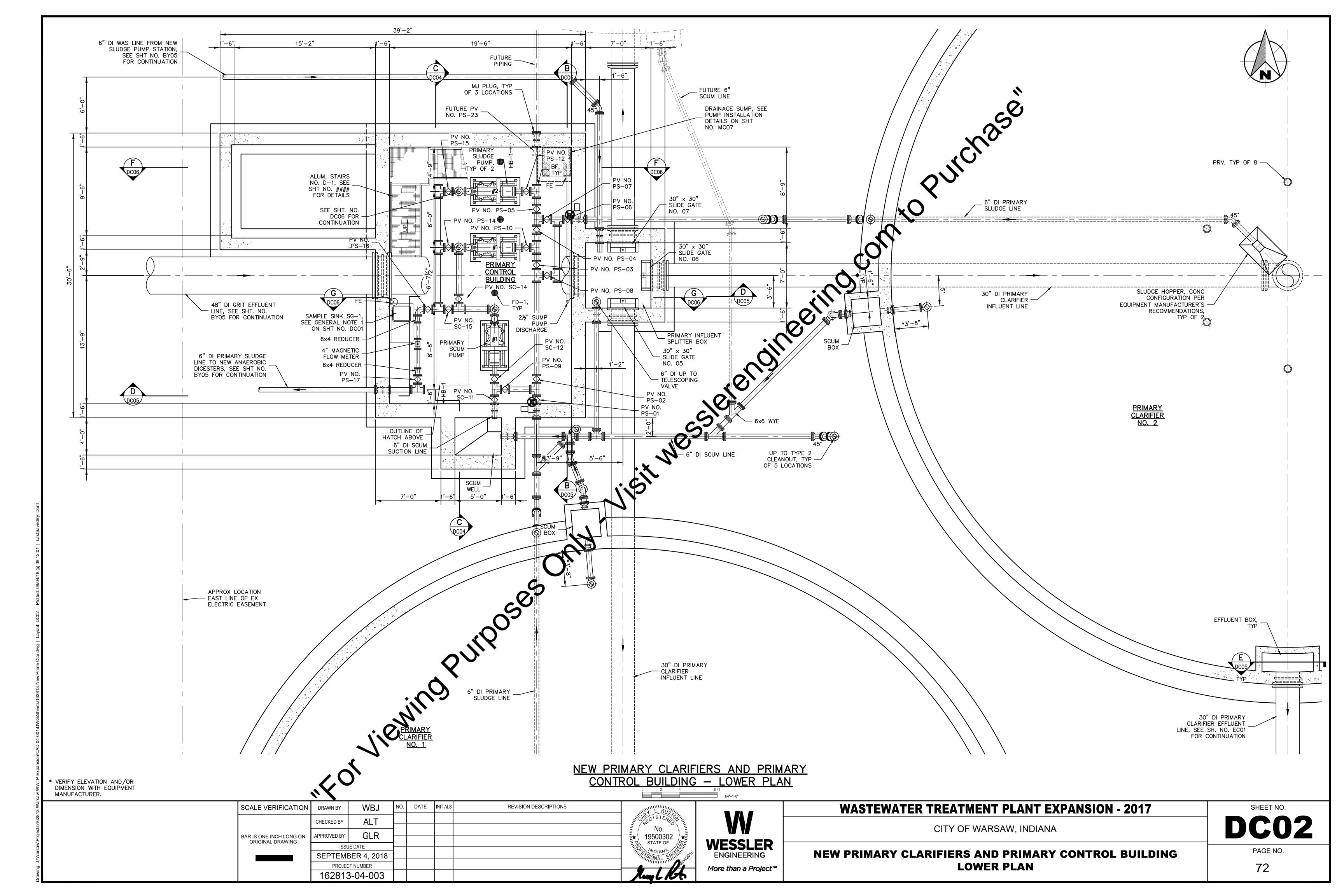
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CITY OF WARSAW, INDIANA	
NEW GRIT TANK	PAGE NO.
SECTIONS	67

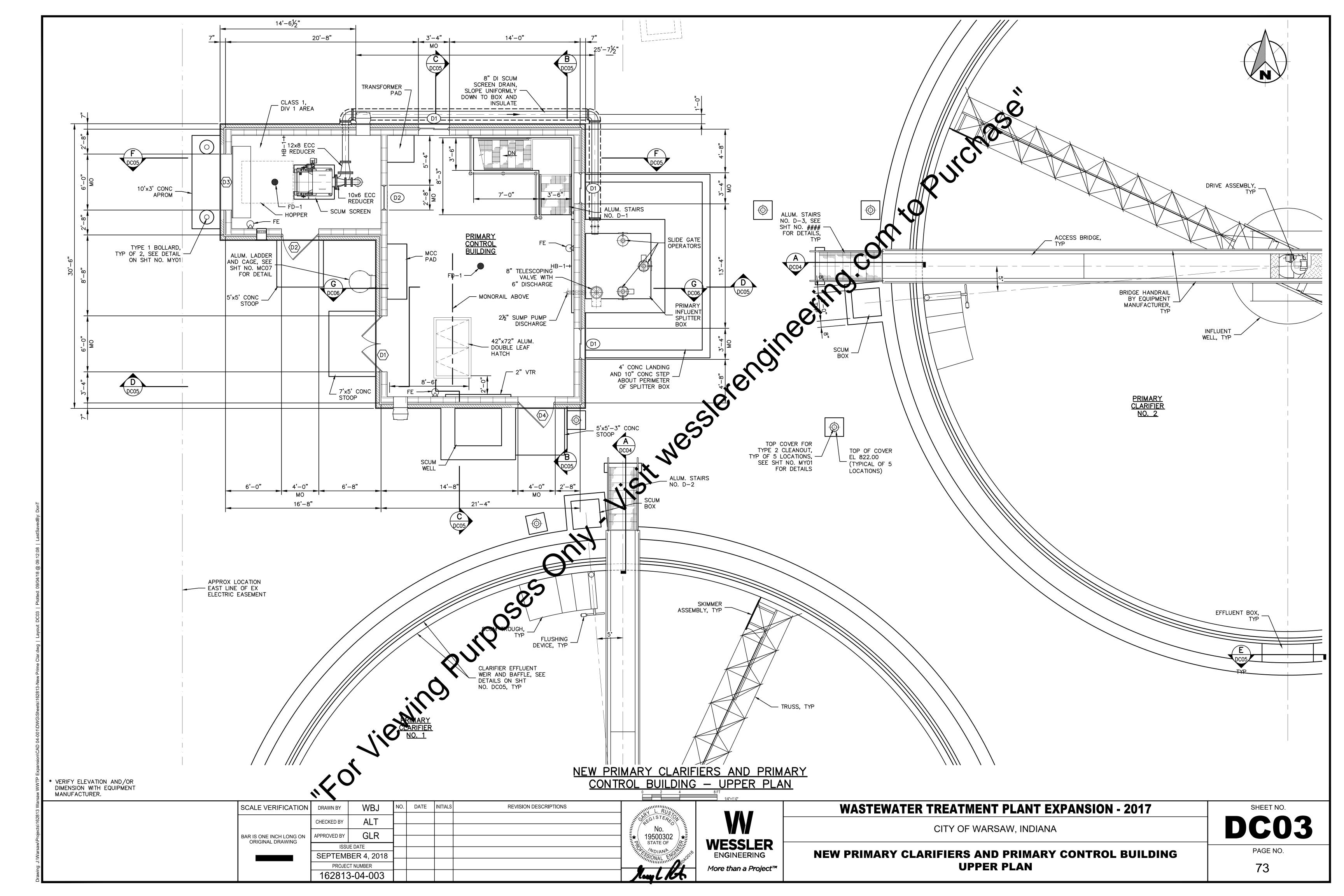


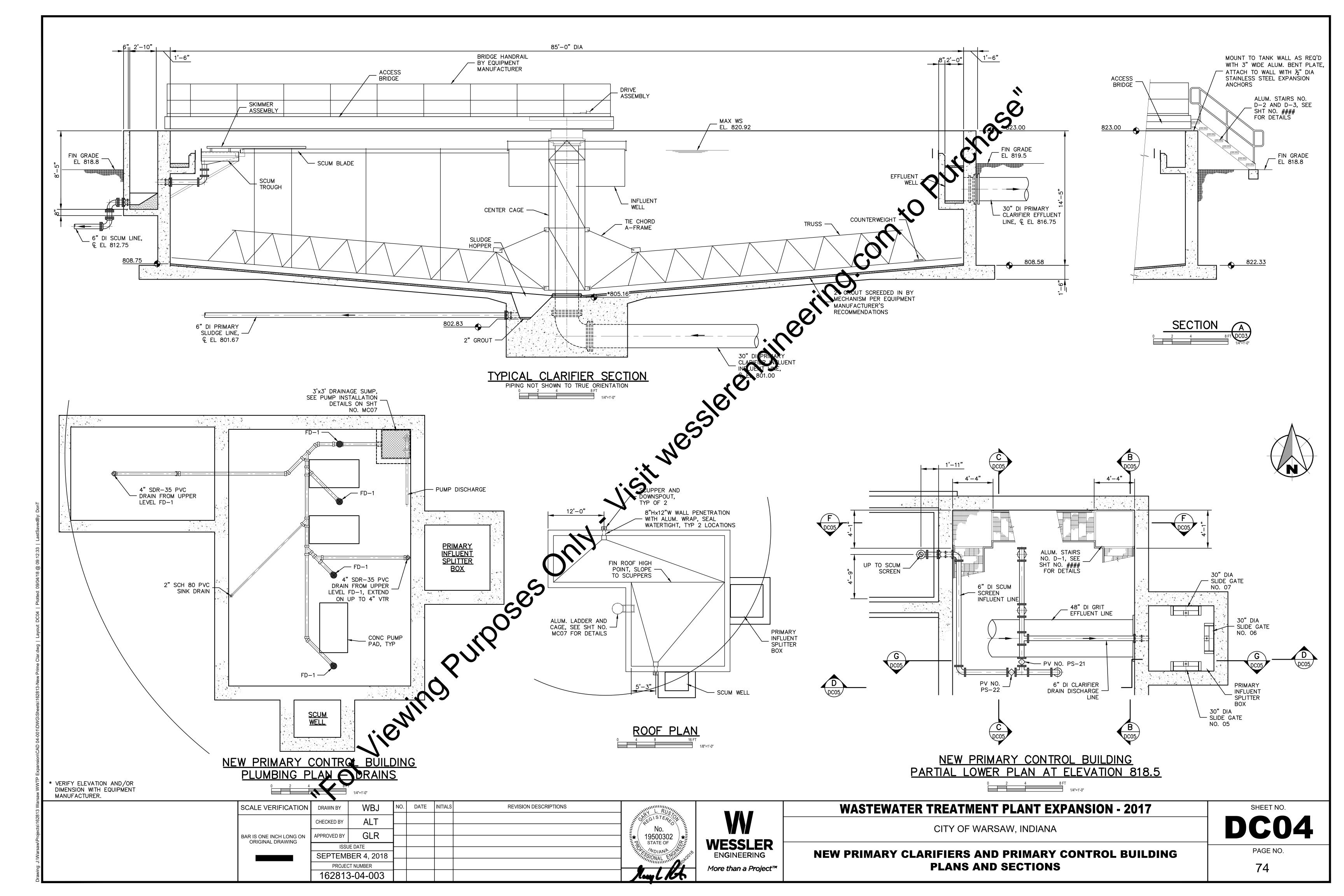


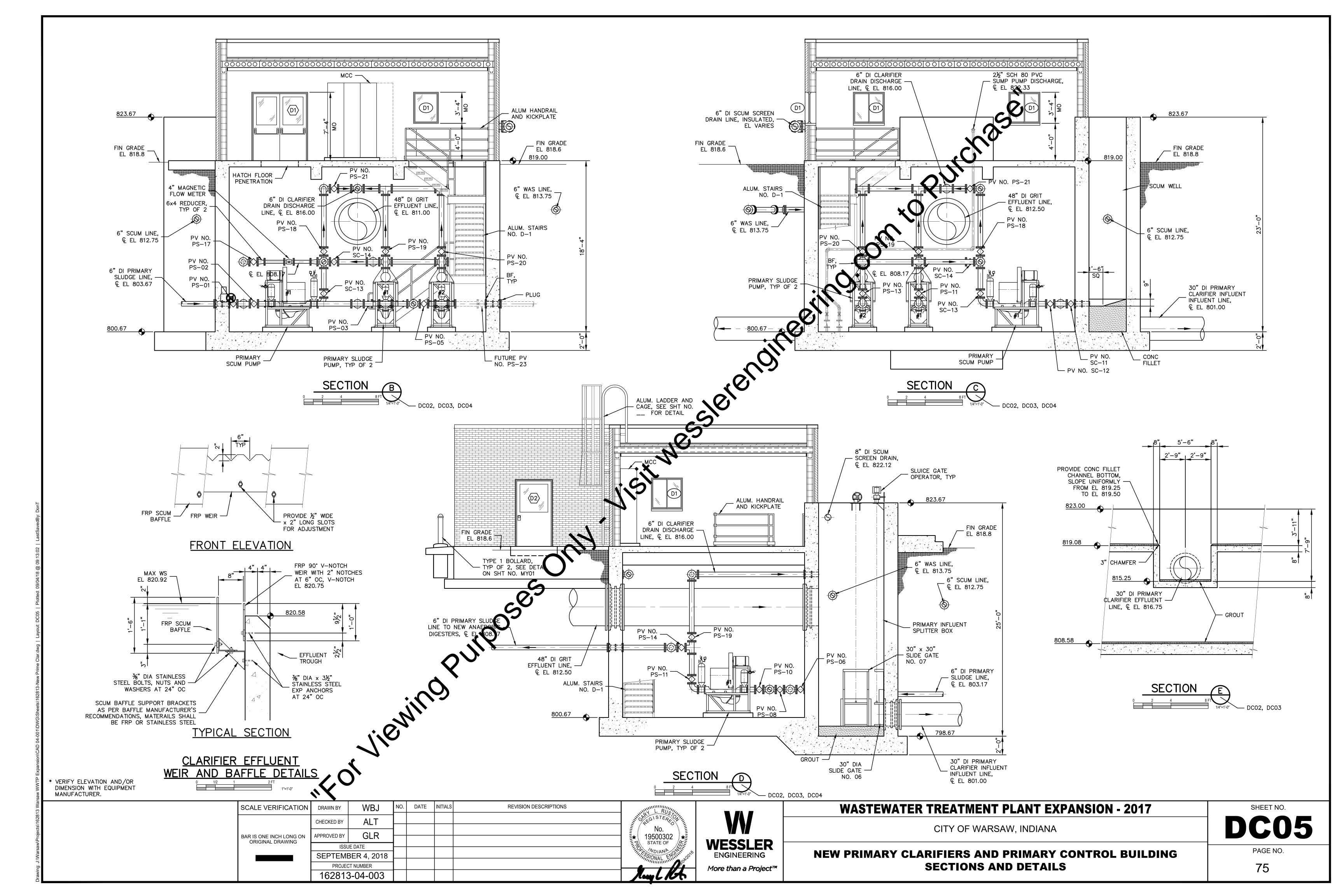


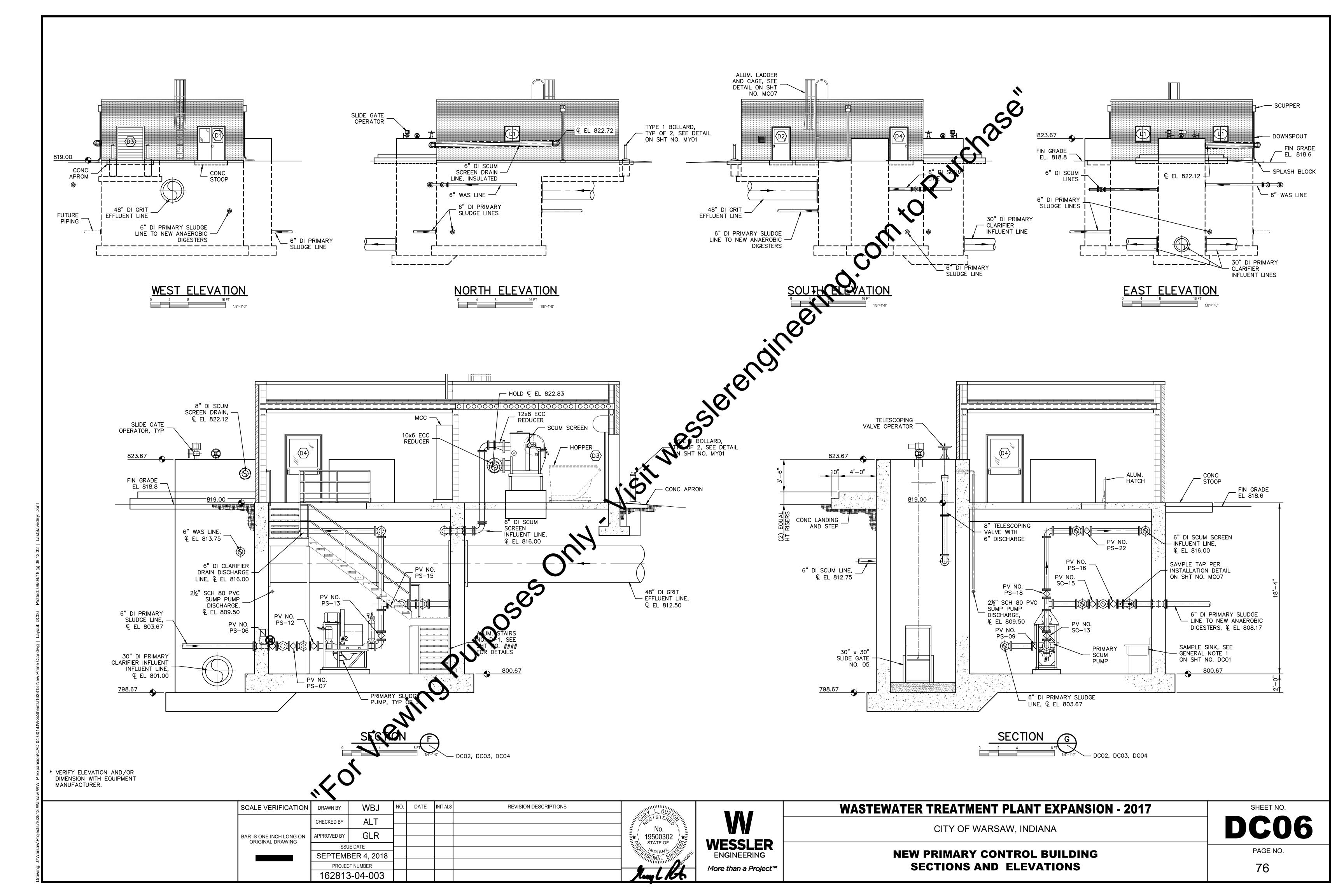


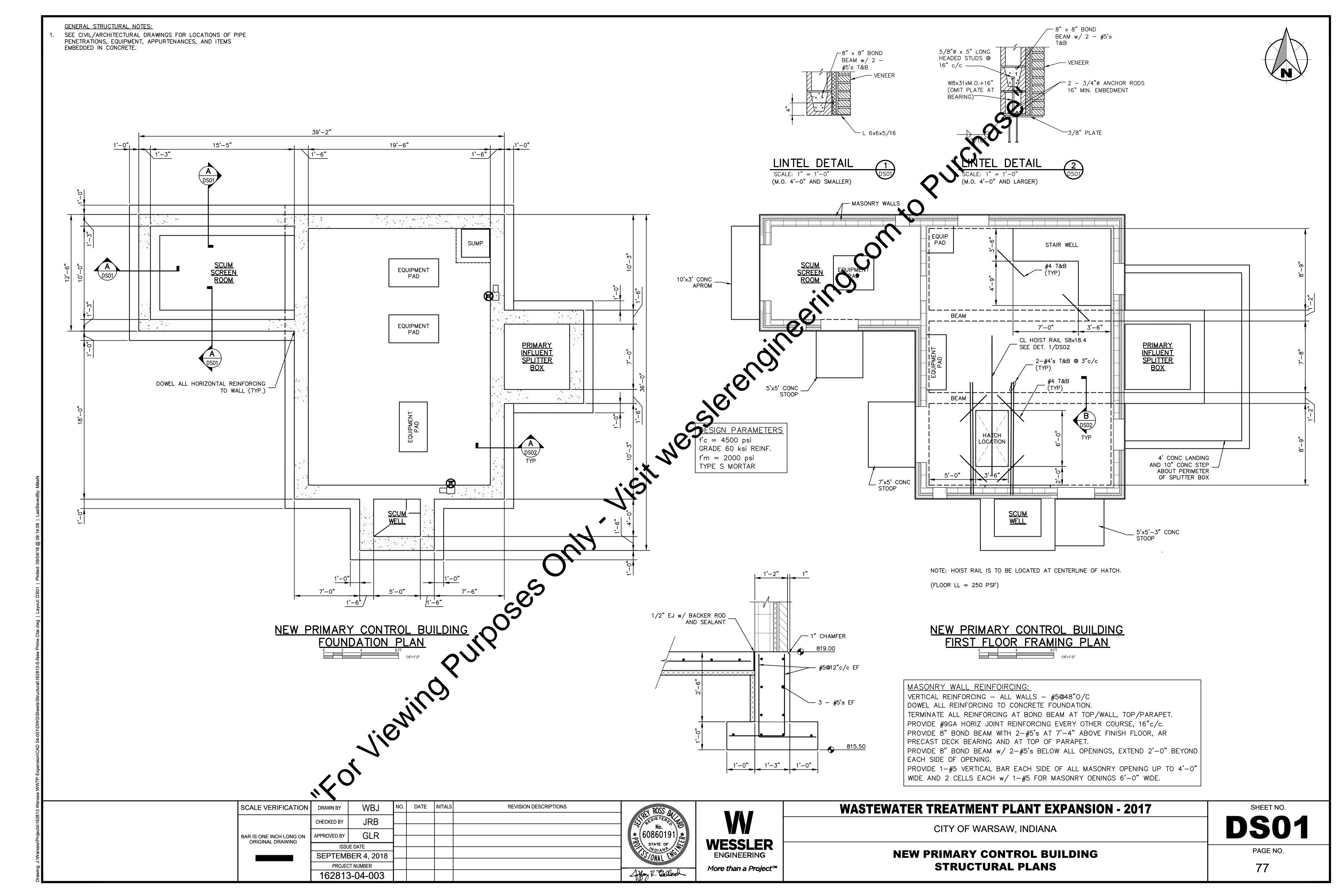


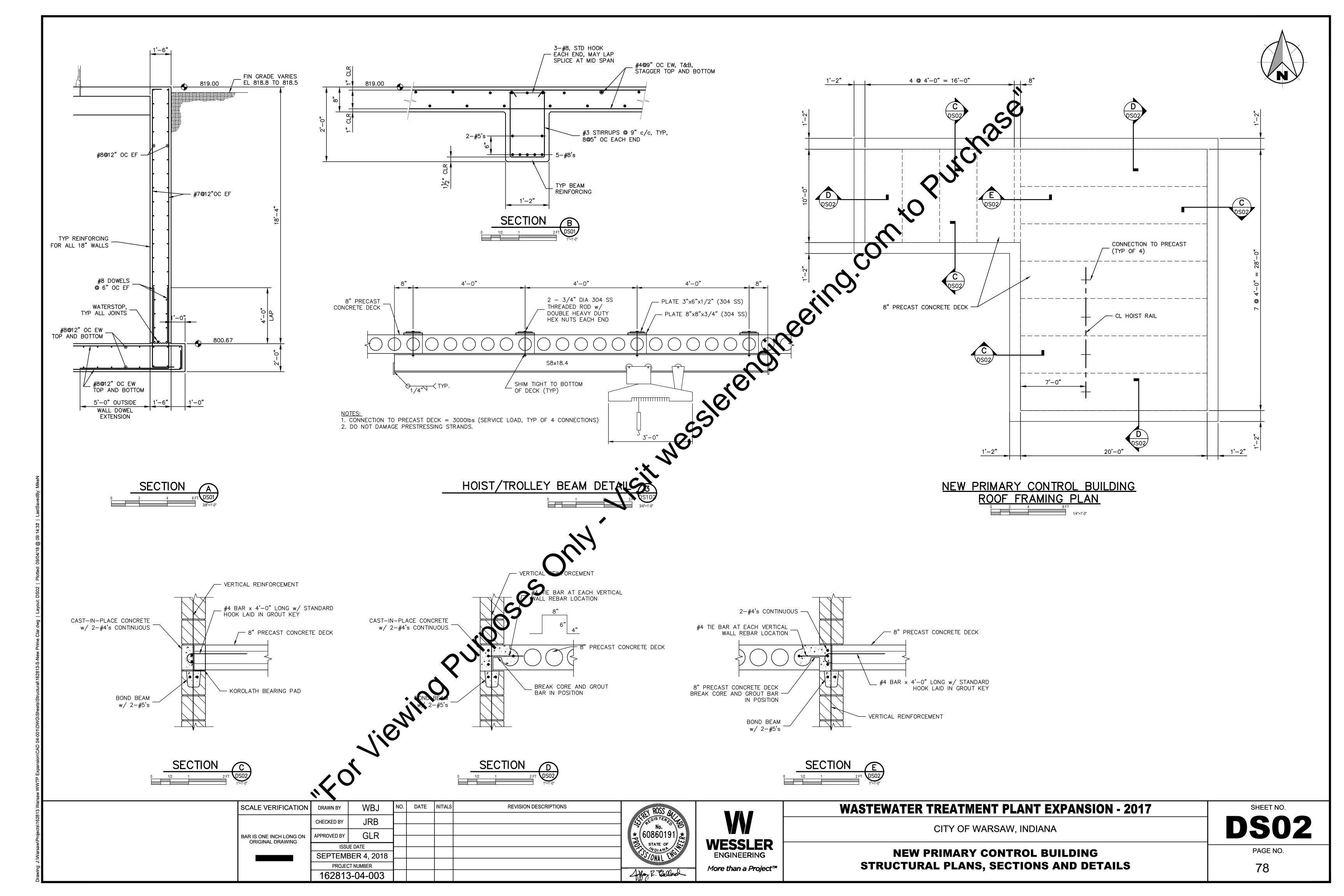


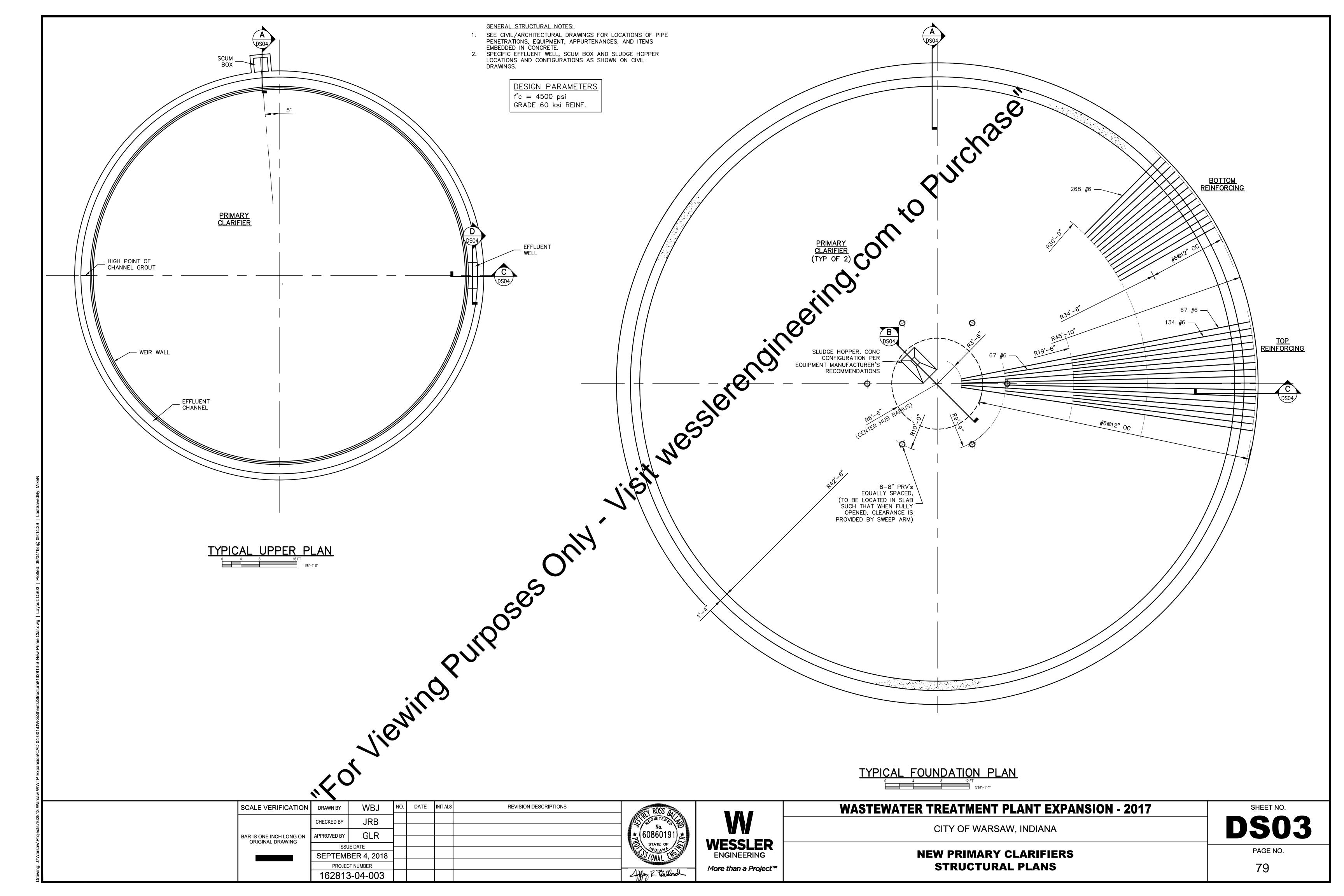


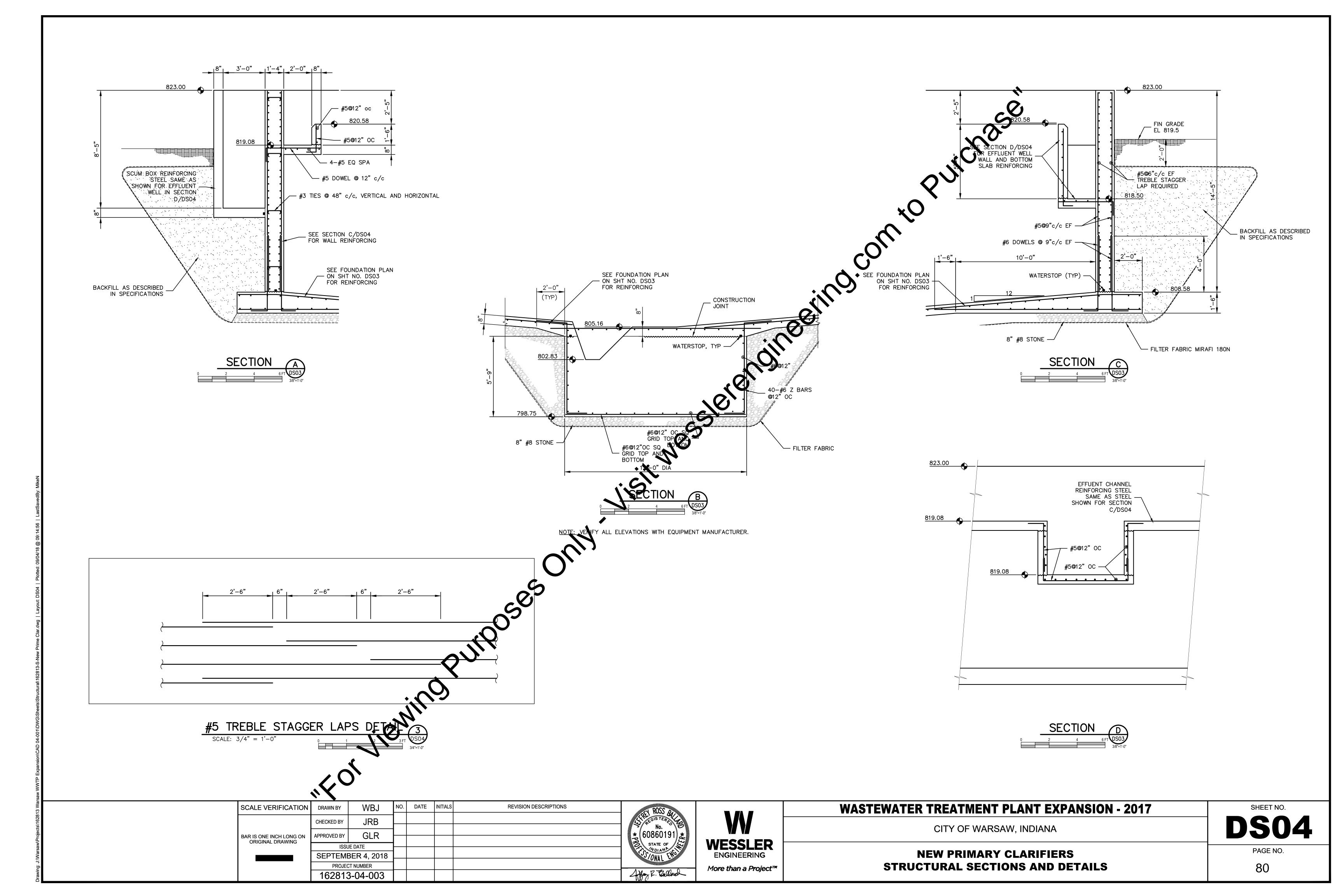












## 

1. GFI CONVENIENCE OUTLET, 2. DDC CONTROLS, 3. INTEGRAL DISCONNECT, 4. ROOF CURB, 5. EXTRA FILTERS, 6. THRU-WALL INSTALLATION PACKAGE, 7. 3-WAY DISCHARGE DIFFUSER, 8. 4-WAY DISCHARGE DIFFUSER

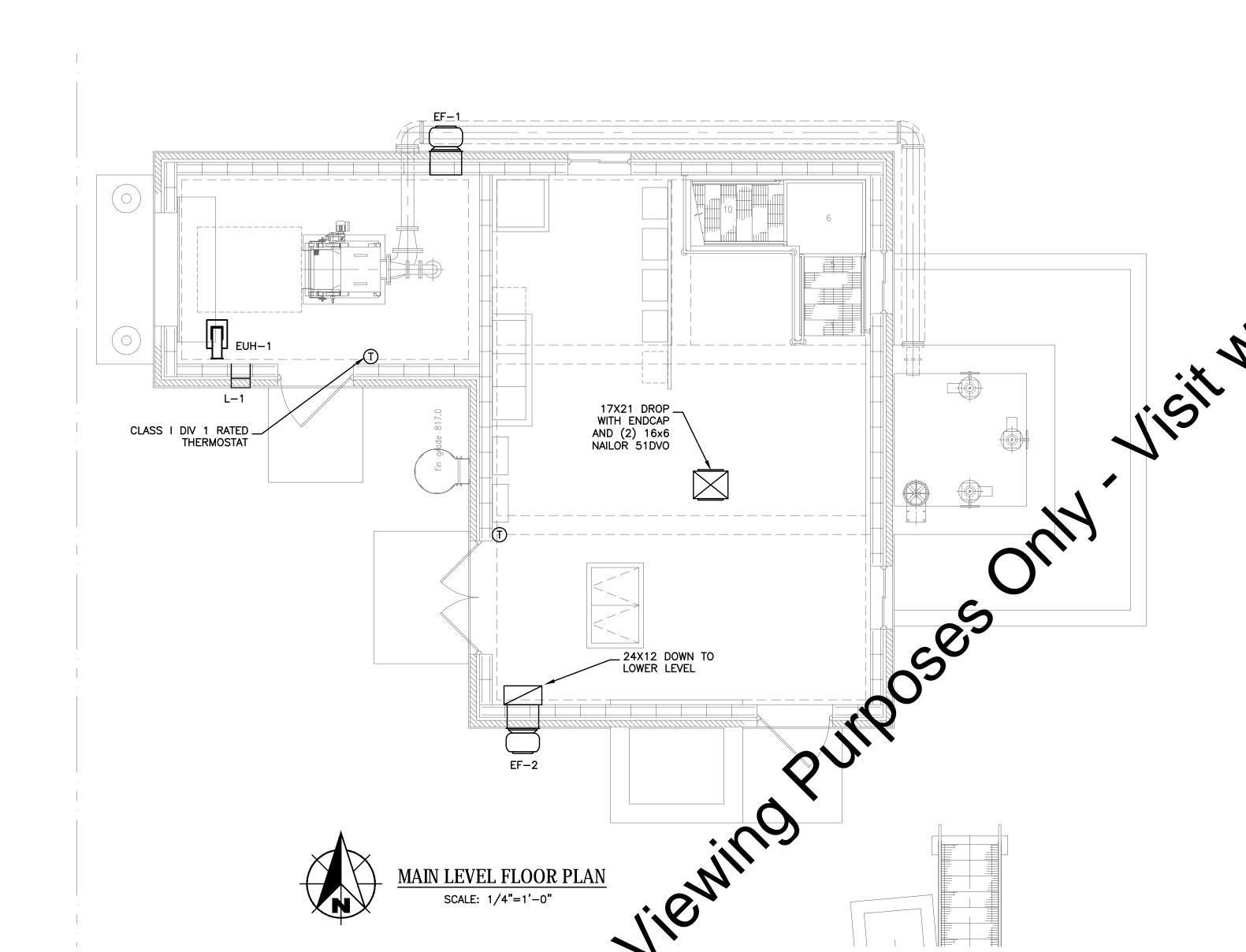
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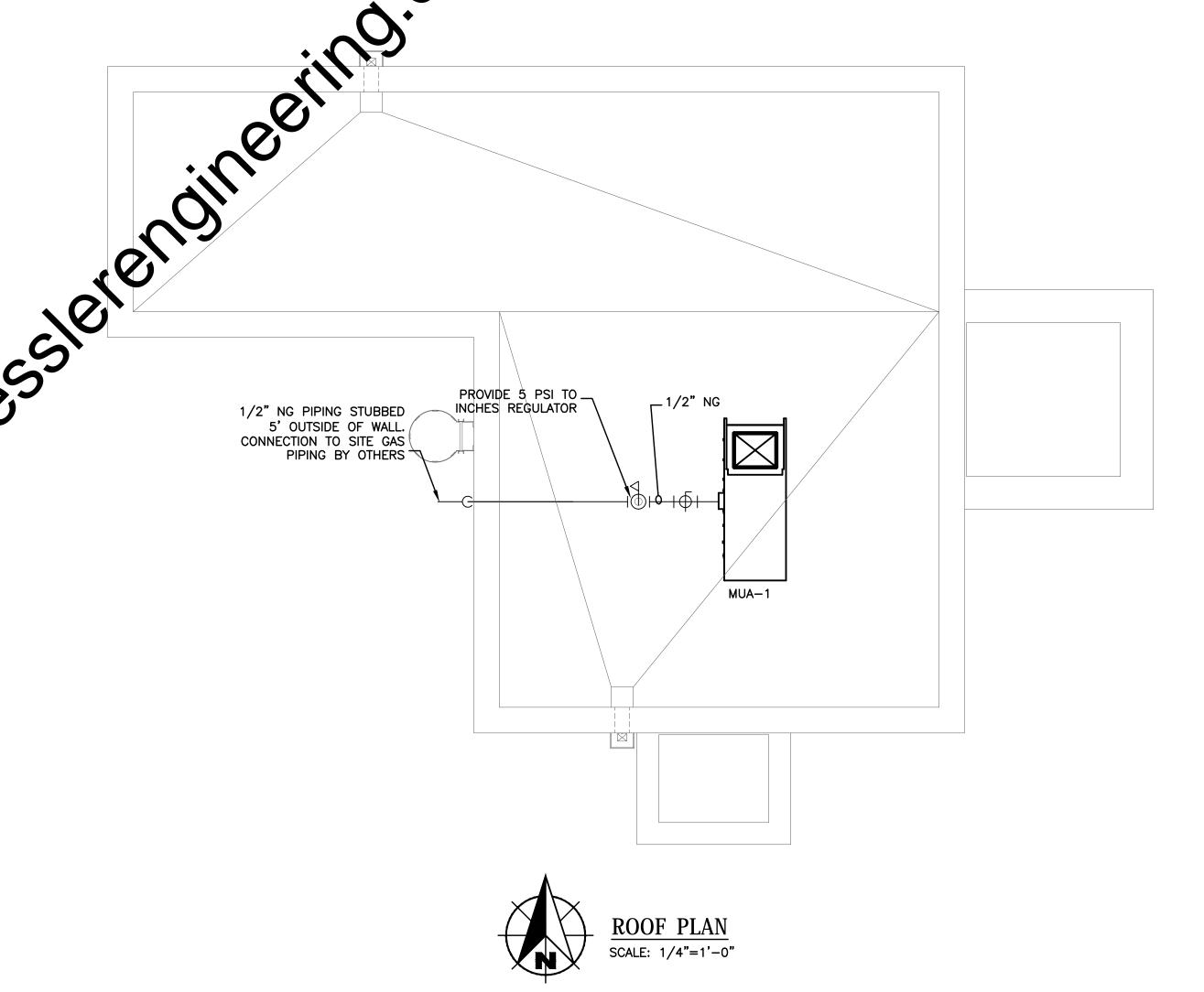
A. INCLUDES 2" THICK METAL MESH FILTERS.

	LOUVER SCHEDULE											
TAG	LOCATION	MANUFACTURER AND MODEL NUMBER	WIDTH	HEIGHT	FREE AREA (SQ. FT)	THCK	OPTIONS	NOTES				
L-1	SCUM ROOM	6"	1,2	INTERLOCK WITH EF-1								
OPTIONS:												
1 120V AC	CTUATOR 2 INTERLOCK WIT											

EXHAUST FAN SCHEDULE MANUFACTURER AND MODEL LOCATION INTERLOCK WITH HP VOLT RPM PH NUMBER 0.250 | 973 | (57) | 2.5 | 1/8 | 115 | 1550 | LOREN COOK 90W15DH EF-1 WALL WALL L-1 0.250 | 533 | (119) | 7.3 | 1/8 | 116 | 1550 | EF-2 LOREN COOK 90W15DH MUA-1 ACCESSORIES: BACKDRAFT DAMPER, DISCONNECT

	ELECTRIC UNIT DEATER SCHEDULE																					
		LOCATION	CONFIGURATION	FAN DATA							1	HE TING DATA			ELECTRICAL DATA			ACCESSORIES			MANUEACTURER WITH MORE	
MARK	MARK			AIRFLOW (CFM)	ESP	DRIVE	DESIGN SPEED(S)		HP	M K		MBH	EAT	LAT	AMPS	VOLTS	PH	DISCONNECT SWITCH	INTEGRAL THERMOSTAT	WALL BRACKET	MANUFACTURER WITH MODEI NUMBER	NOTES
	EUH-1	SCUM ROOM	UNIT HEATER	700	_	AXIAL	1	4	100	<b>)</b>	5	17.1	_	_	12.1	480	3	Y	Y	Y	QMARK GUX5004832	1
		NOTES: 1. EXPLOSION PROC	OF CONSTRUCTION			-		1														







SCALE VERIFICATION DRAWN BY NO. DATE INITIALS REVISION DESCRIPTIONS

CHECKED BY

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ISSUE DATE

MARCH 2018

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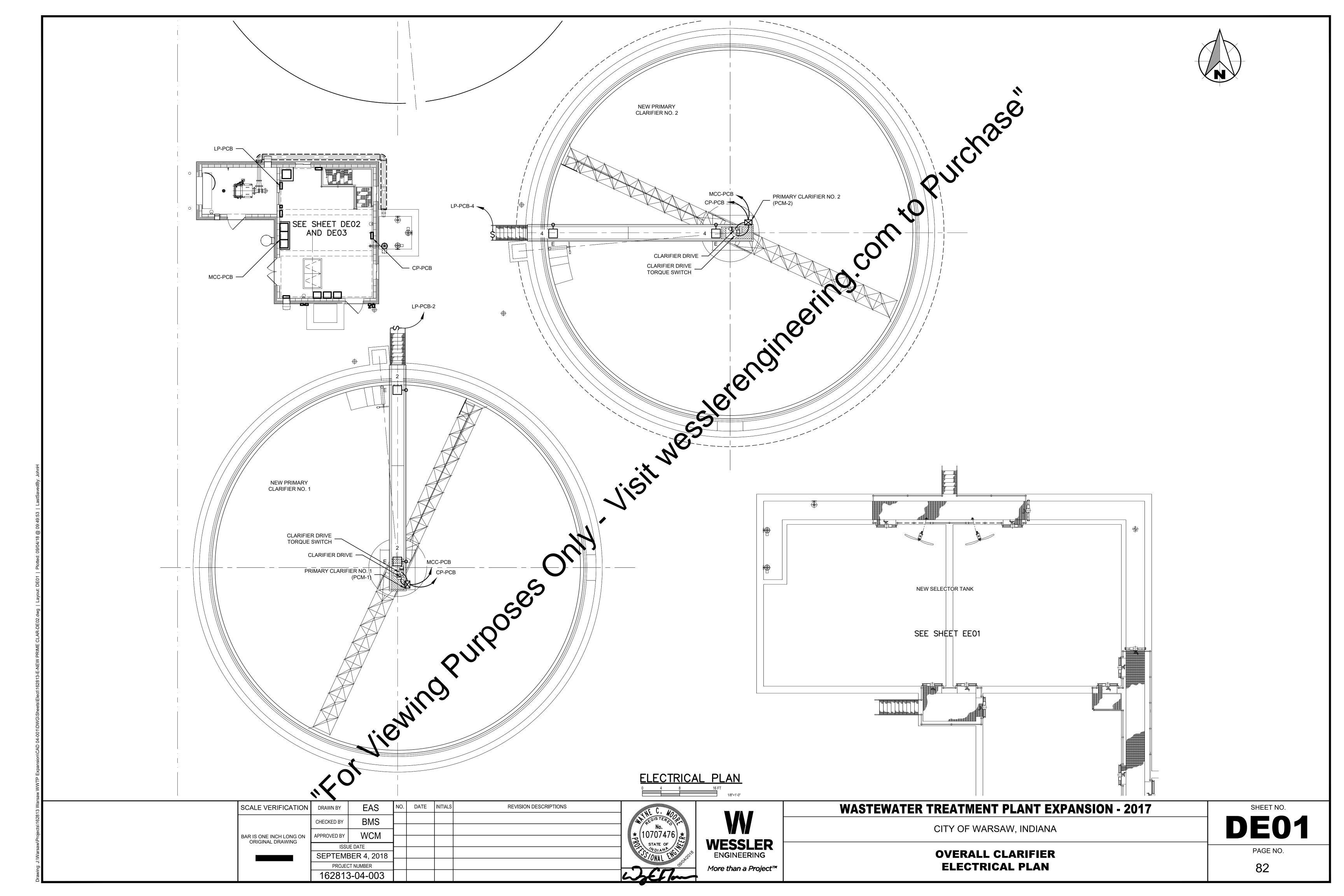
WASTEWATER TREATMENT PLANT EXPANSION - 2017

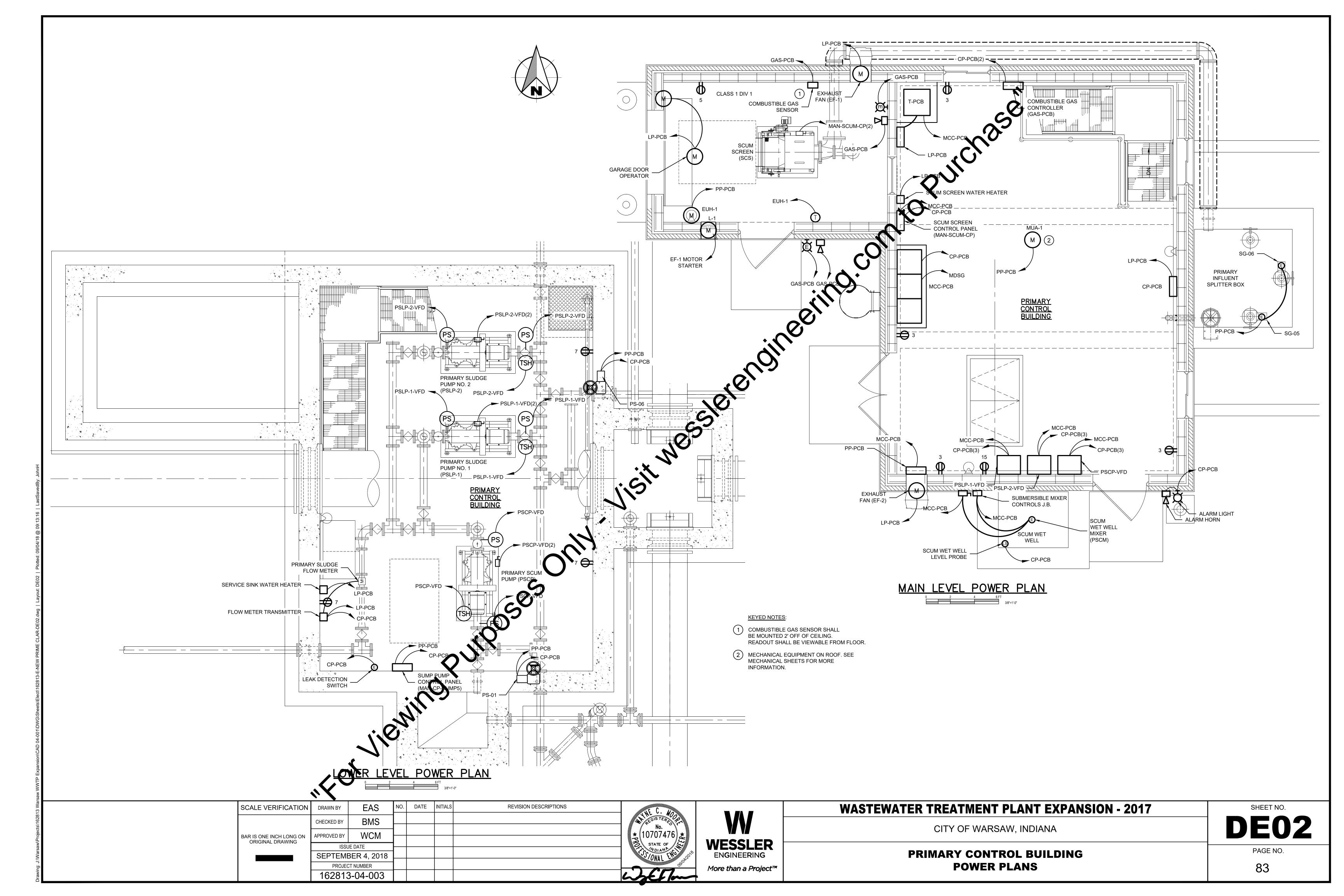
CITY OF WARSAW, INDIANA

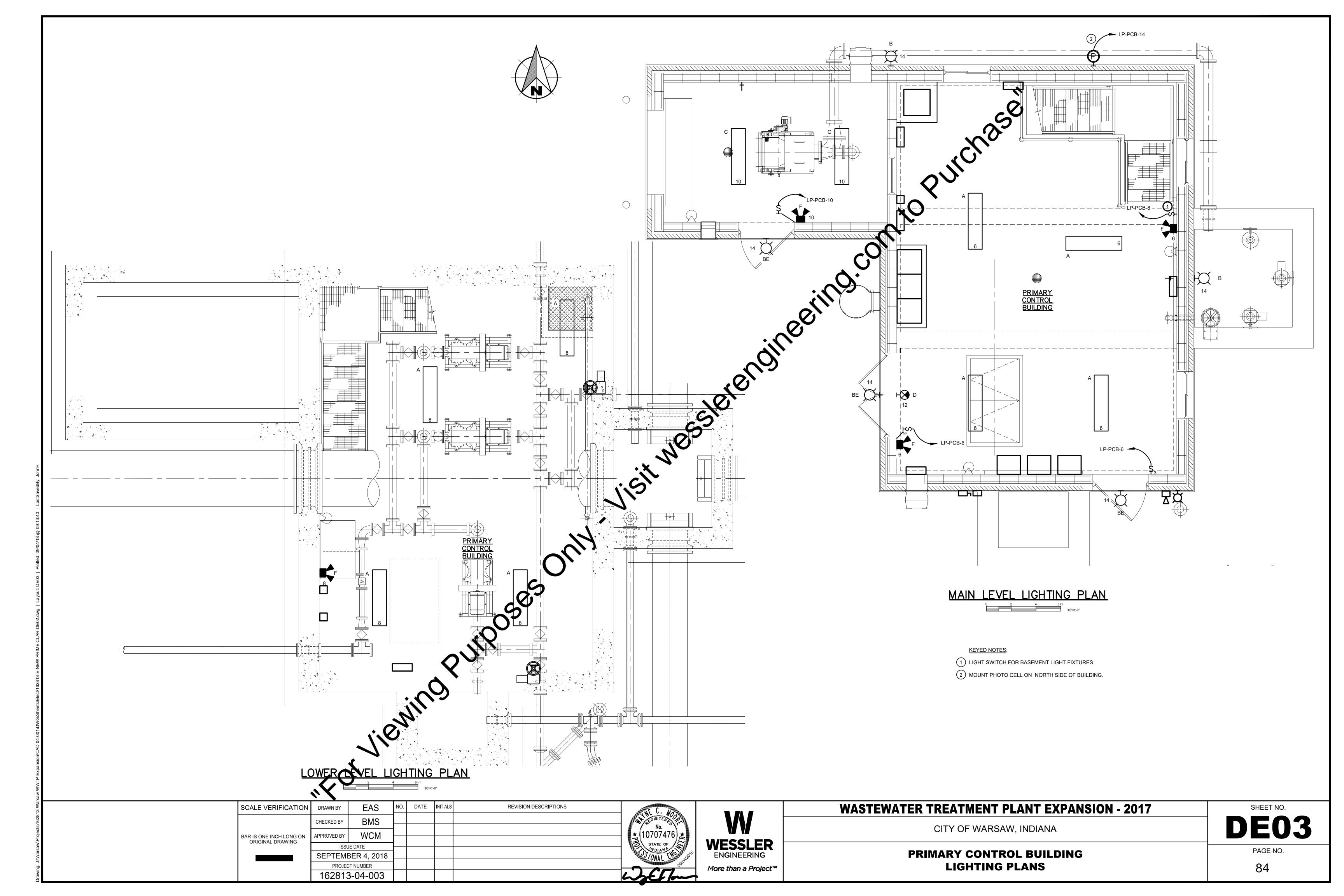
NEW PRIMARY CLARIFIERS AND PRIMARY CONTROL BUILDING MECHANICAL PLAN

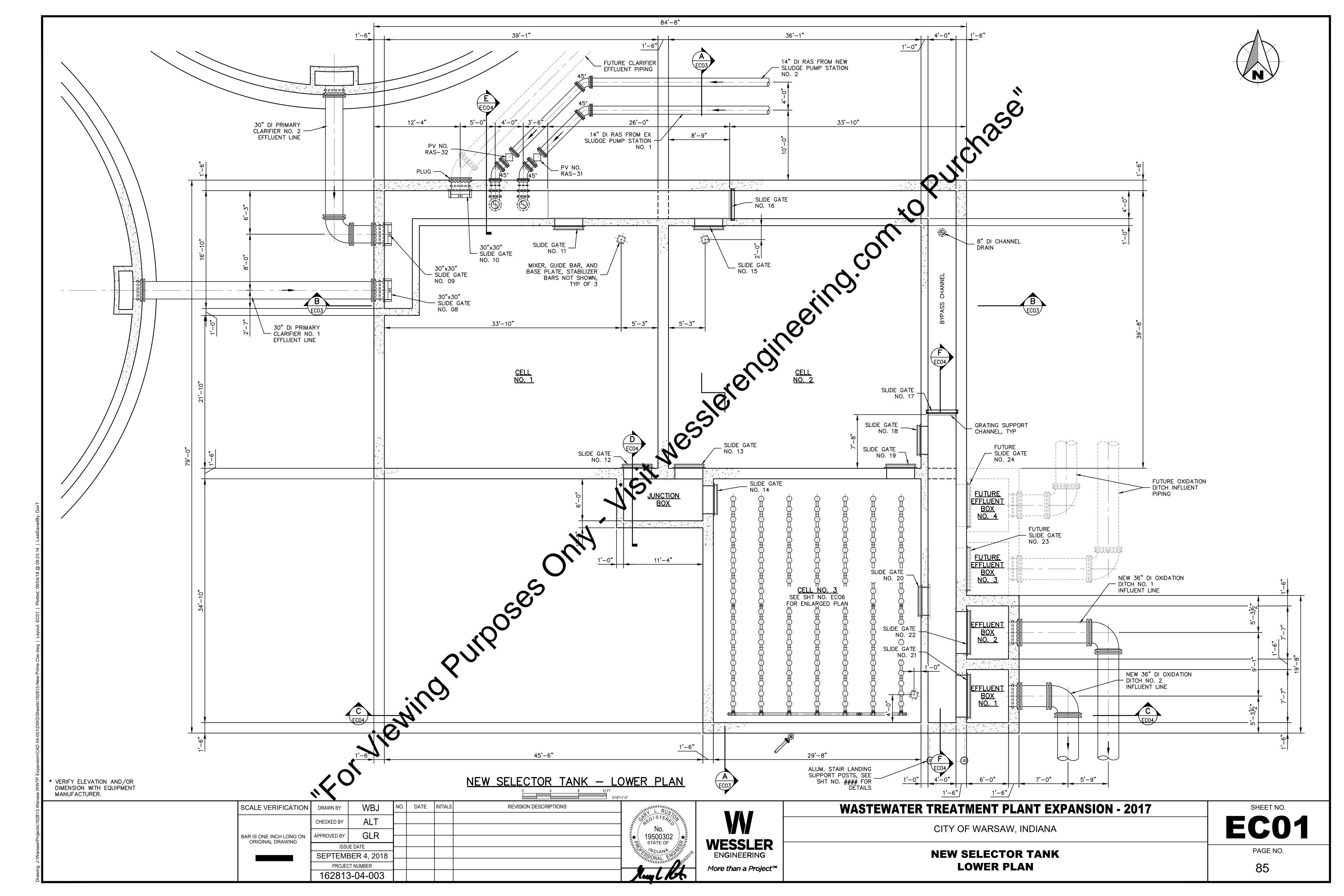
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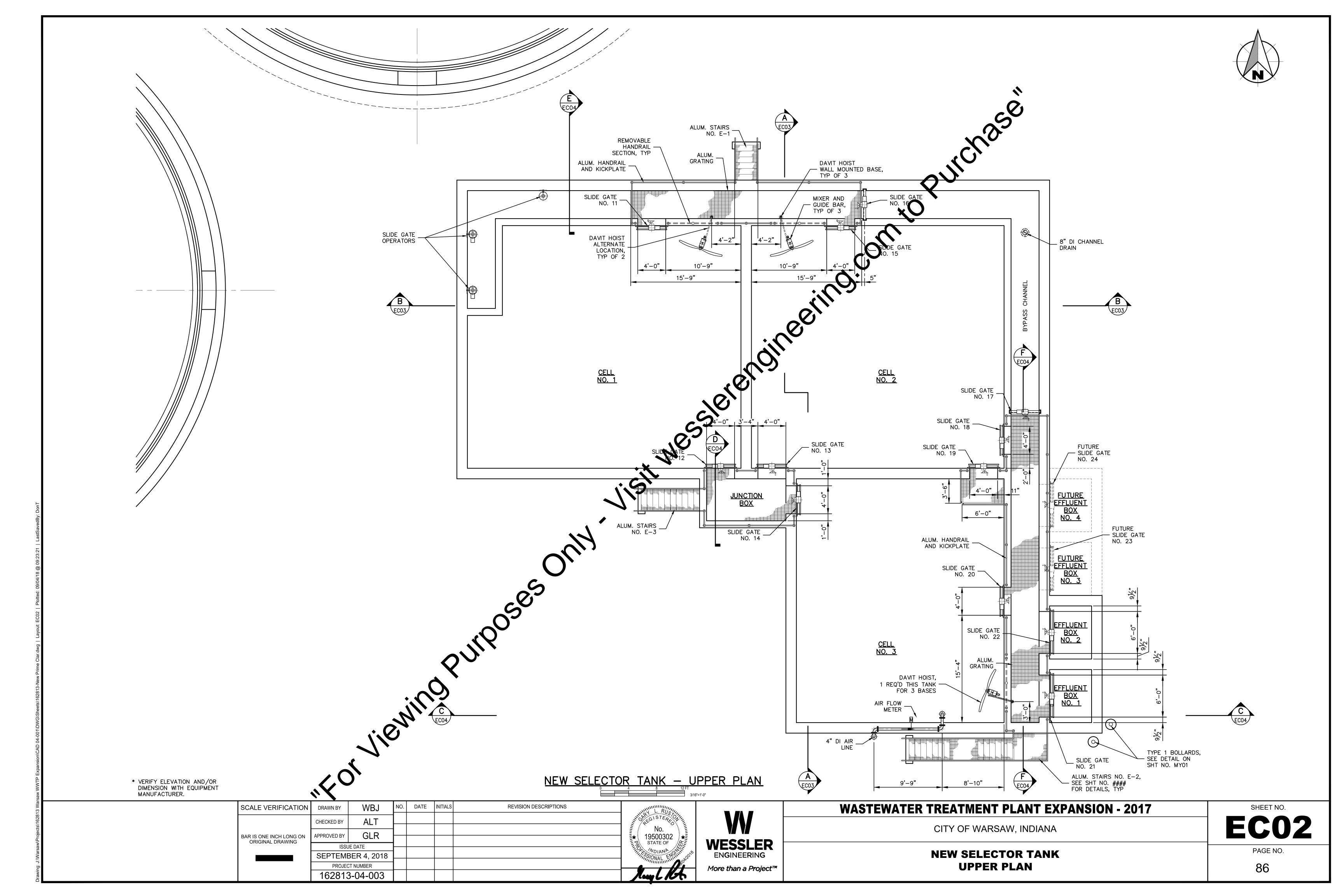
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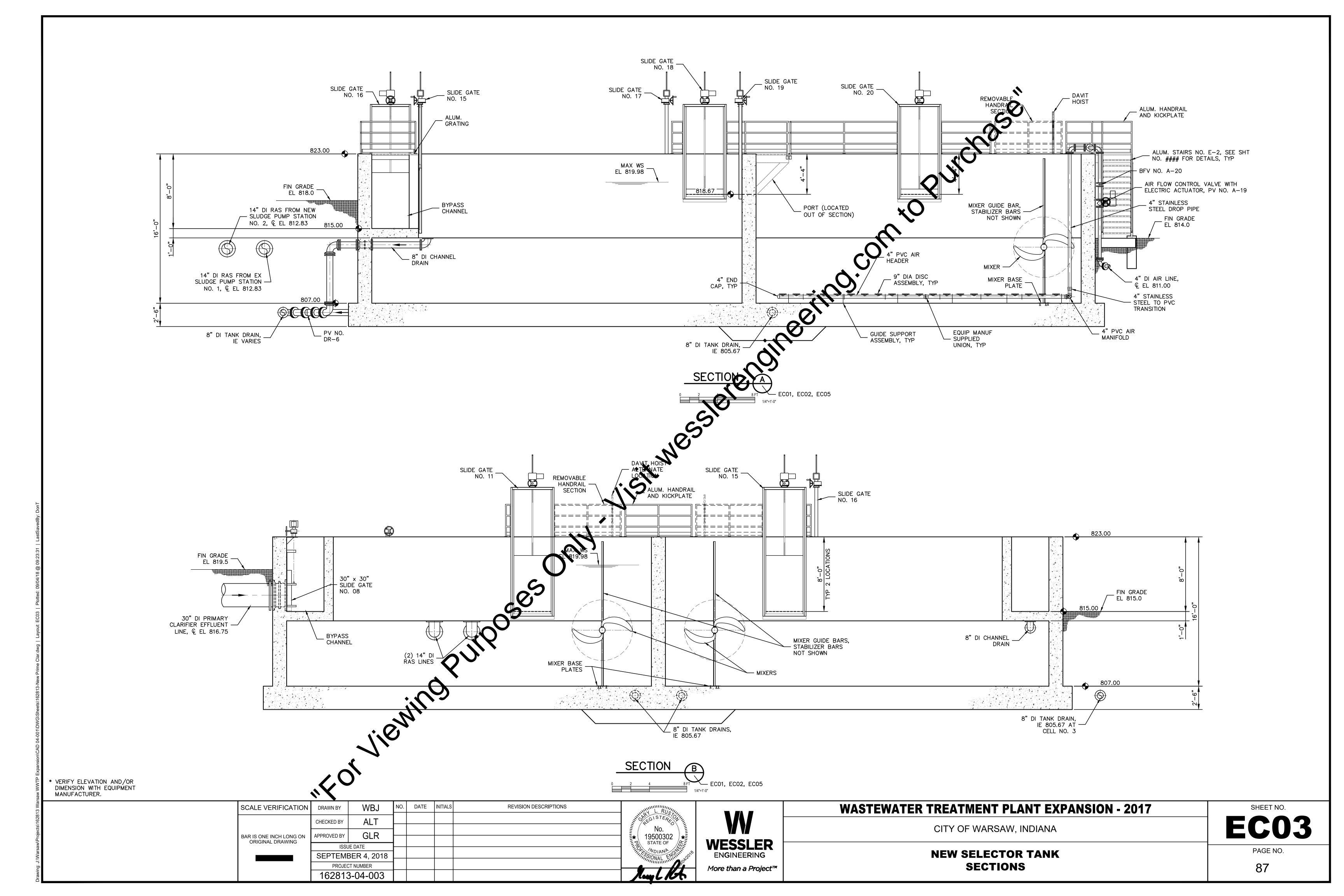


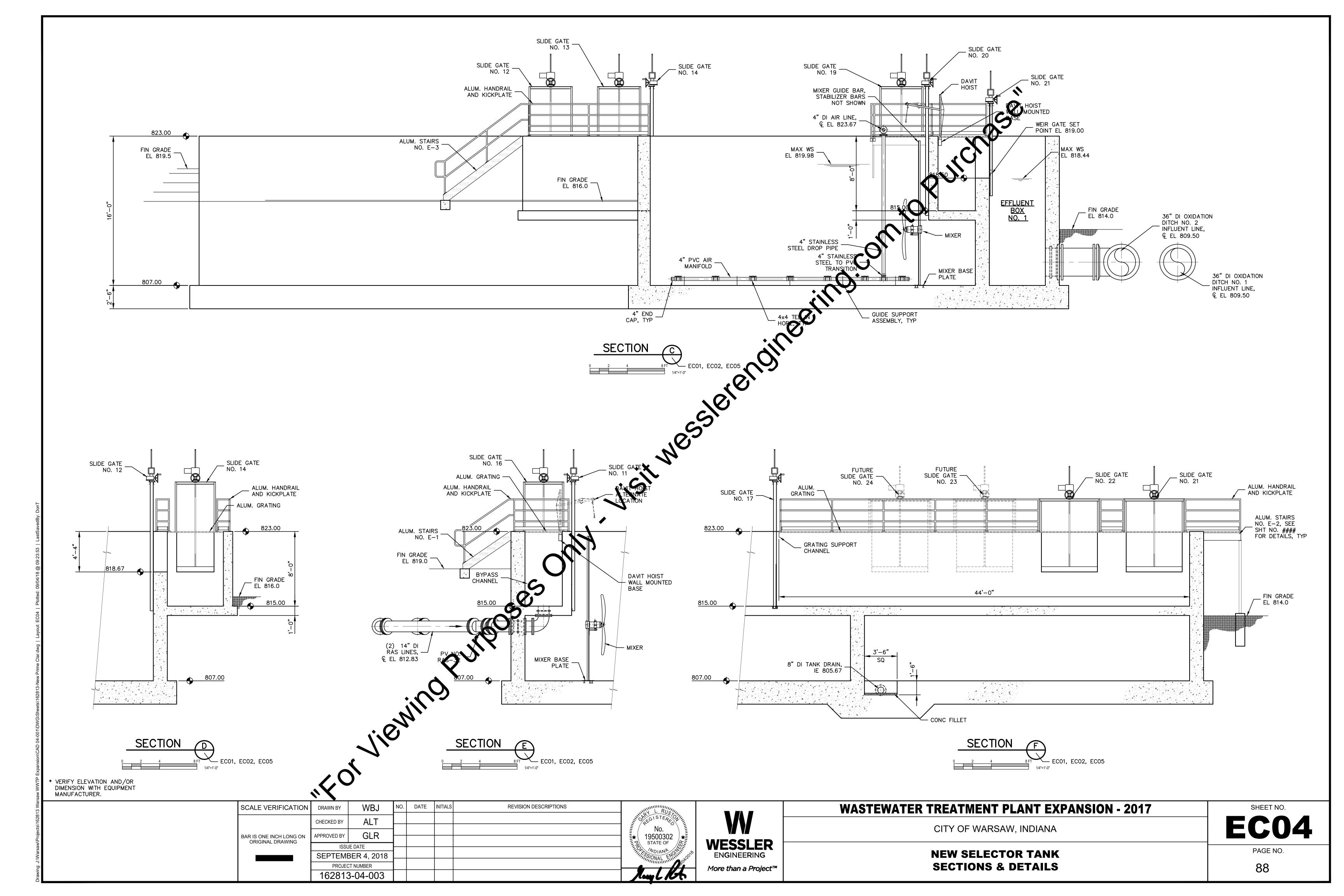


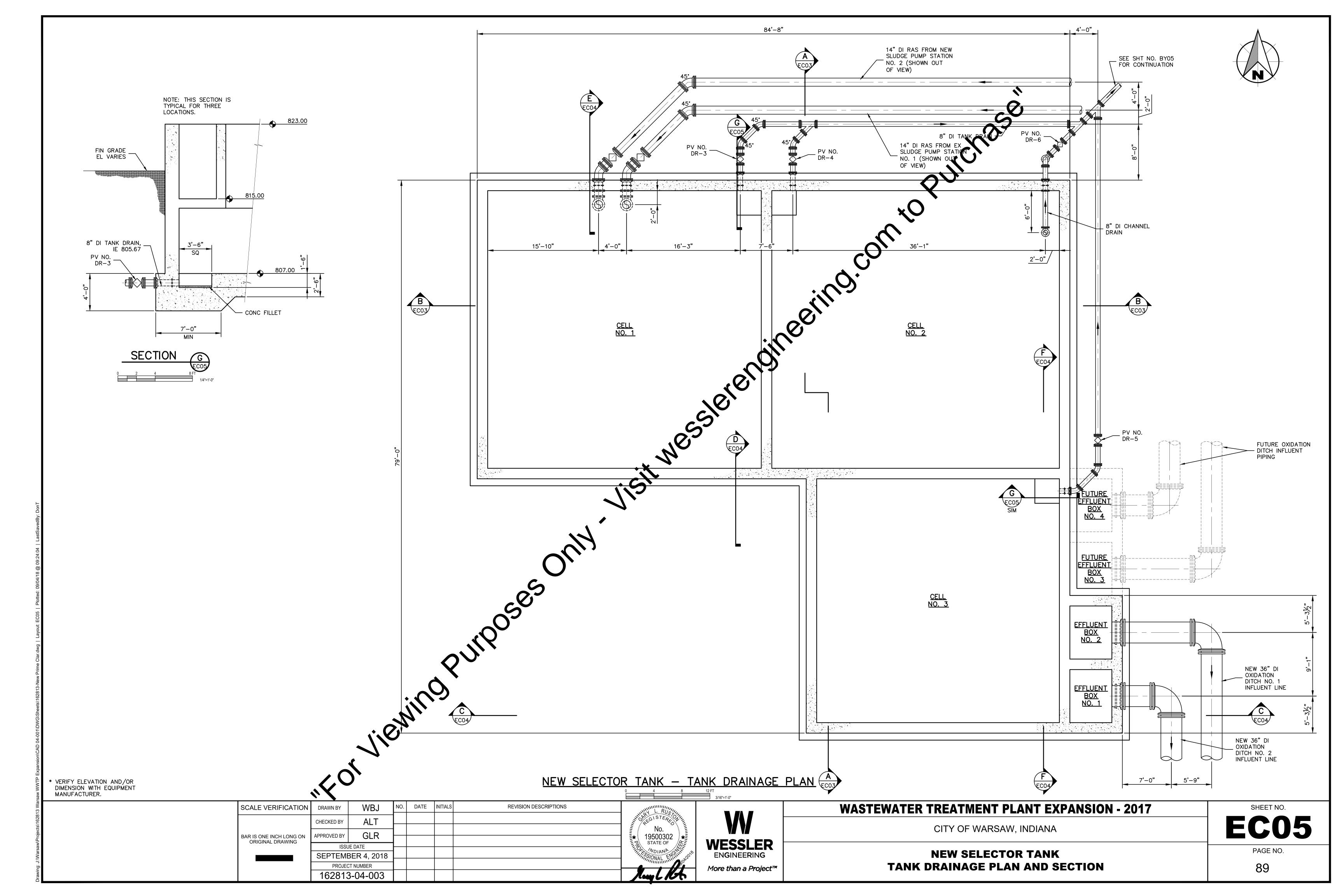


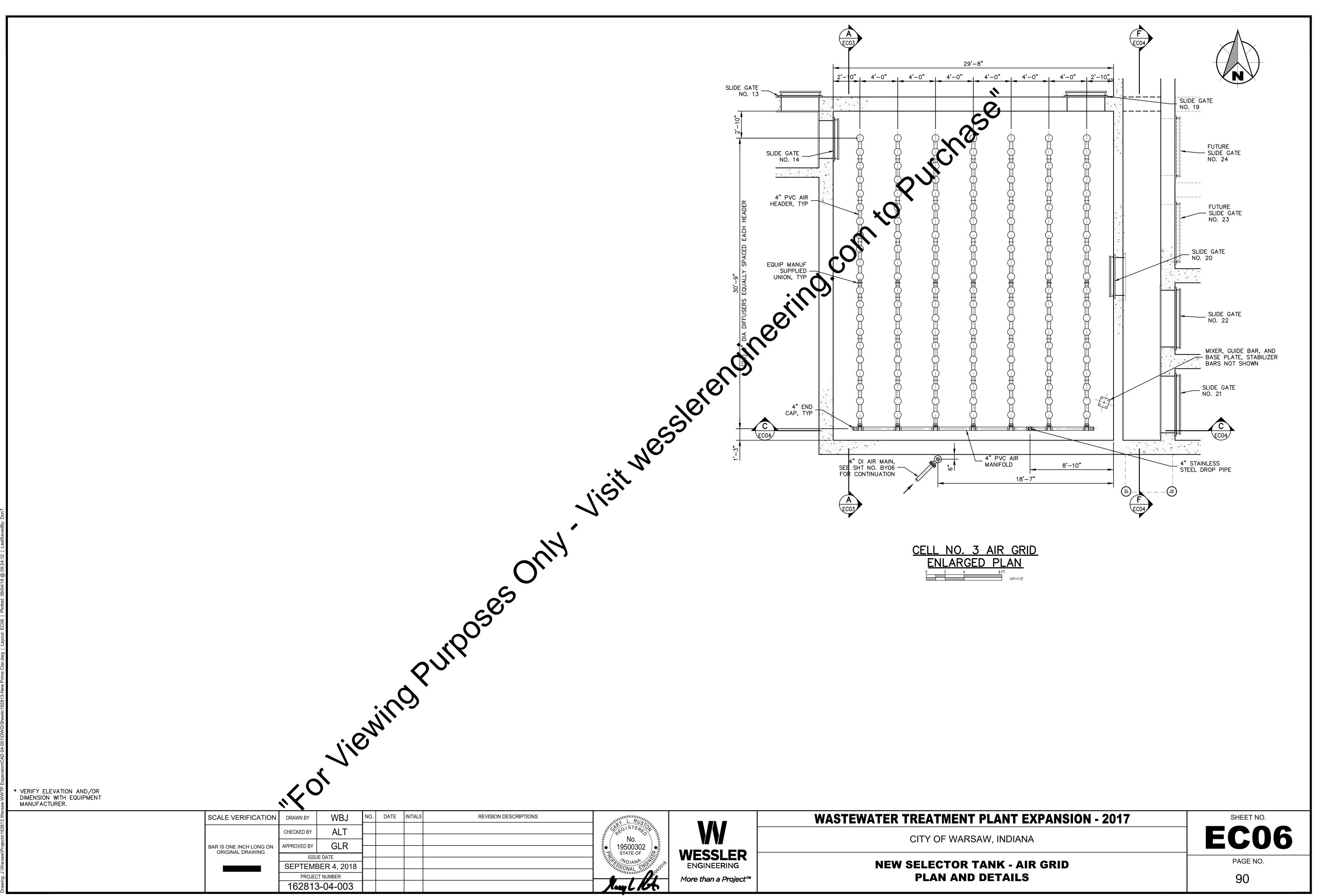












**OXIDATION DITCH GENERAL NOTES:** 

THE TOP OF THE LOWER WALL ELEVATION FOR THE OXIDATION DITCH HAS BEEN SHOWN AT 821.50, AND THE TOP OF WALL ELEVATION FOR GATE CHAMBER NO. 1 HAS BEEN SHOWN AT 821.00, ON THE 2001 WASTEWATER TREATMENT PLANT PROJECT, BY JONES & HENRY ENGINEERS, LTD. THE WESSLER ENGINEERING SITE SURVEY FOR THIS PROJECT DETERMINED THE TOP OF WALL ELEVATION TO BE 820.75 FOR THE OXIDATION DITCHES AND 820.25 FOR GATE CHAMBER NO. 1. ALL VERTICAL INFORMATION SHOWN FOR THIS STRUCTURE HAS BEEN REVISED TO AGREE WITH THE SITE SURVEY, WITH THE ASSUMPTION THAT VERTICAL DISTANCES REFERENCED ON THE JONES & HENRY DRAWINGS ARE ACCURATE.

ALT

GLR

ISSUE DATE

SEPTEMBER 4, 2018

PROJECT NUMBER

162813-04-003

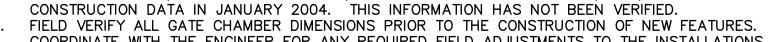
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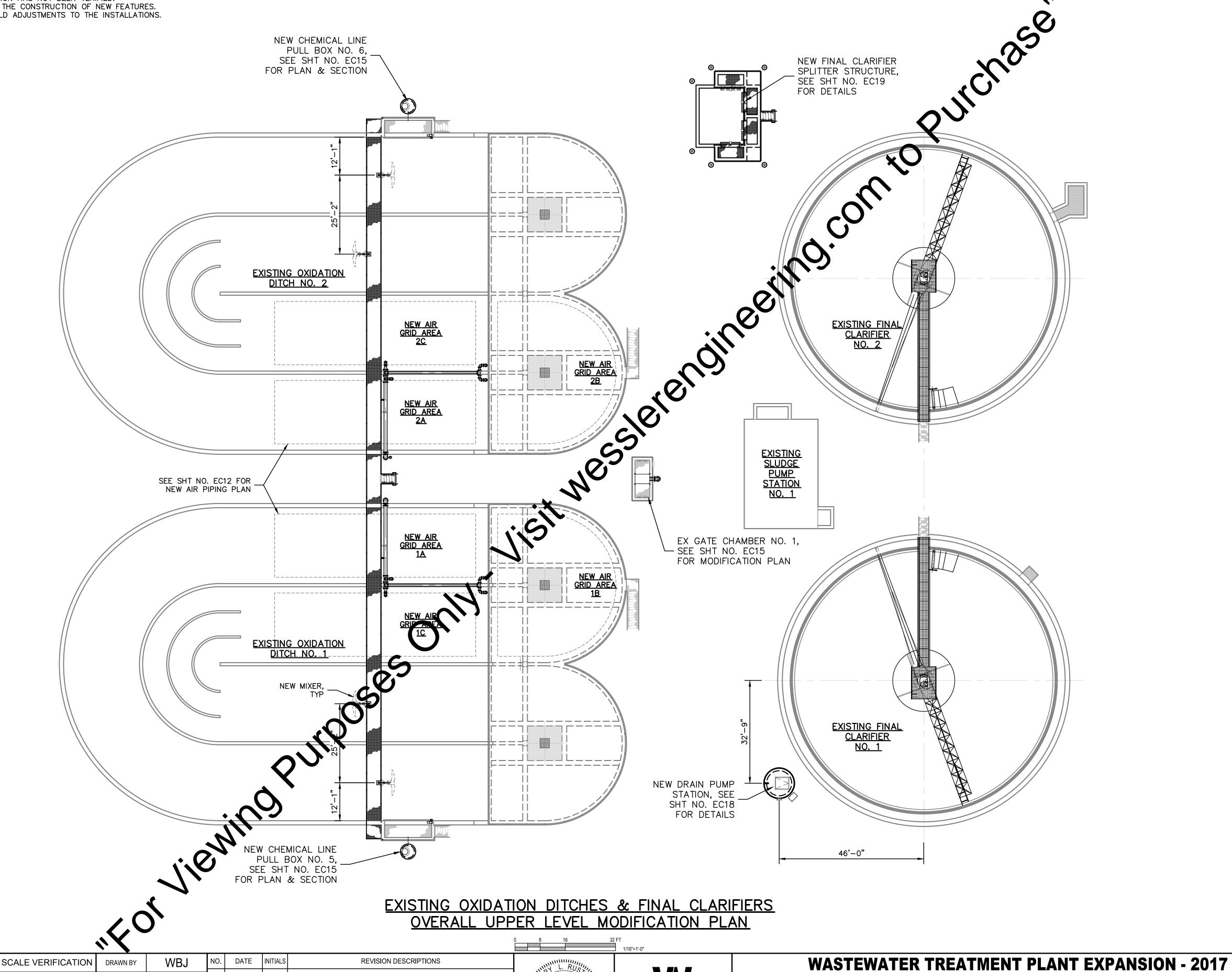
BAR IS ONE INCH LONG ON

ORIGINAL DRAWING

- 2. EXISTING BELOW GRADE PIPING SHOWN ON THE DRAWINGS FOR AREA D IS AS SHOWN ON THE WASTEWATER TREATMENT PLANT, CONTRACT 9 PROJECT, BY JONES & HENRY ENGINEERS, LTD., CORRECTED FROM
- 3. FIELD VERIFY ALL GATE CHAMBER DIMENSIONS PRIOR TO THE CONSTRUCTION OF NEW FEATURES. COORDINATE WITH THE ENGINEER FOR ANY REQUIRED FIELD ADJUSTMENTS TO THE INSTALLATIONS.







No. 19500302 STATE OF

**WESSLER** 

**ENGINEERING** 

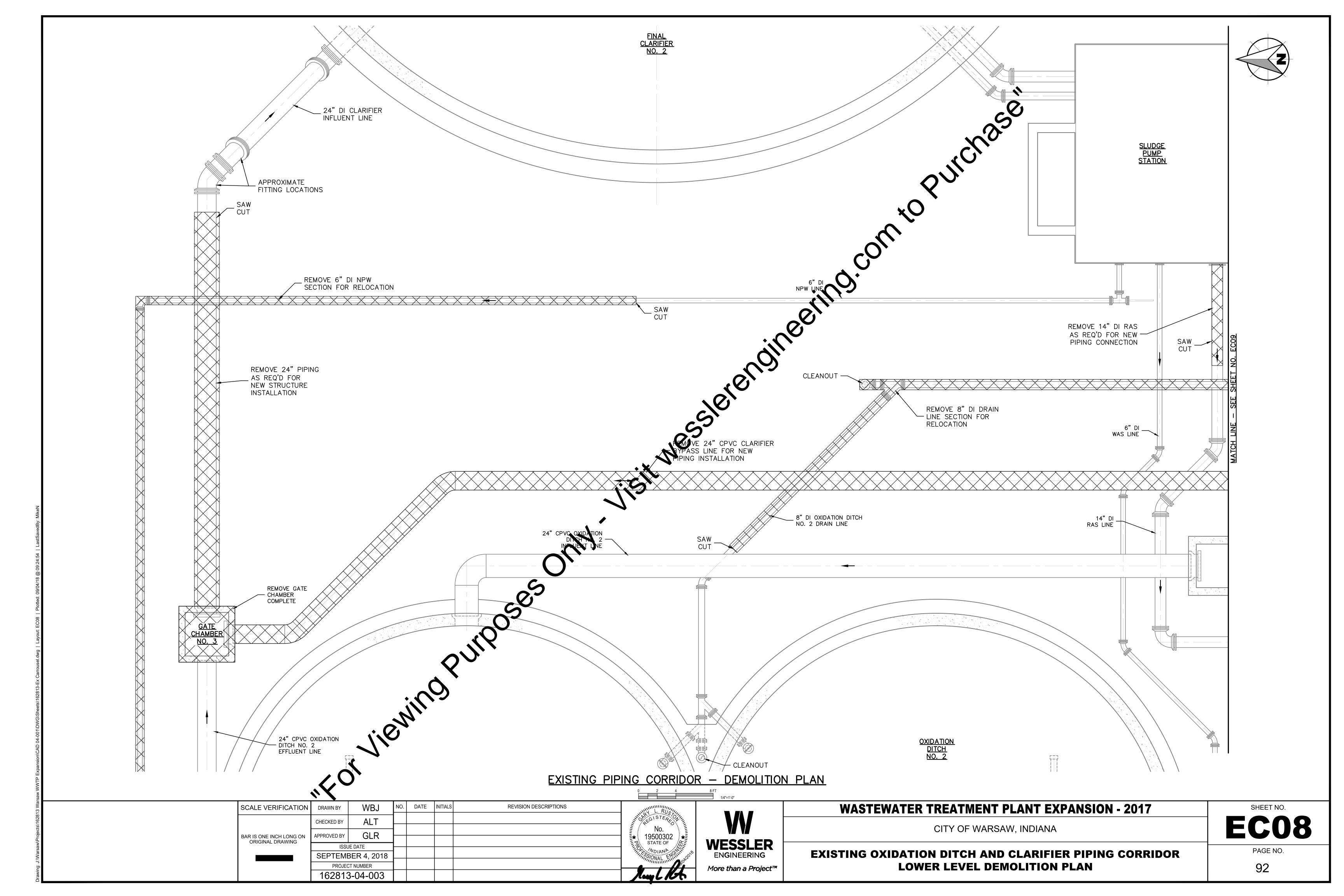
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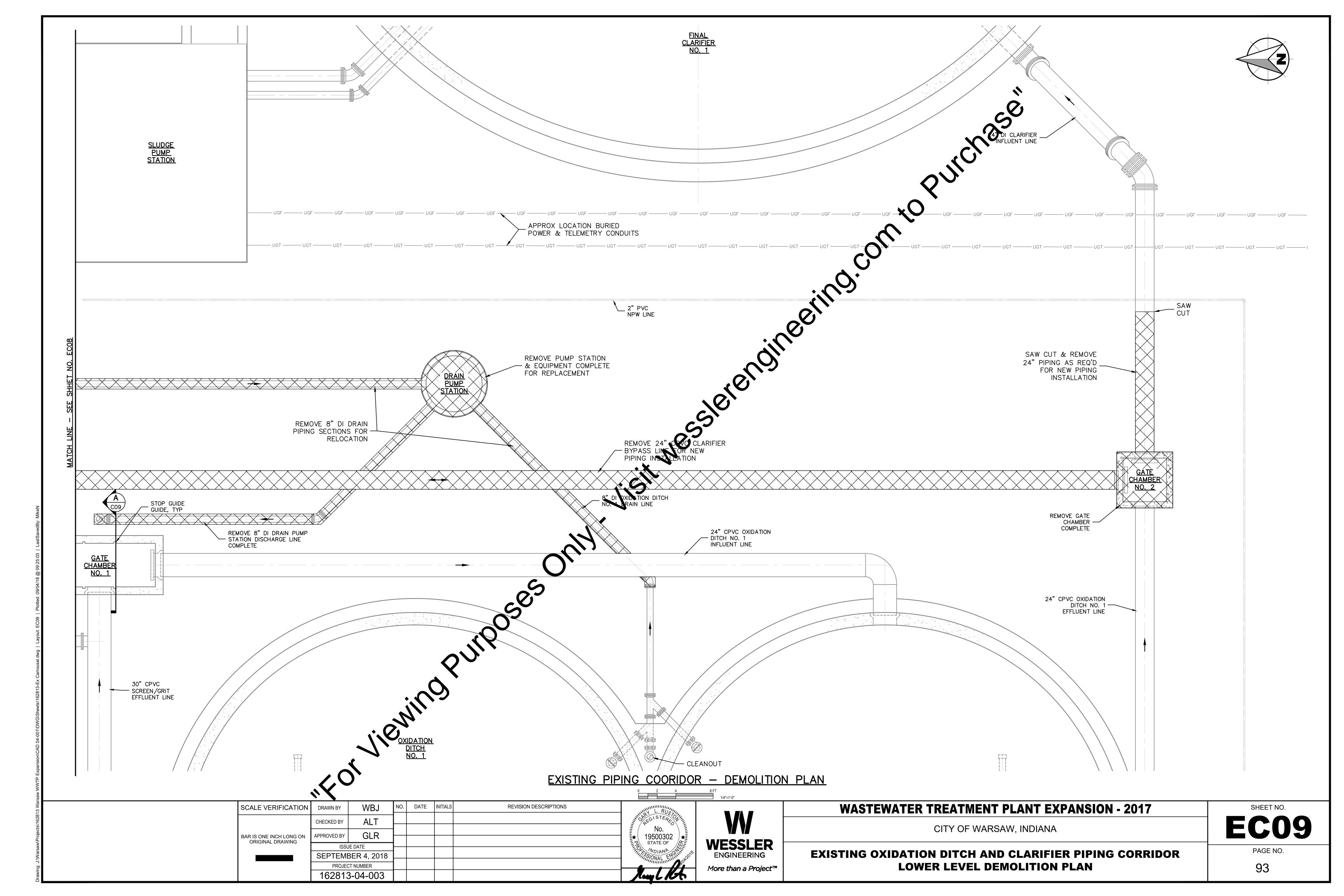
EC07 CITY OF WARSAW, INDIANA

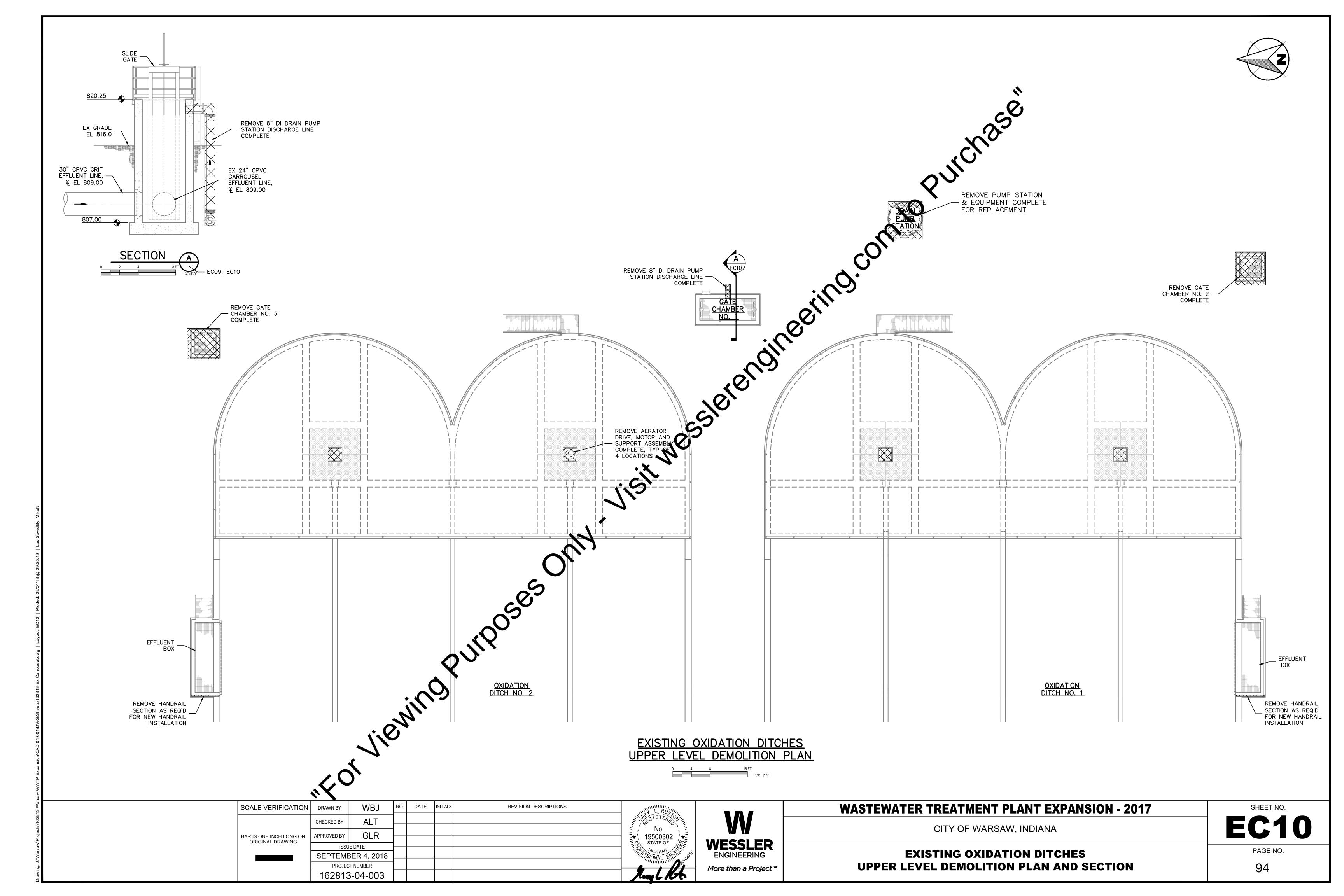
**EXISTING OXIDATION DITCHES AND FINAL CLARIFIERS OVERALL UPPER LEVEL MODIFICATION PLAN** 

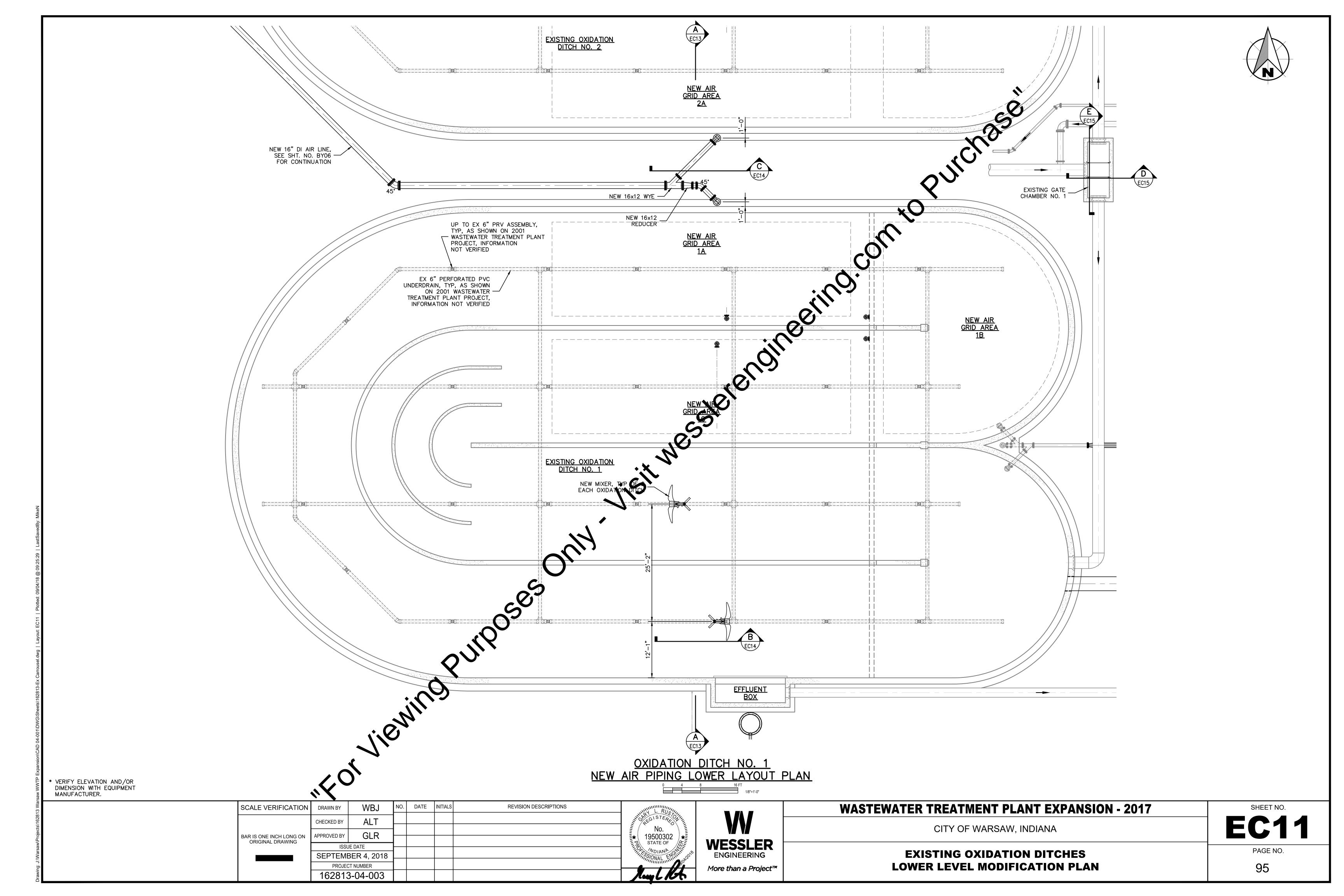
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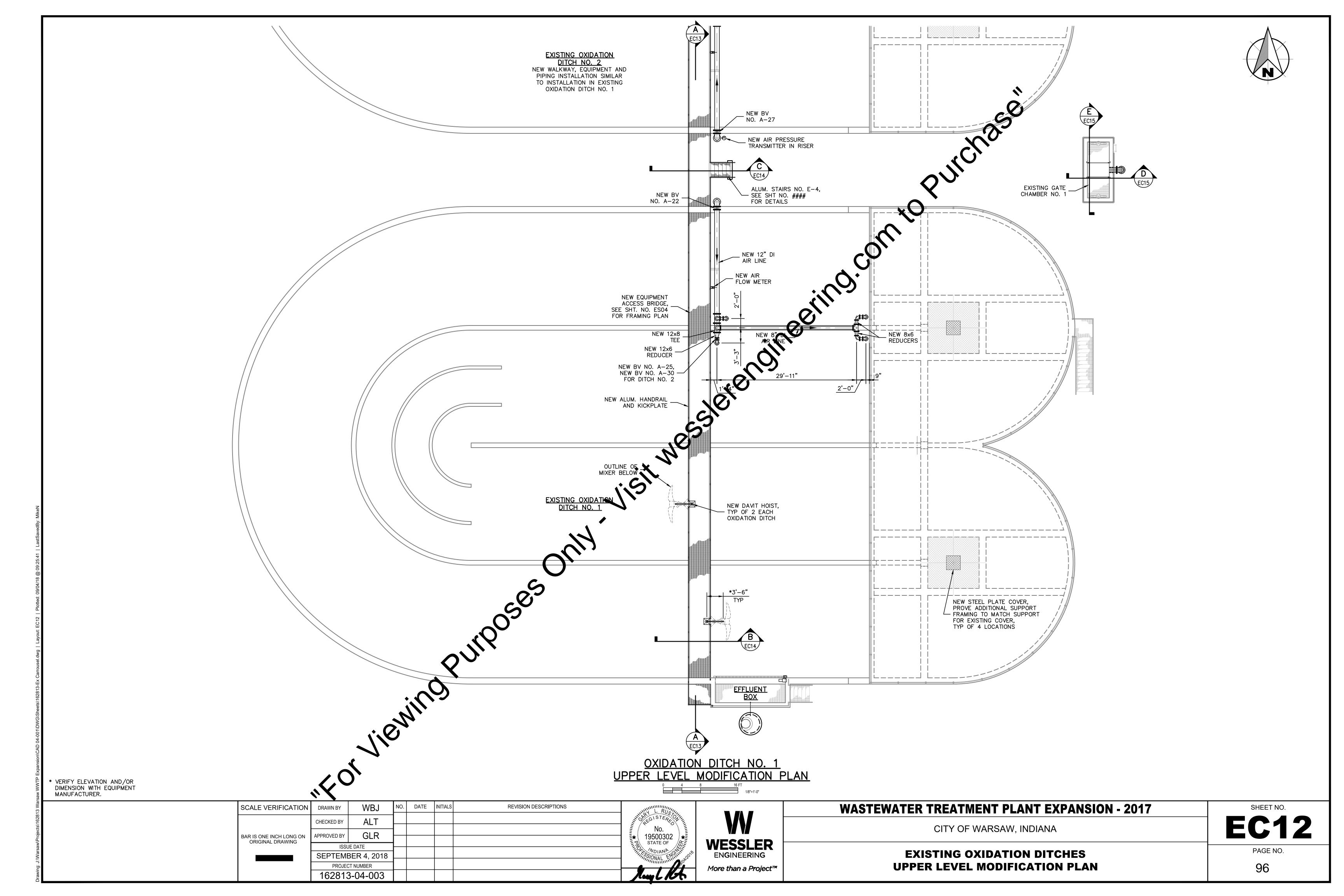
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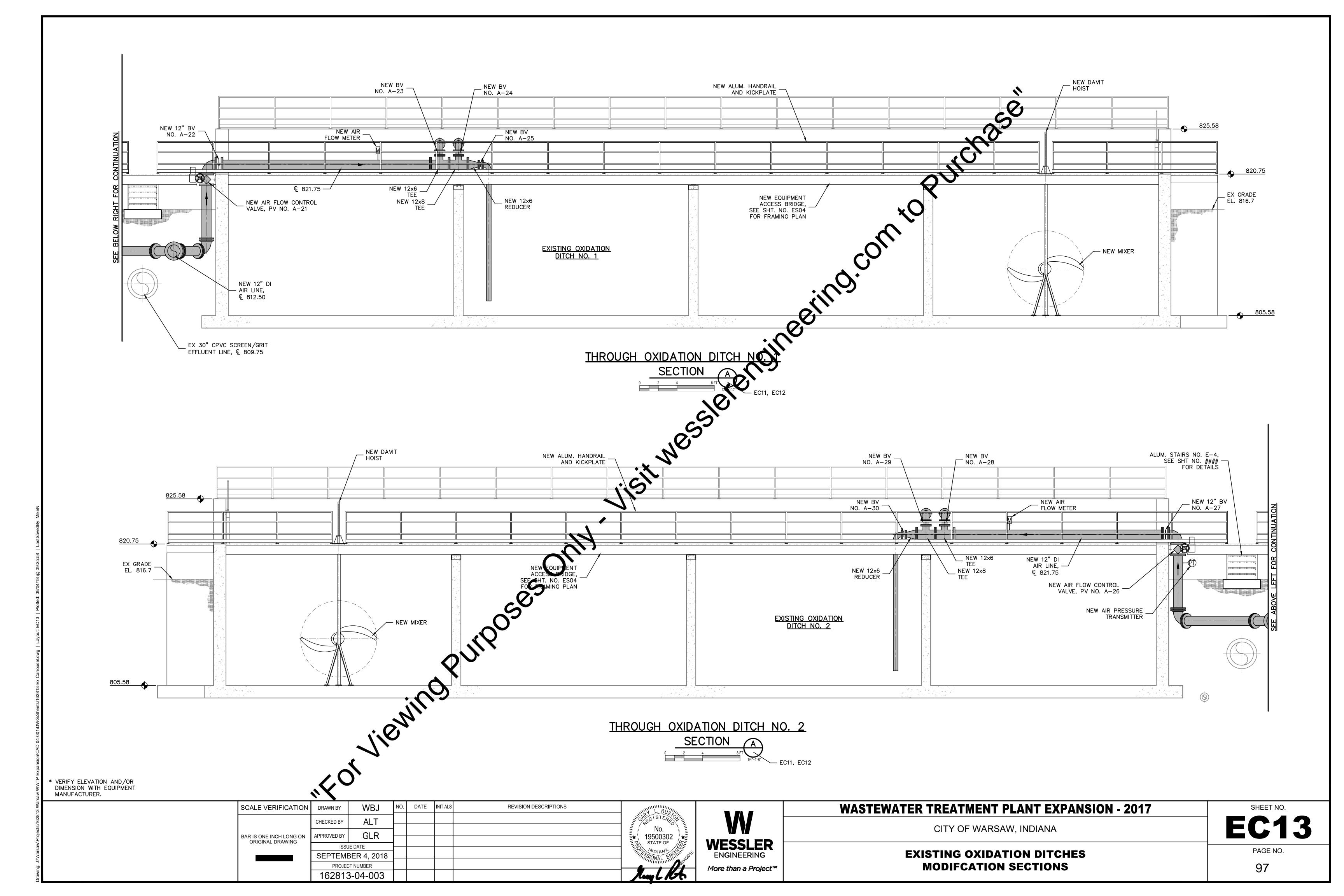


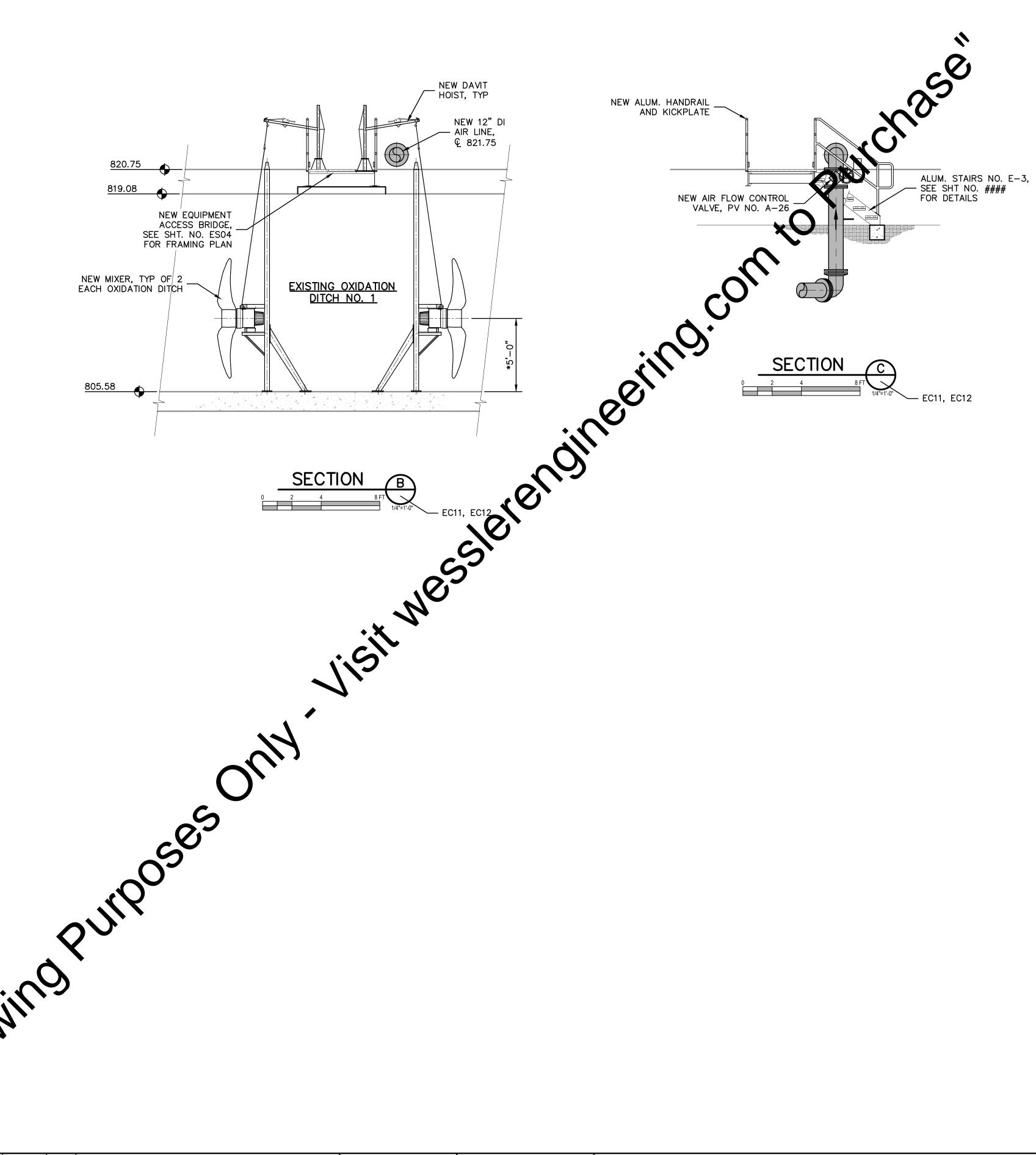












\* VERIFY ELEVATION AND/OR DIMENSION WITH EQUIPMENT MANUFACTURER.

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CHECKED BY ALT

APPROVED BY GLR

ISSUE DATE

SEPTEMBER 4, 2018

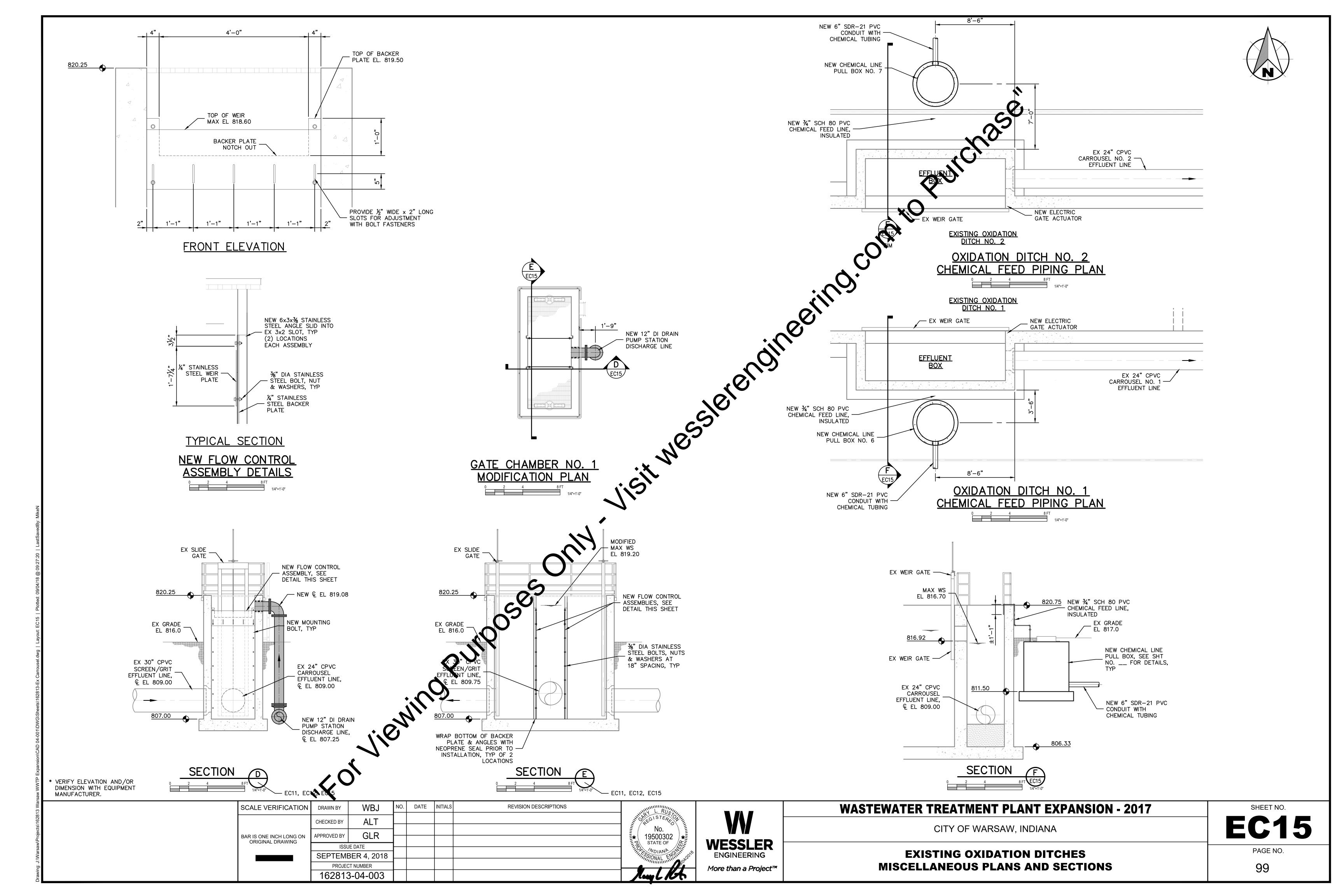
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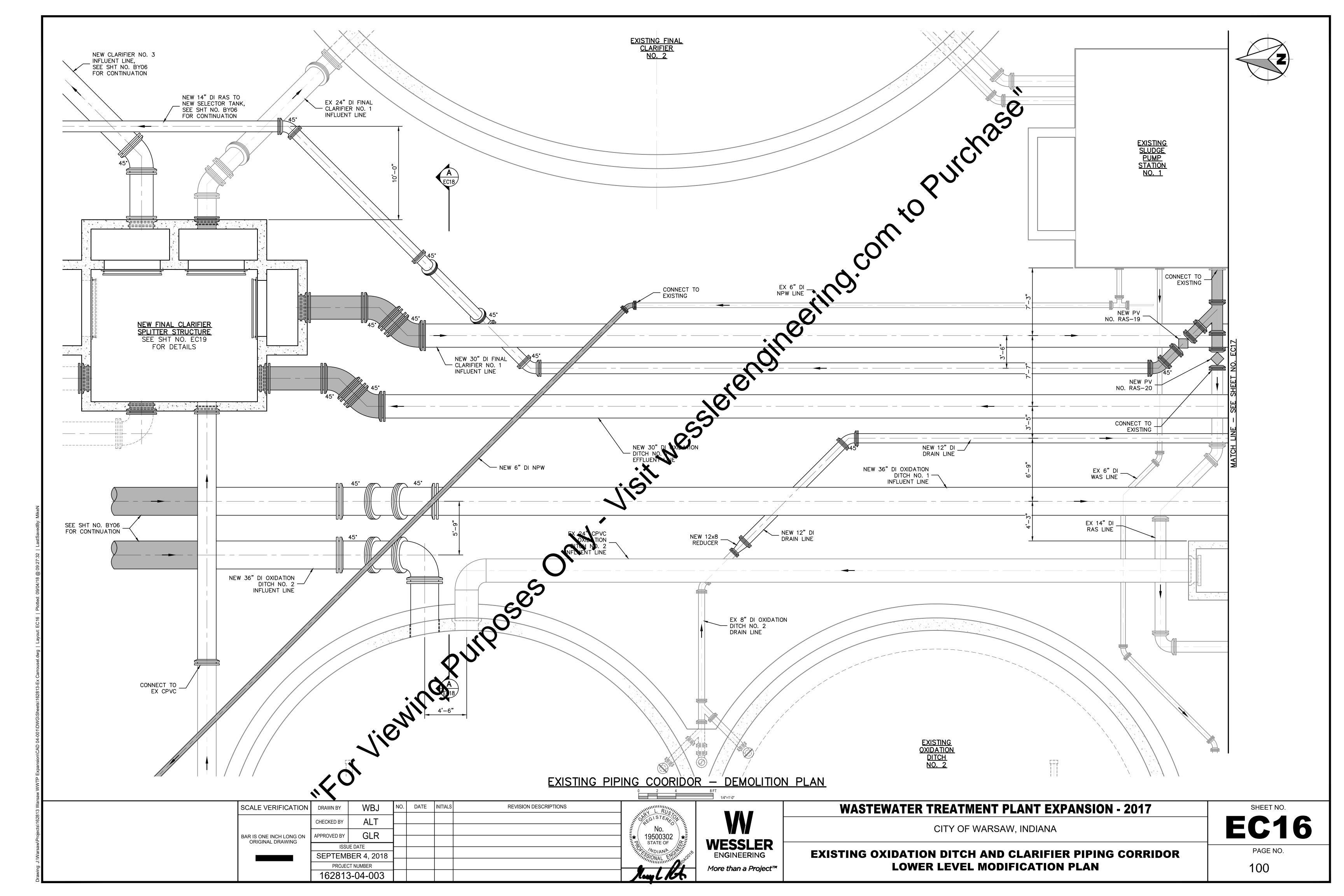
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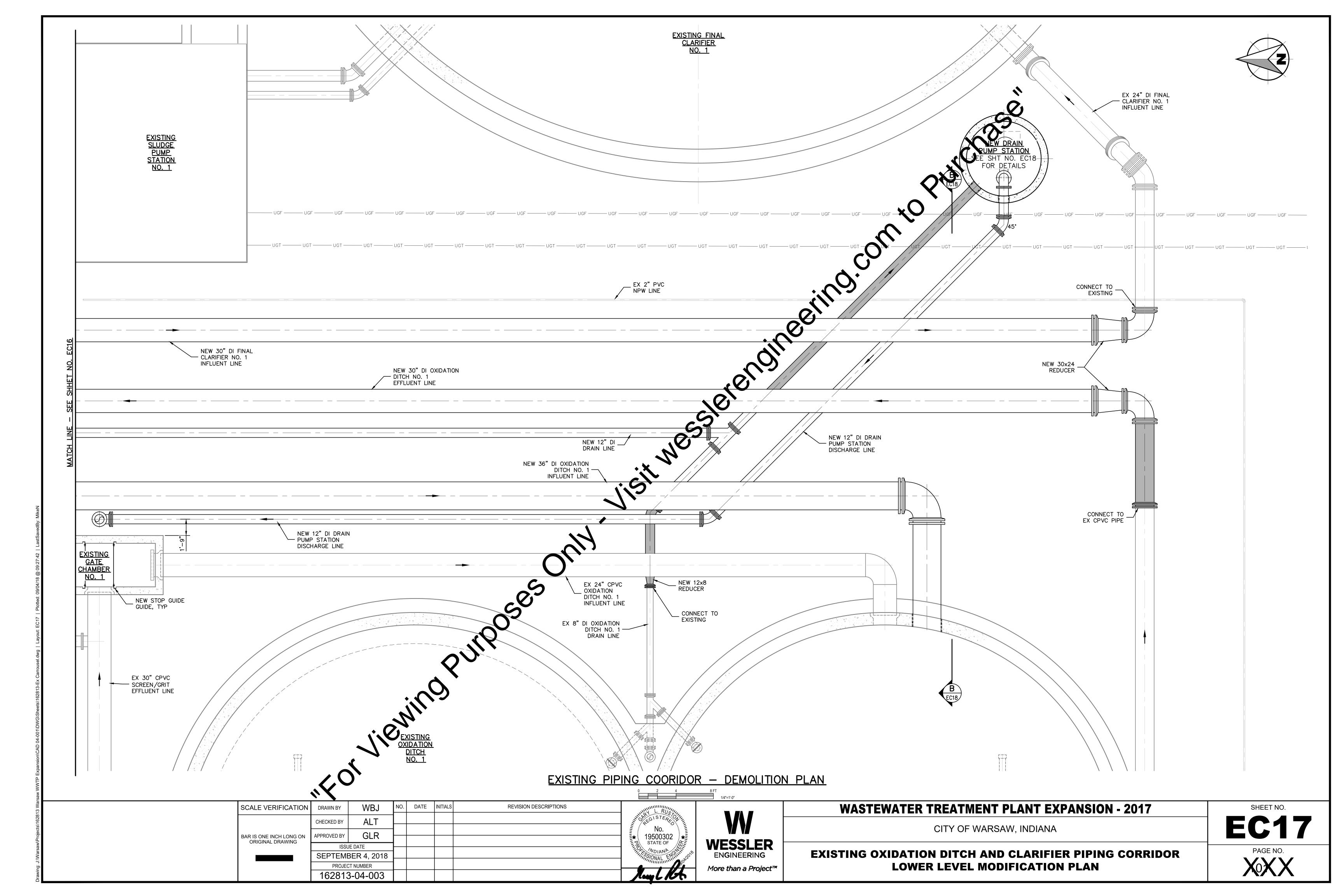


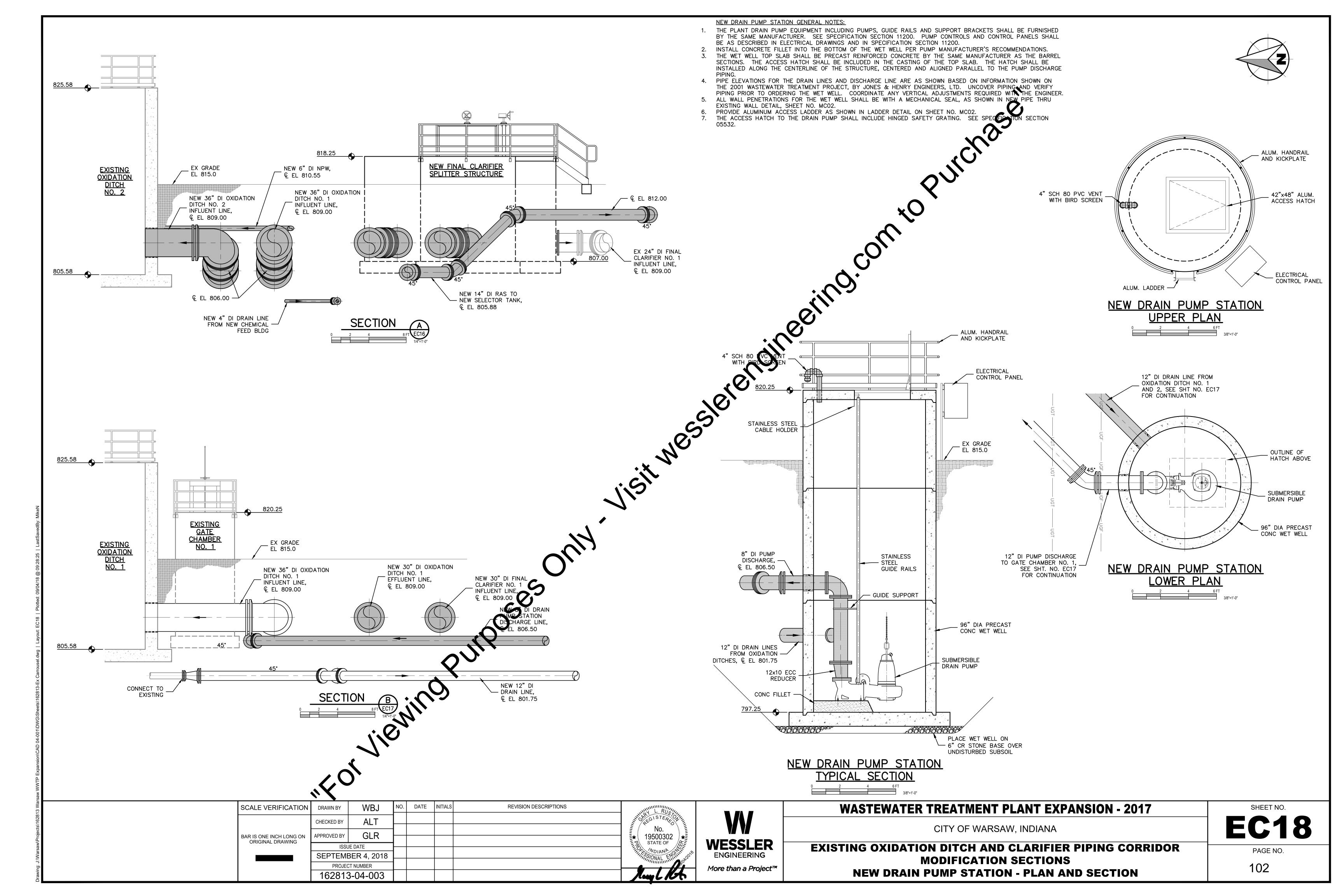


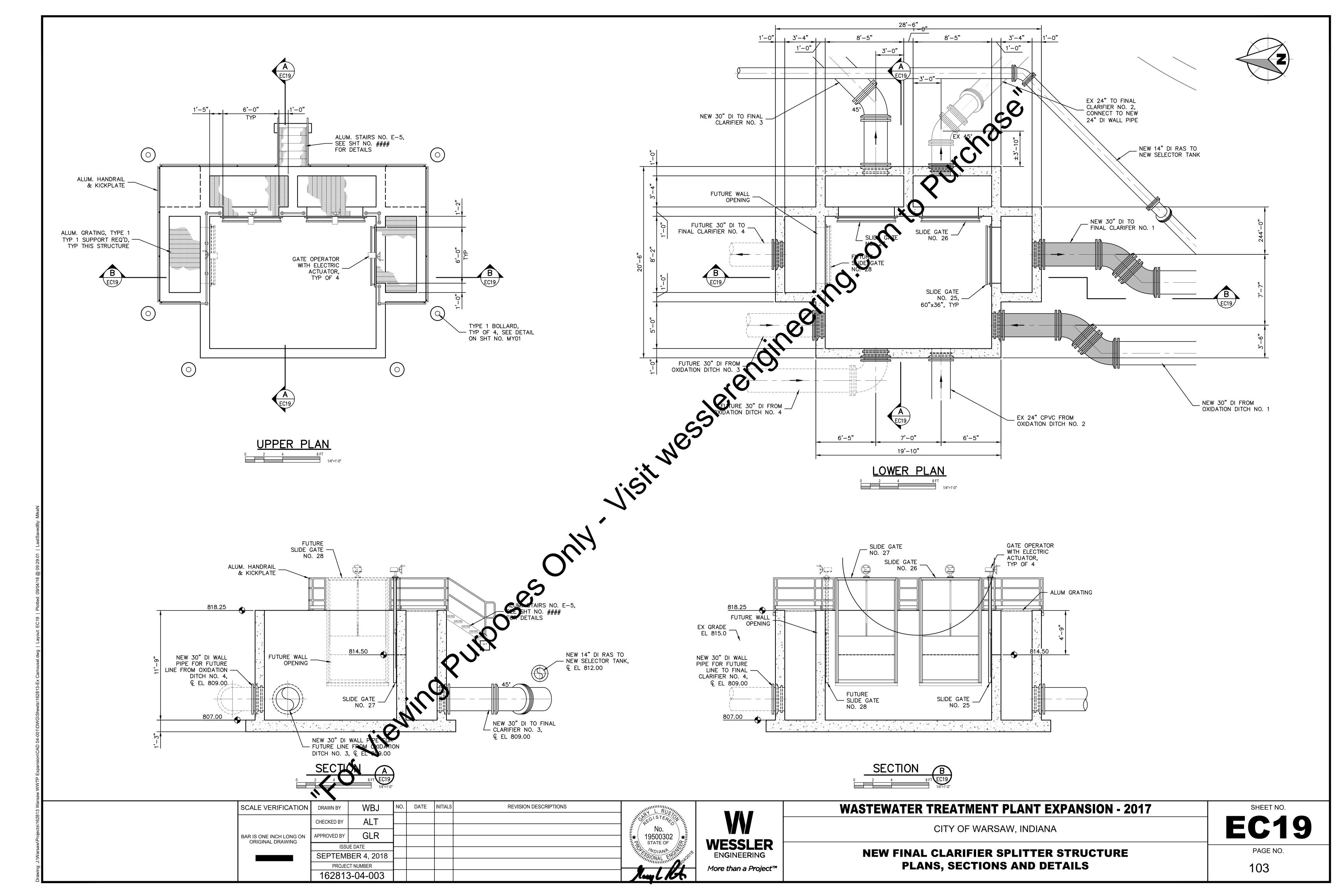
WASTEWATER TREATMENT PLANT EXPANSION - 2017	SHEET NO.
CITY OF WARSAW, INDIANA	EC1
EXISTING OXIDATION DITCHES	PAGE NO.
NEW AIR PIPING SECTIONS	98

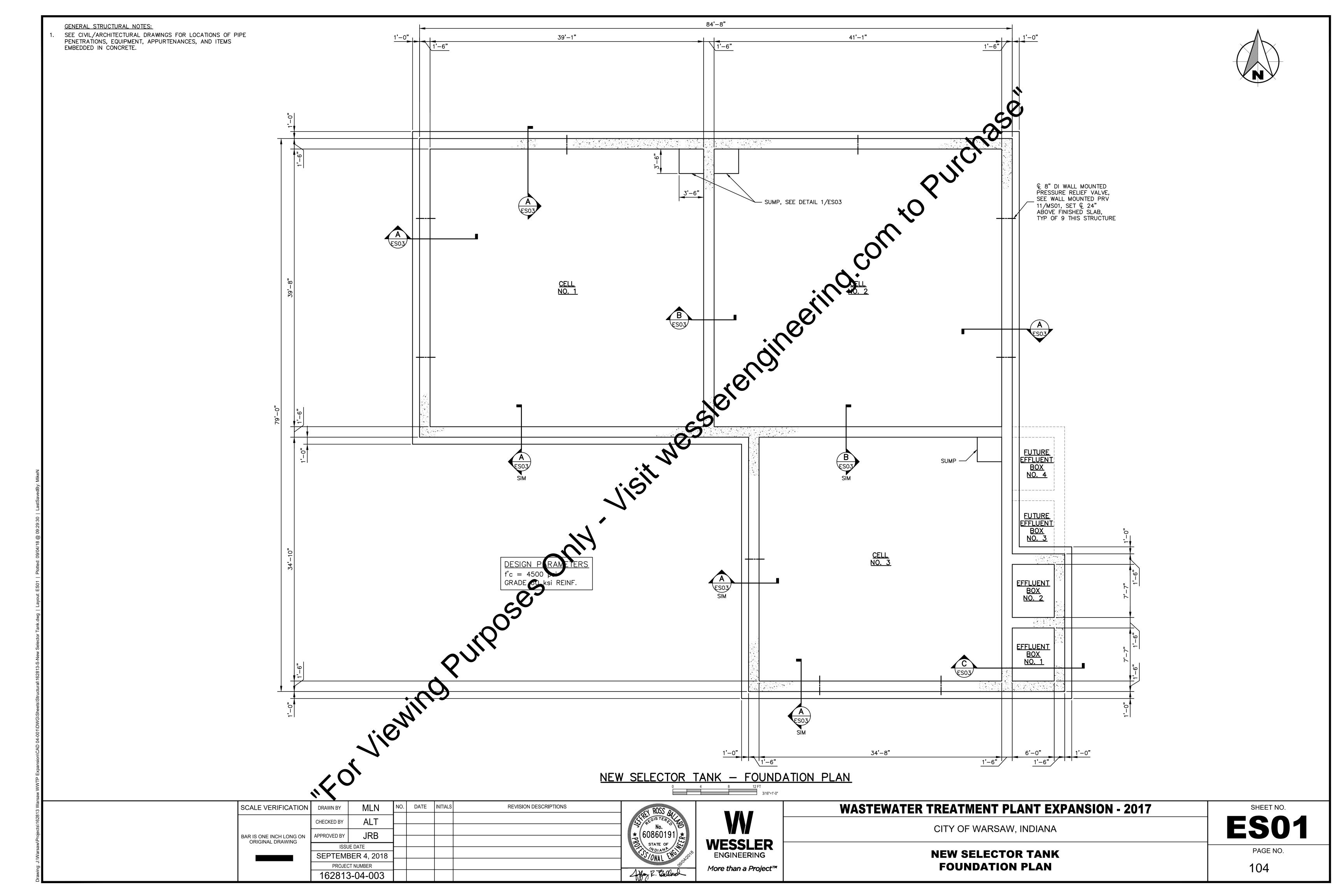


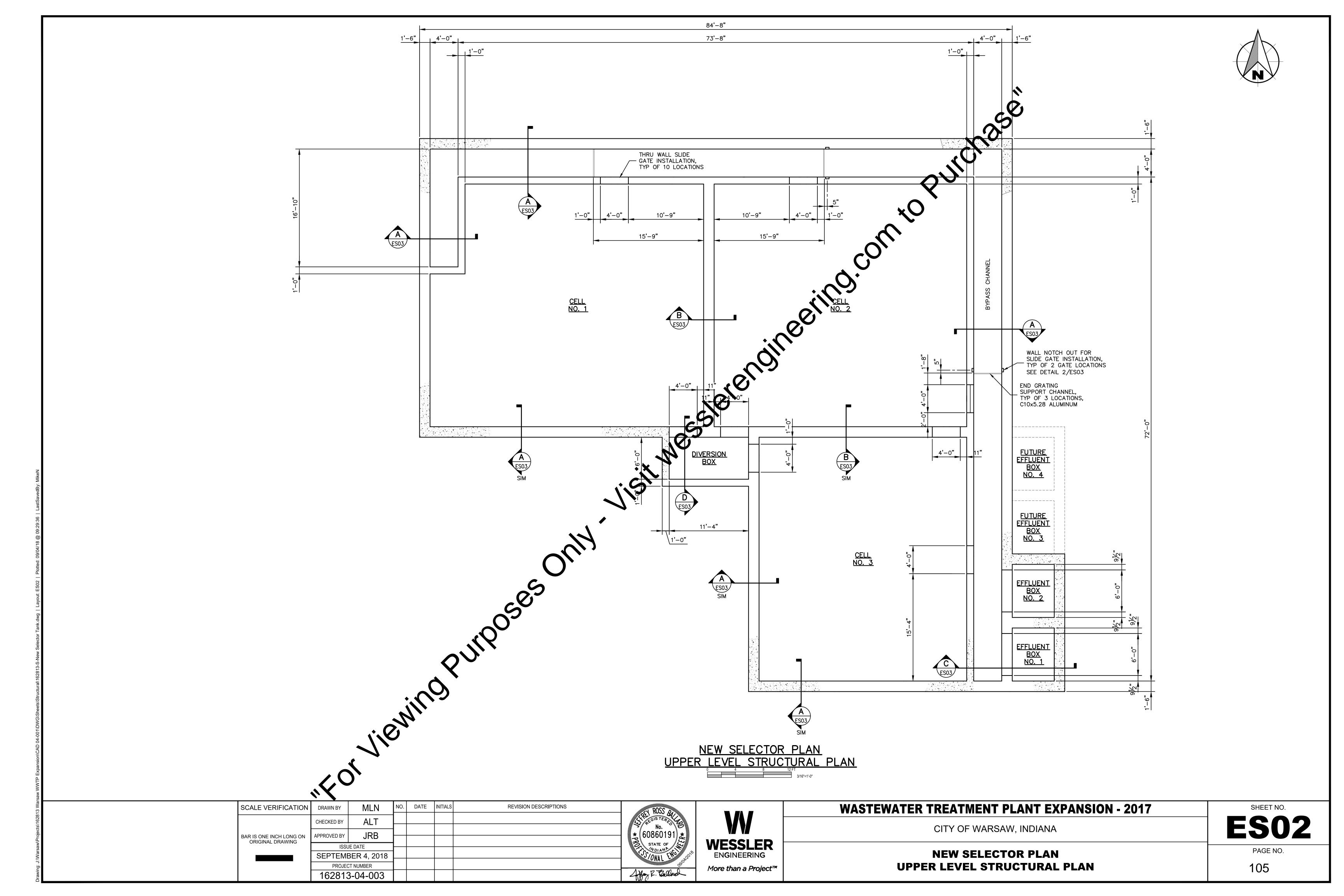


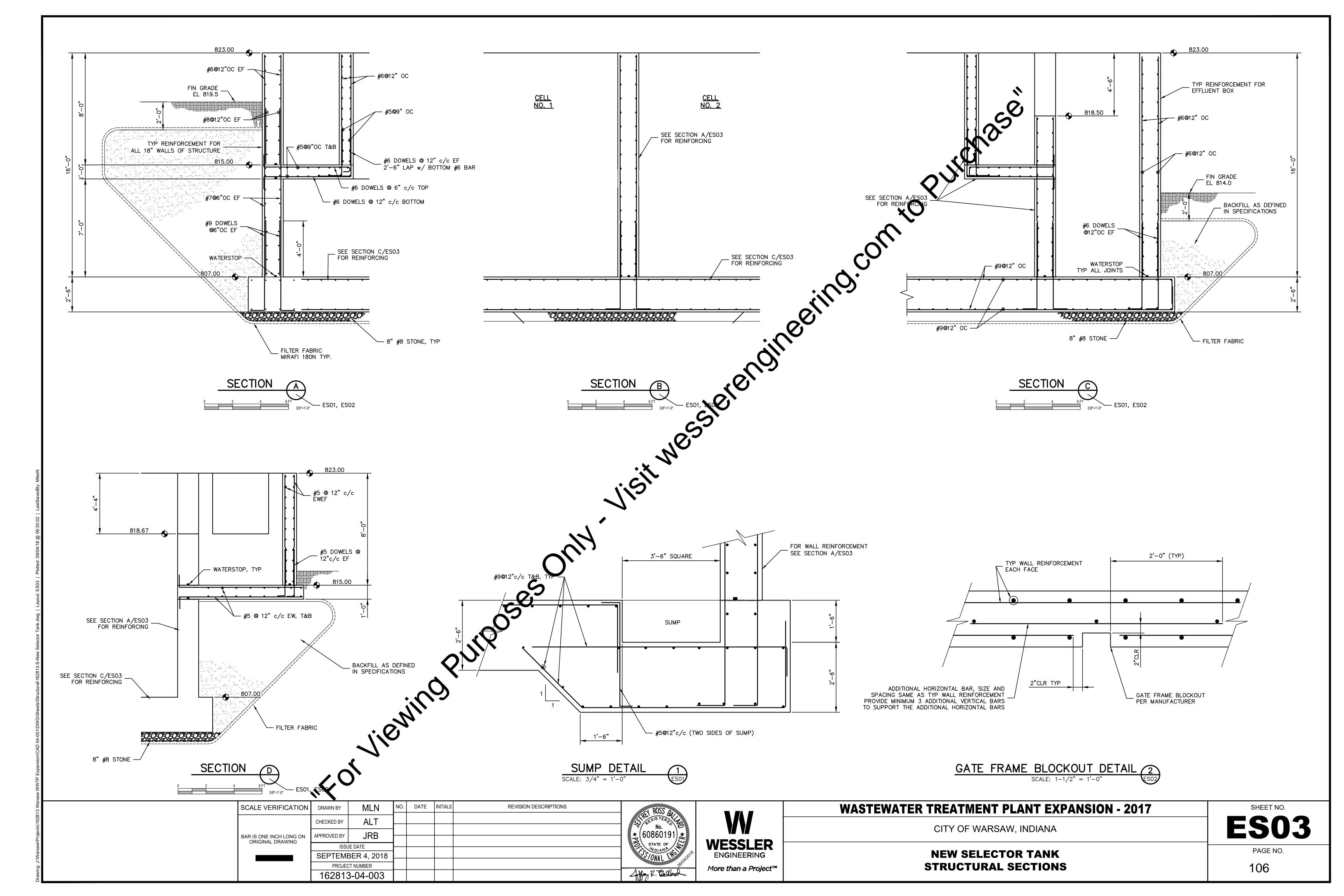


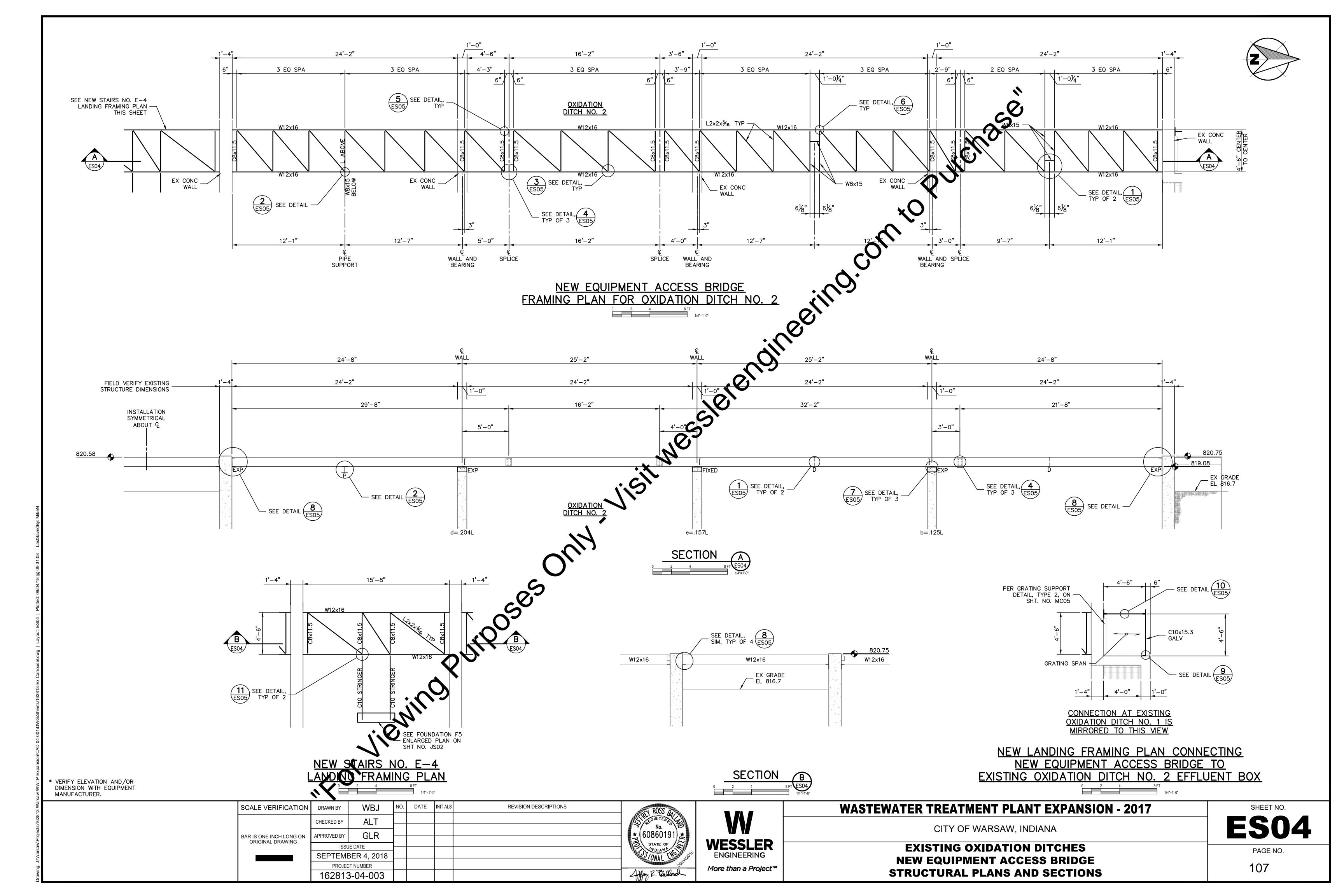


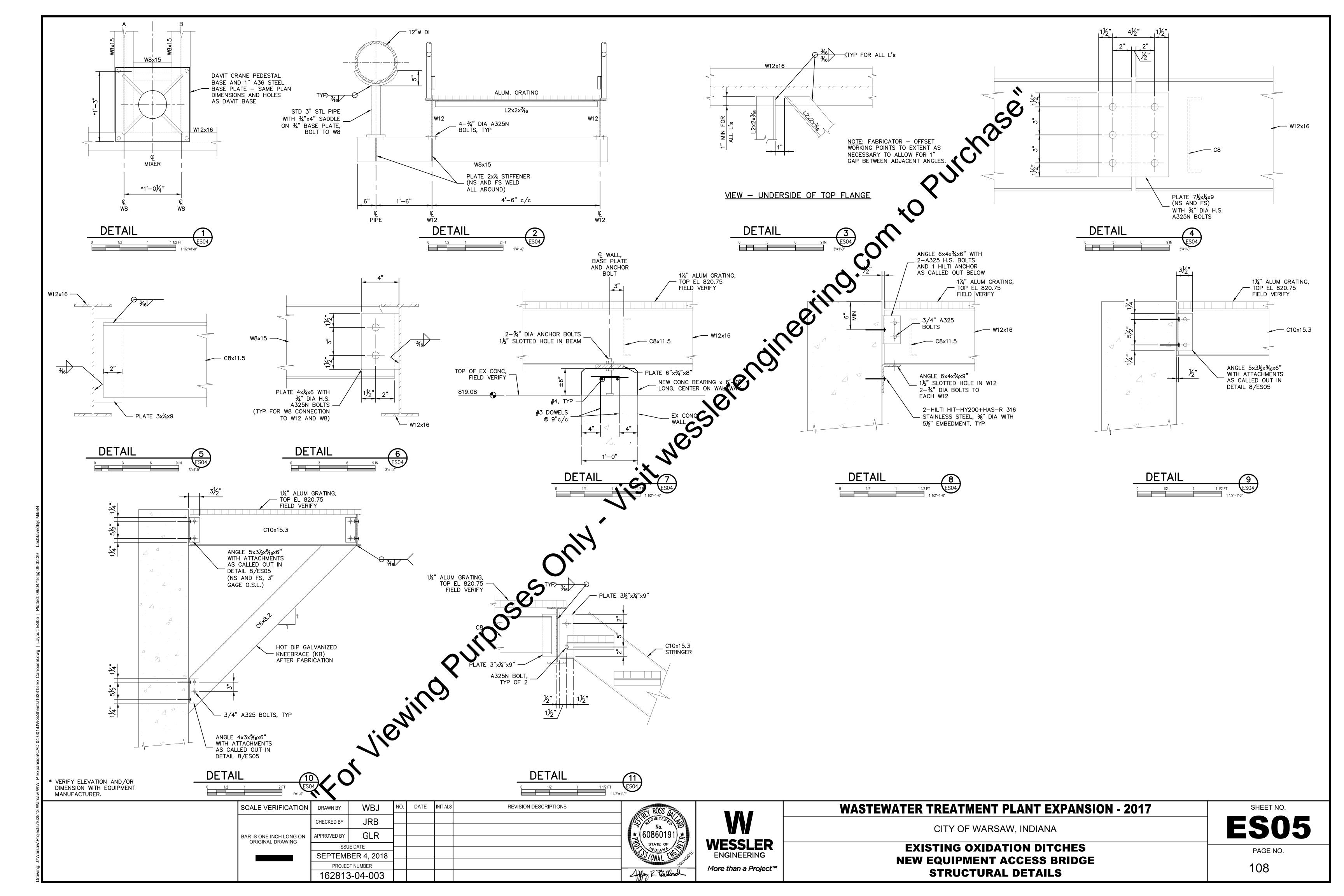


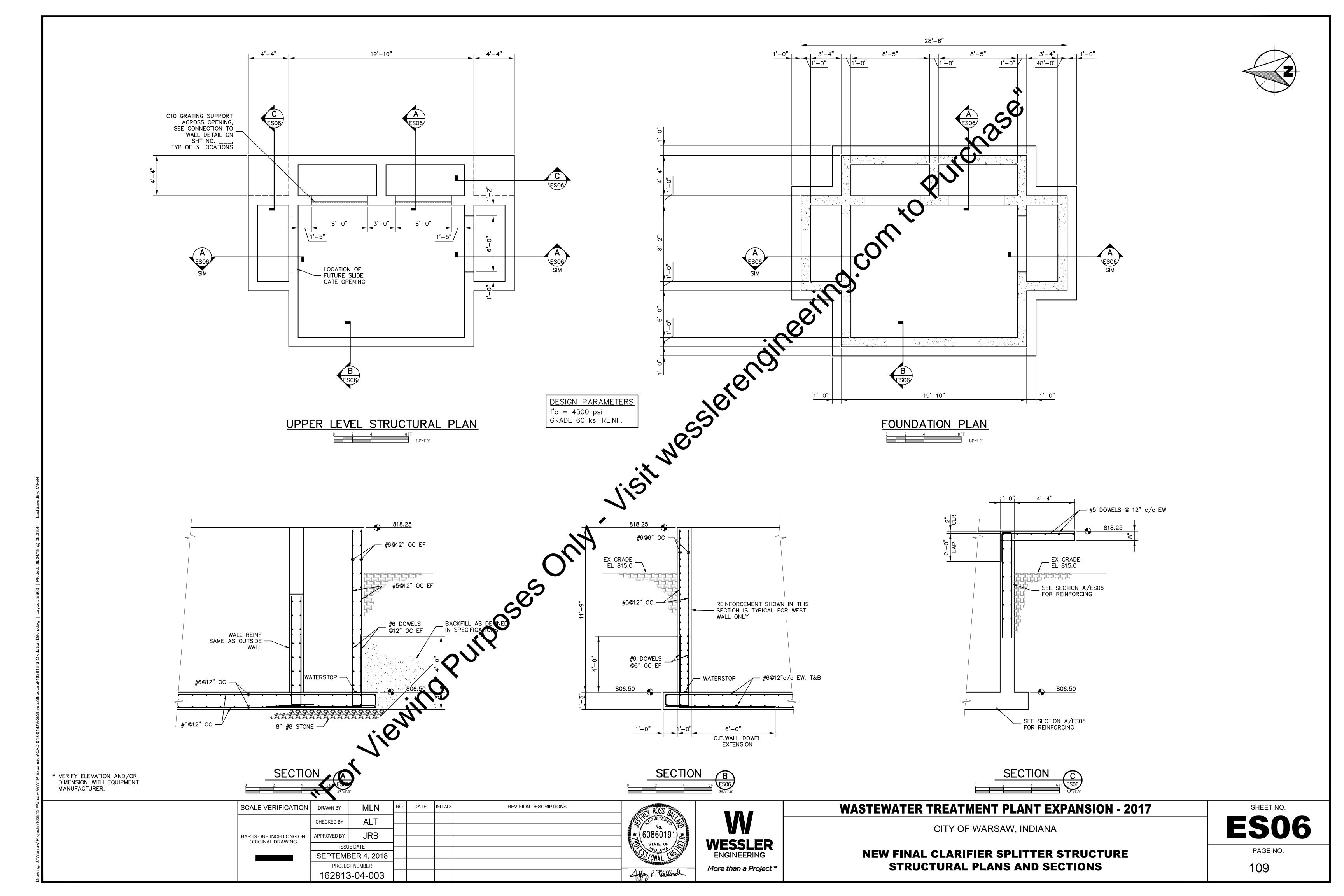


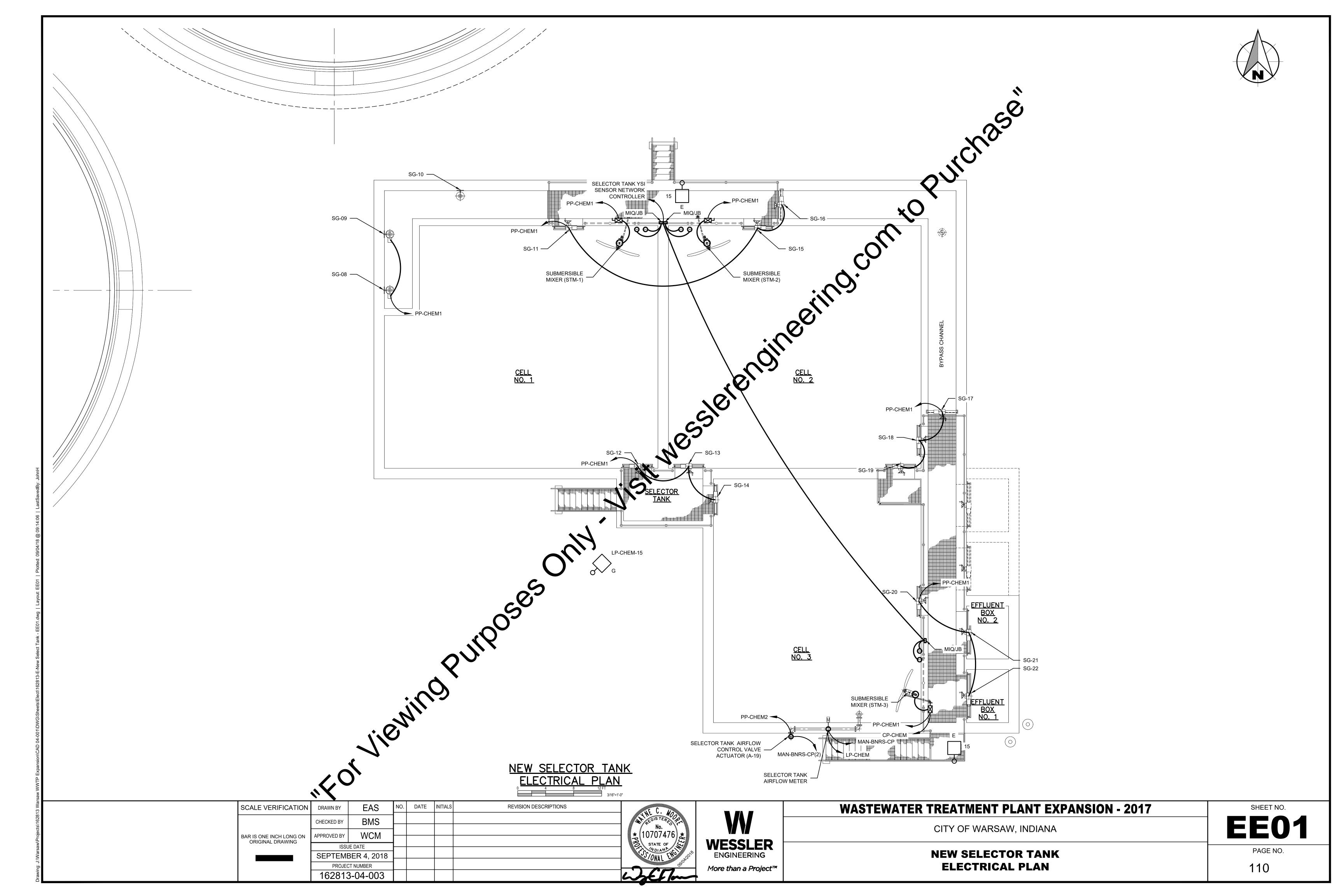


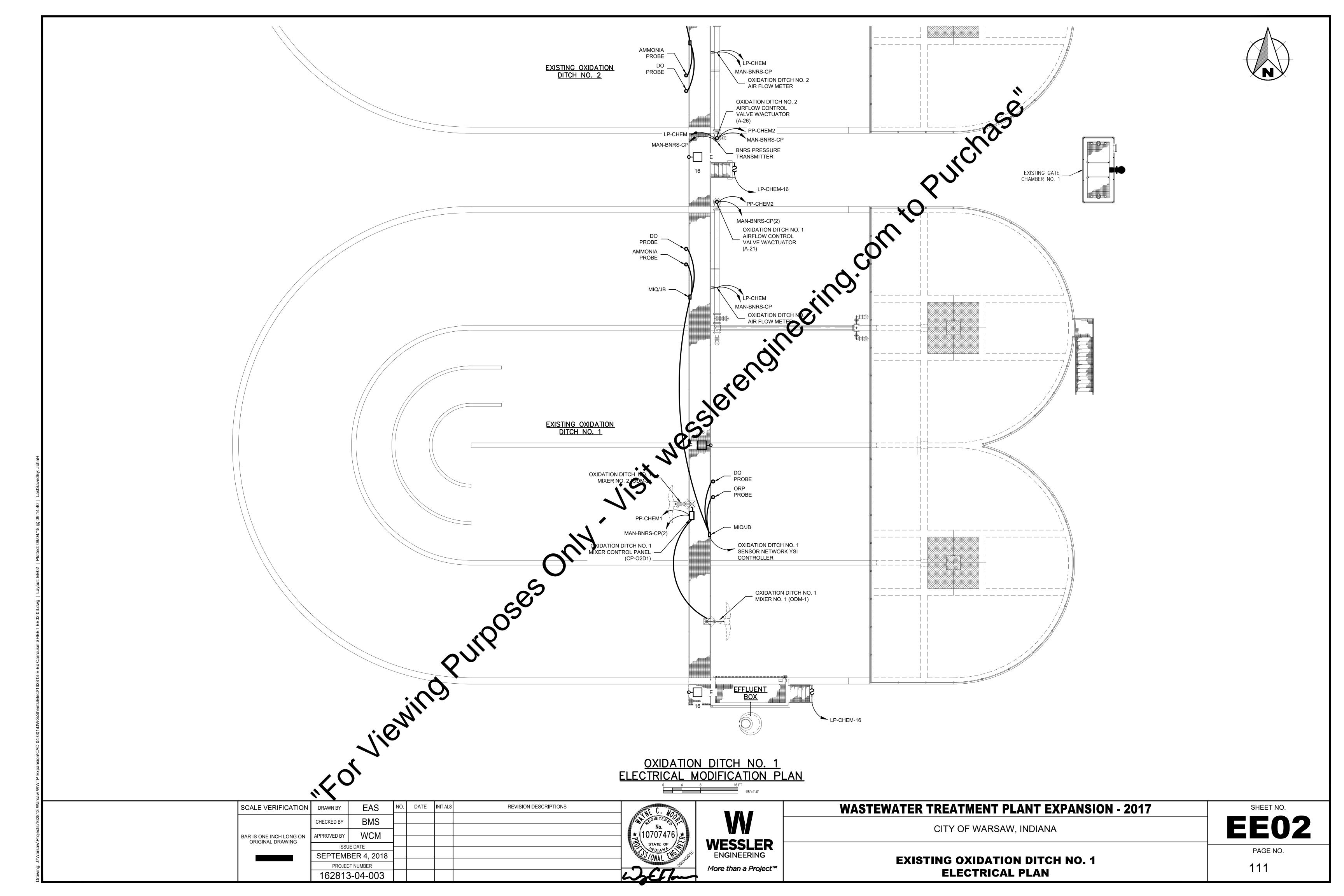


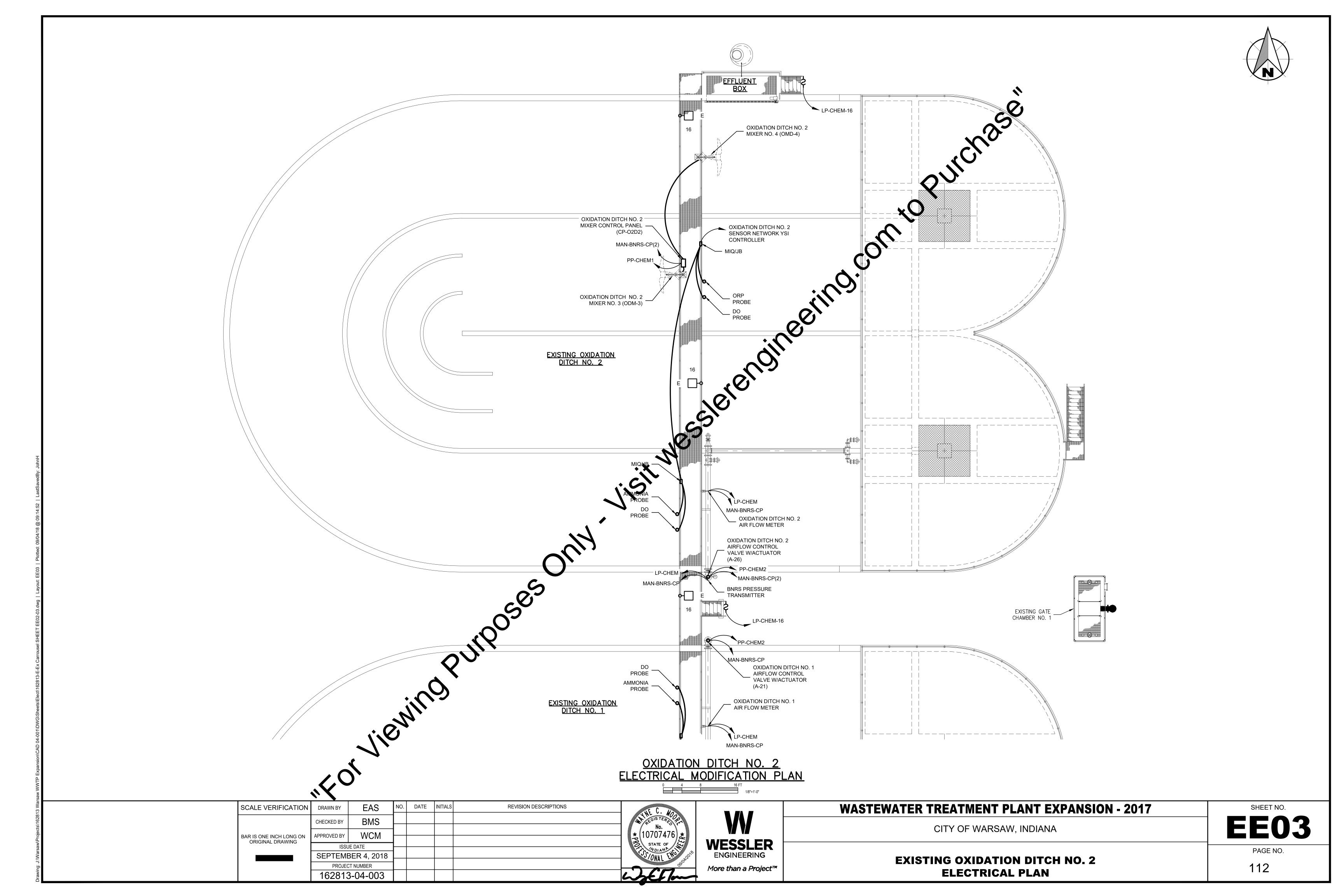




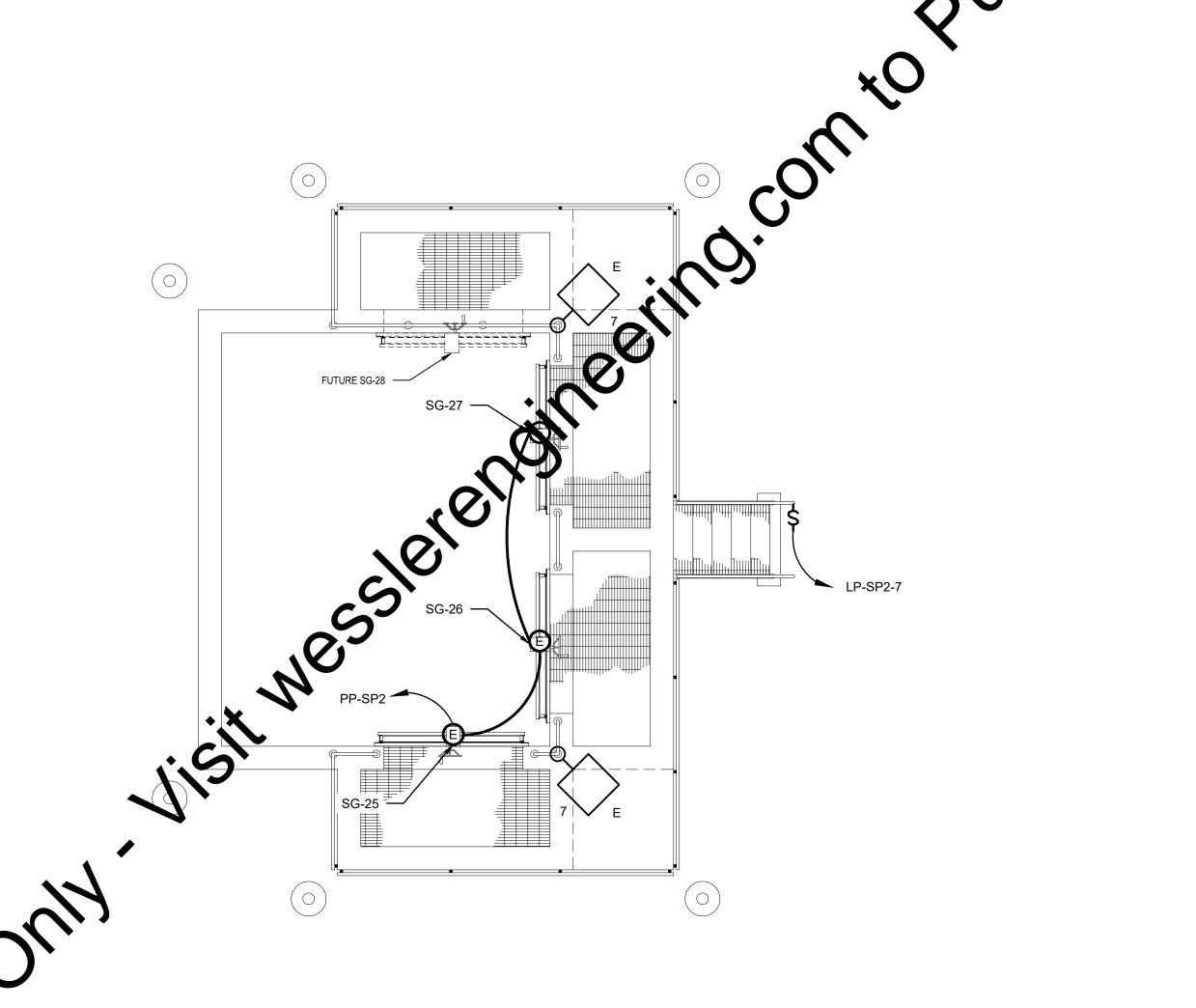










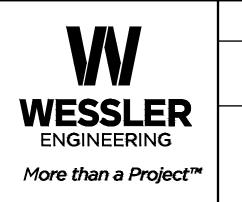


NEW FINAL INFLUENT SPLITTER BOX

ELECTRICAL PLAN

1/4'=1'-0'

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<b>WASTEWATER</b>	TREATMENT	<b>PLANT</b>	<b>EXPANSION - 2017</b>	
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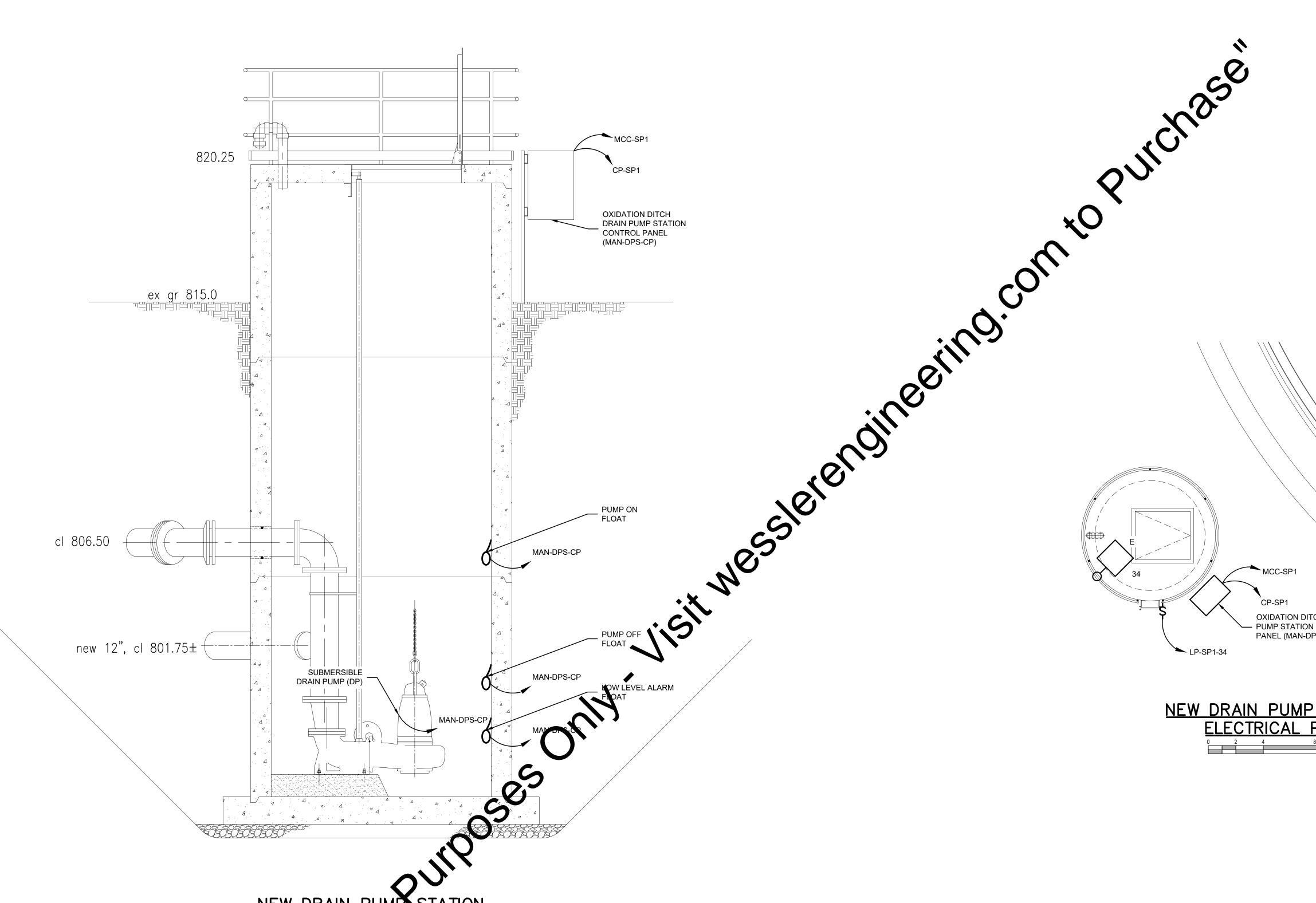
CITY OF WARSAW, INDIANA

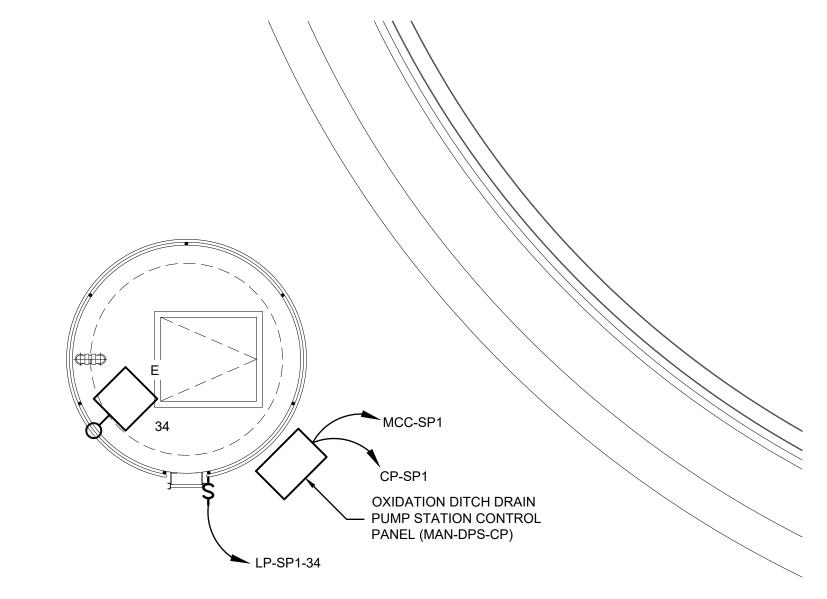
NEW FINAL INFLUENT SPLITTER STRUCTURE
ELECTRICAL PLAN

PAGE NO.

113







NEW DRAIN PUMP STATION
ELECTRICAL PLAN

1. SEE C SHEETS FOR FLOAT MOUNTING ELEVATION.

NEW DRAIN PUMP STATION ELECTRICAL SECTION PLAN

	1. /						
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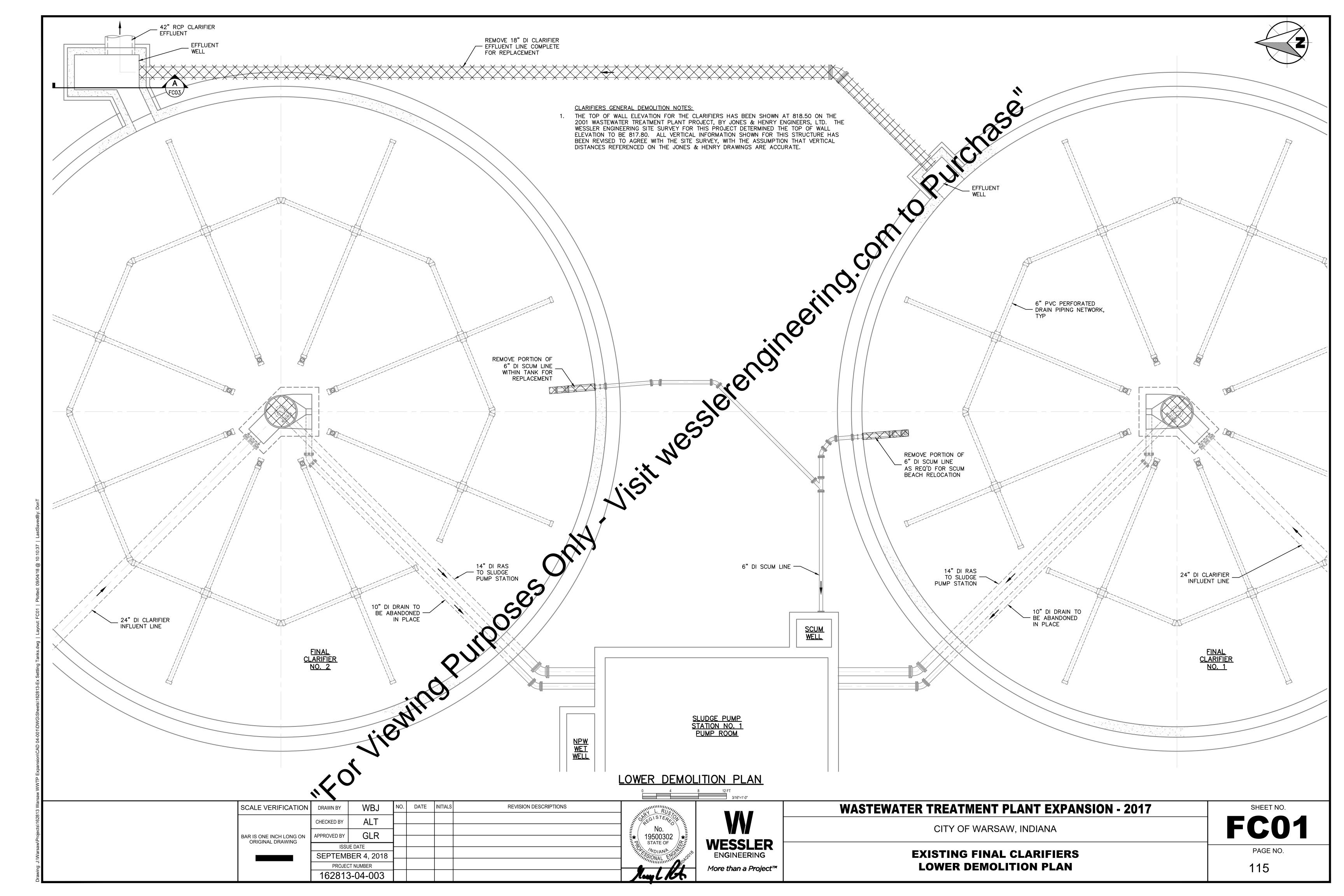


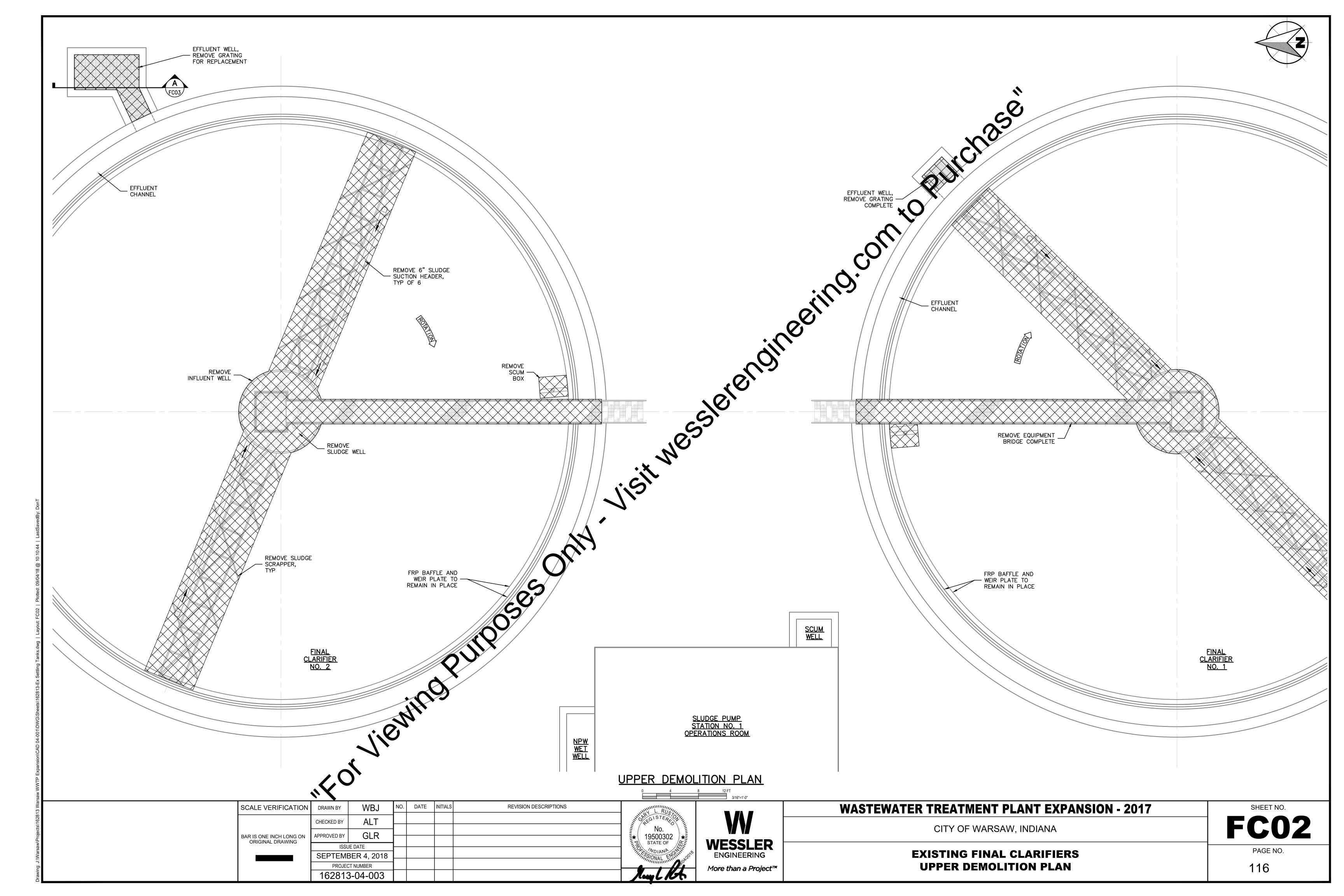
WASTEWATER TREATMENT PLANT EXPANSION - 2017	
CITY OF WARSAW, INDIANA	

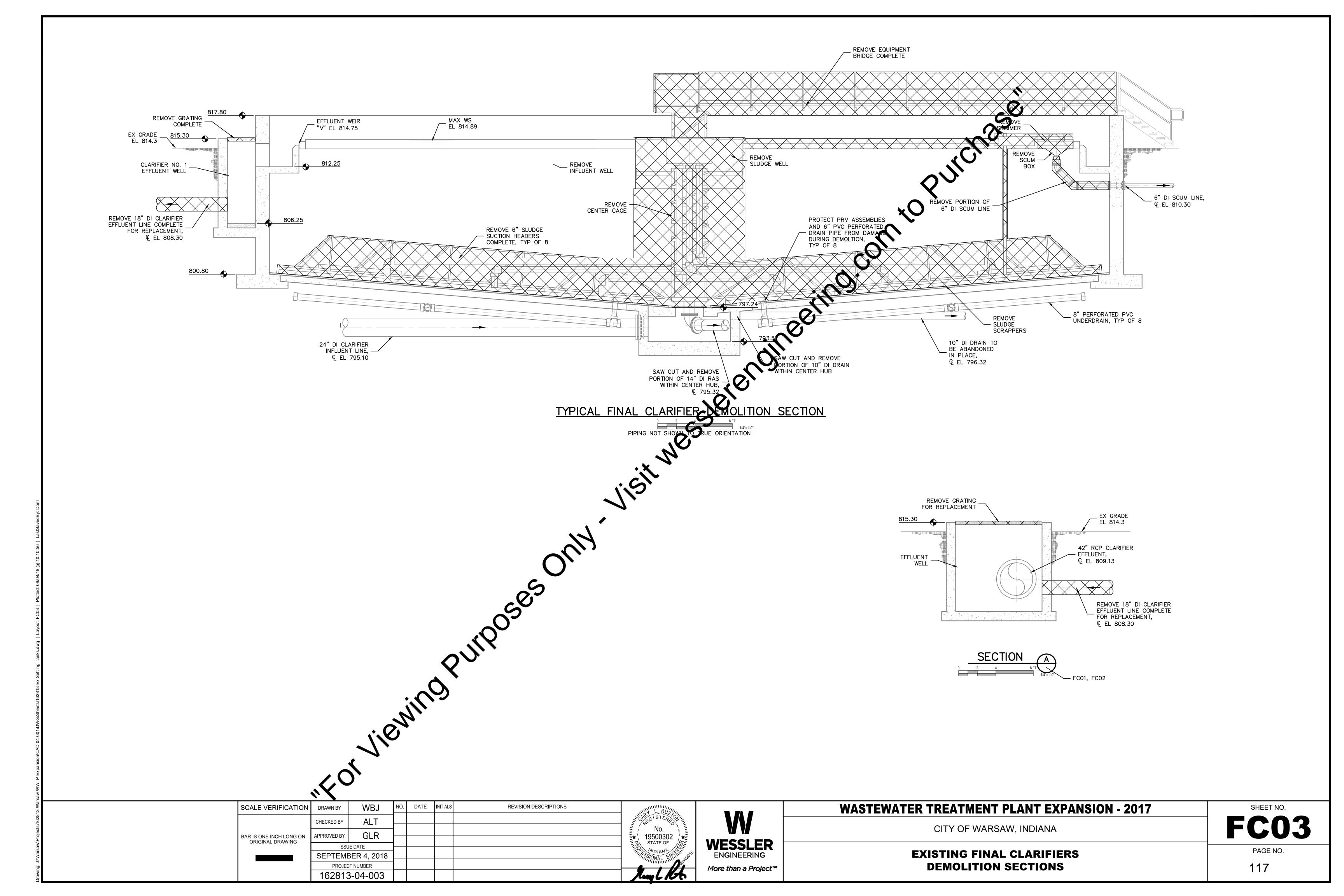
**NEW DRAIN PUMP STATION ELECTRICAL PLAN** 

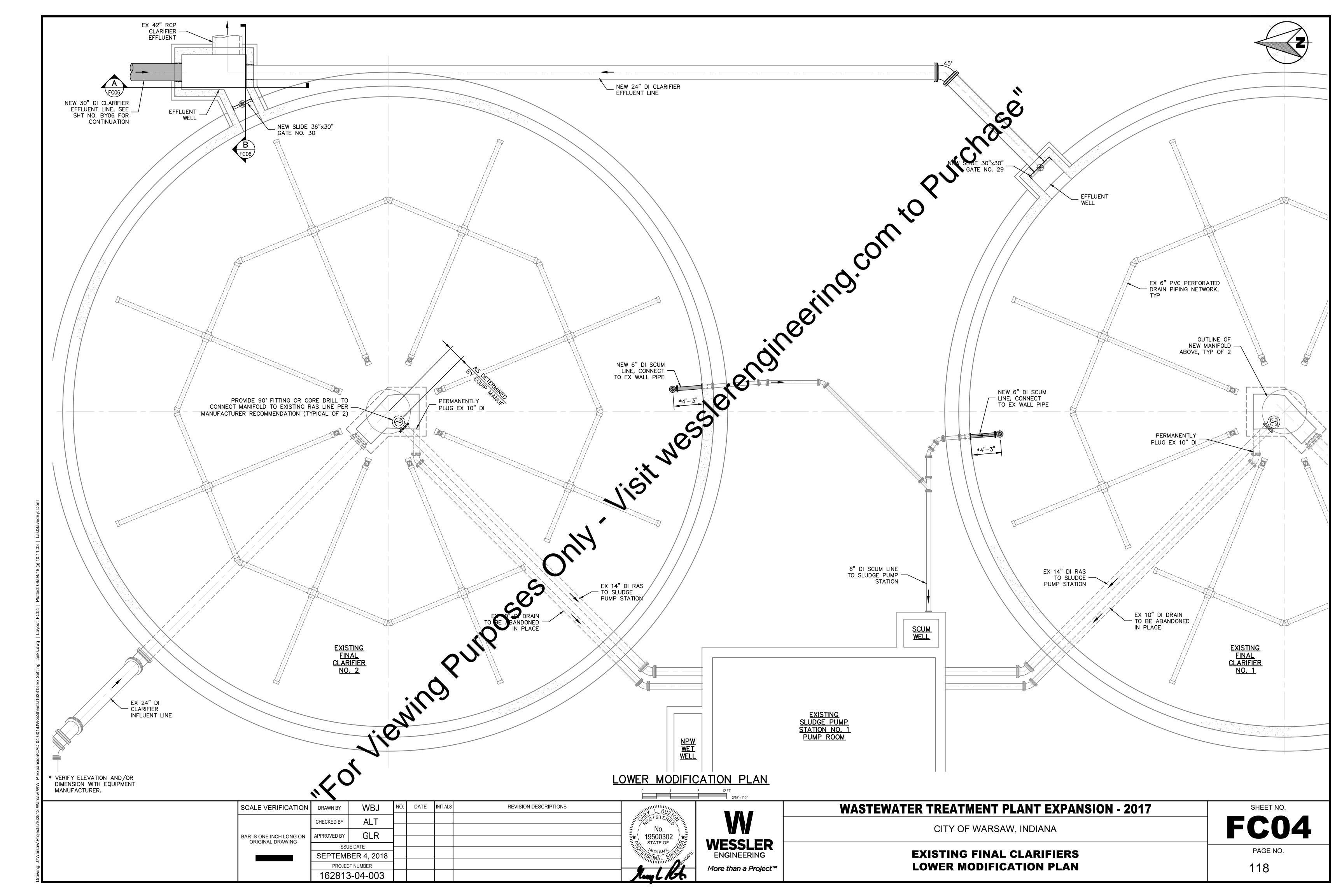
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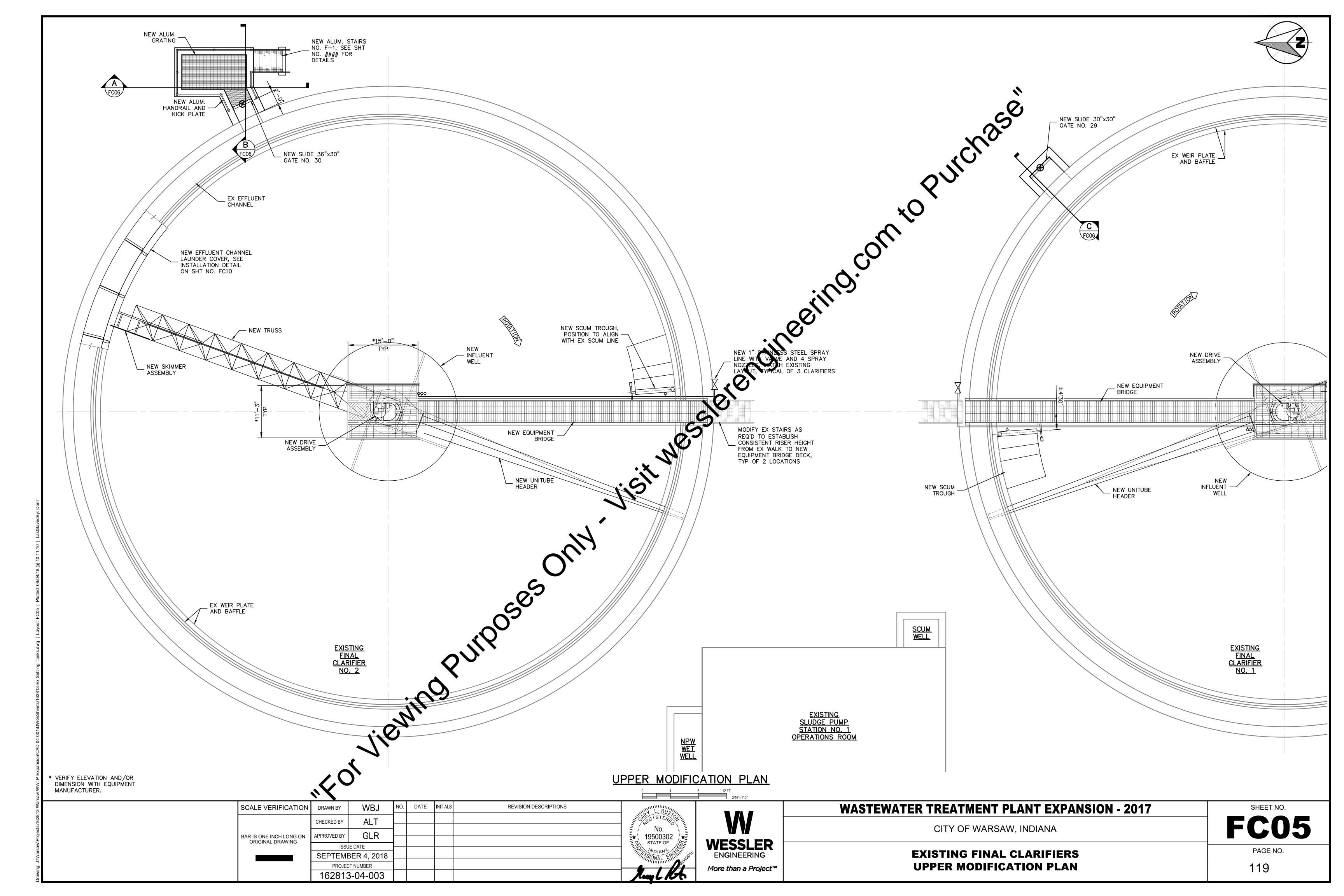
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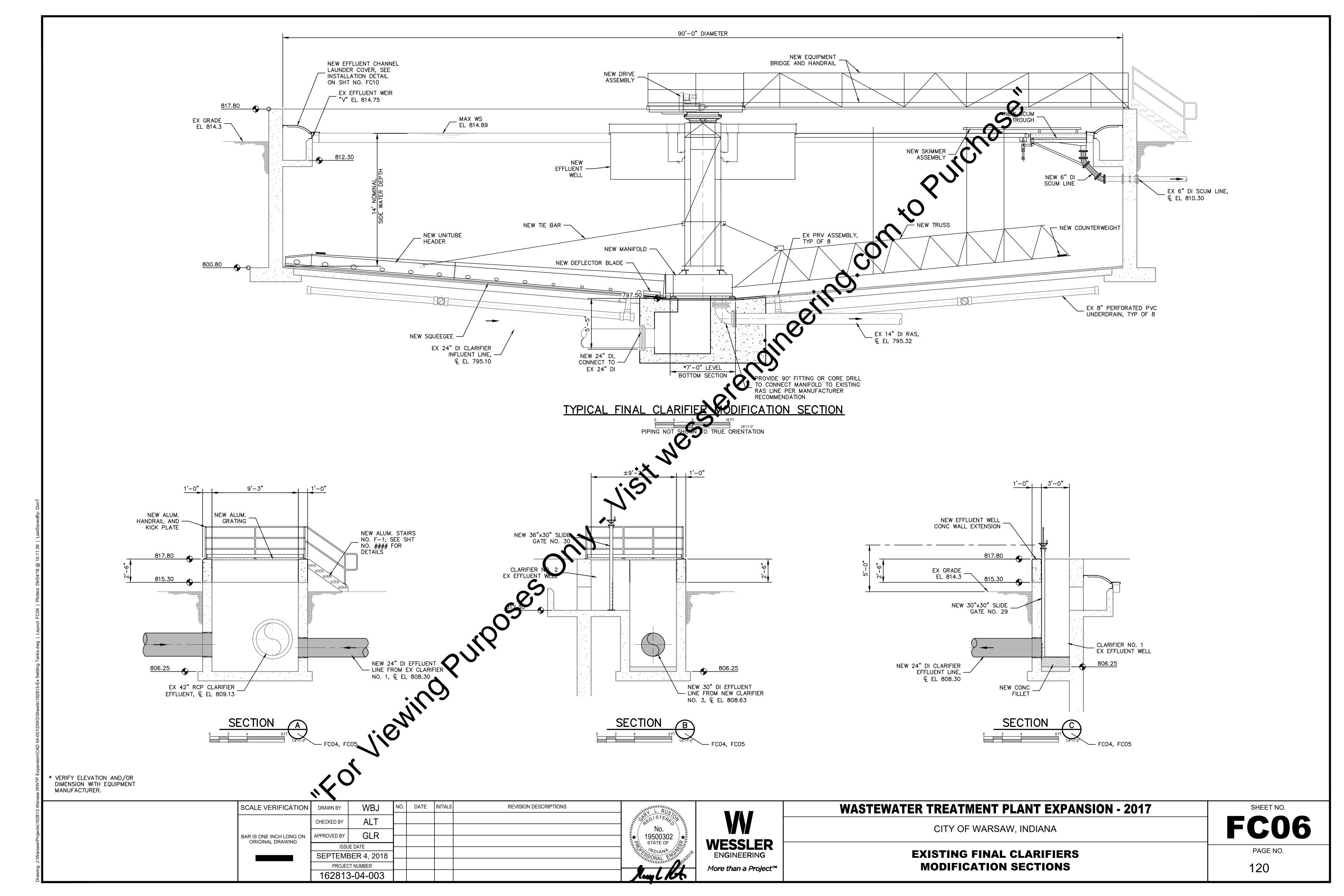


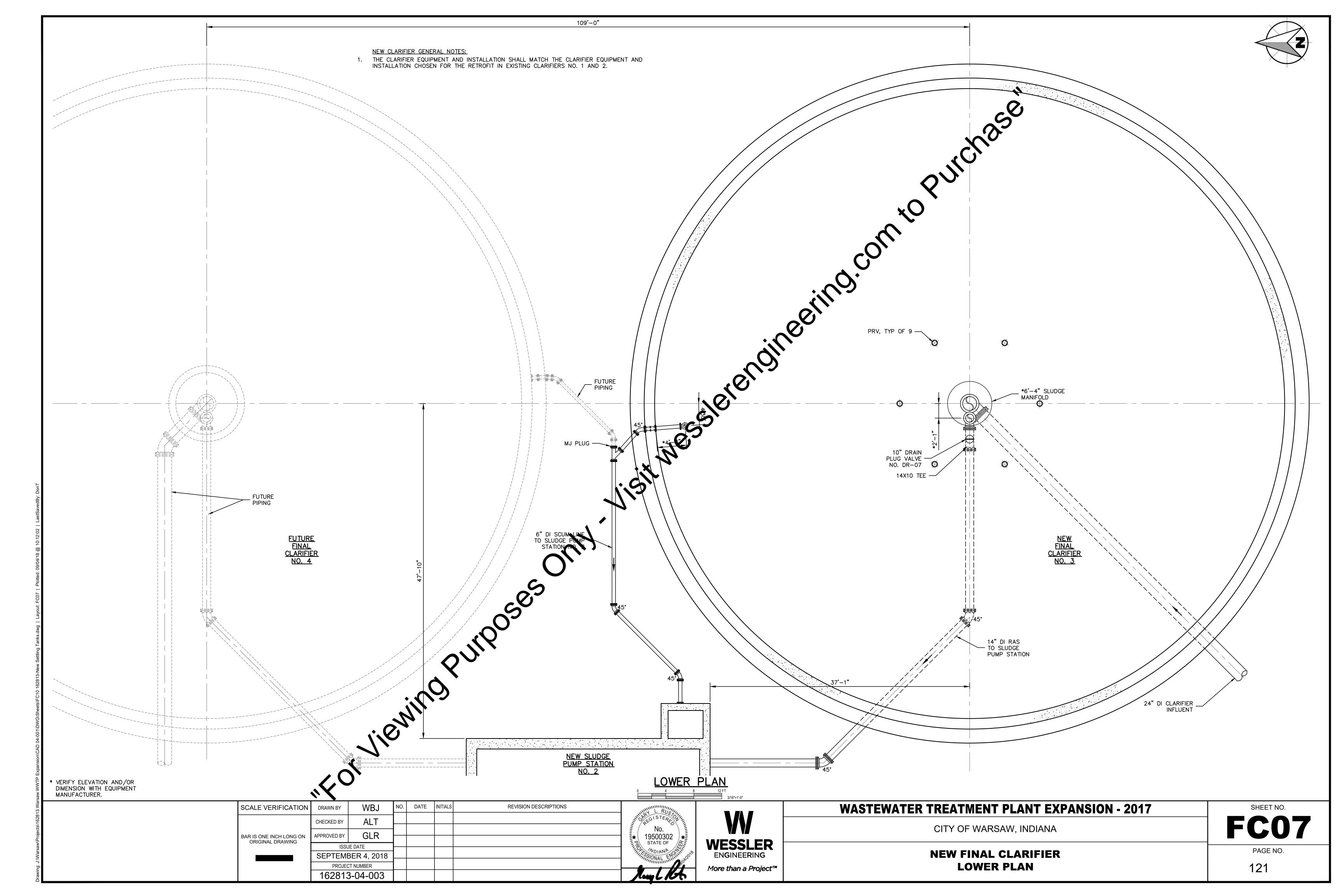


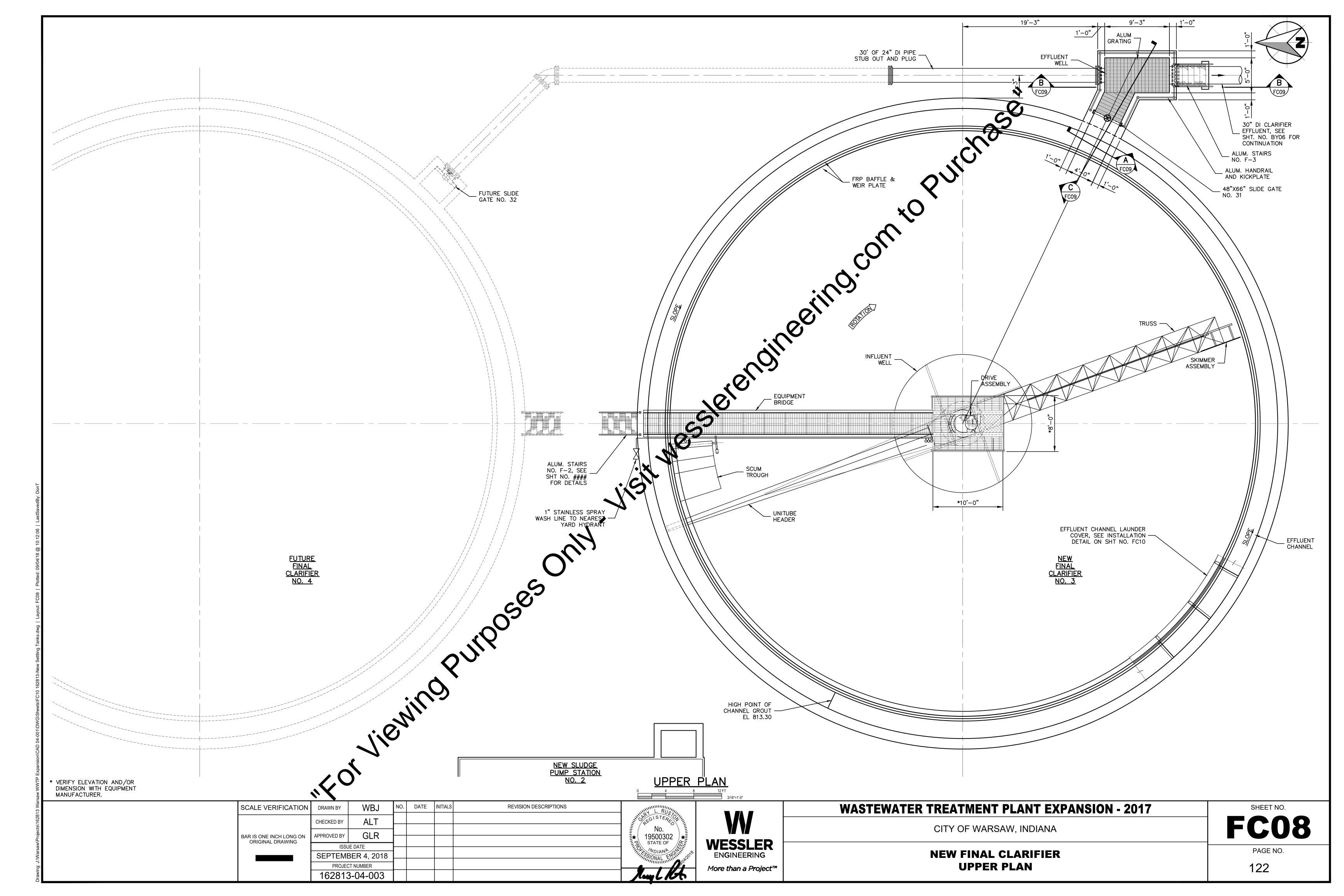


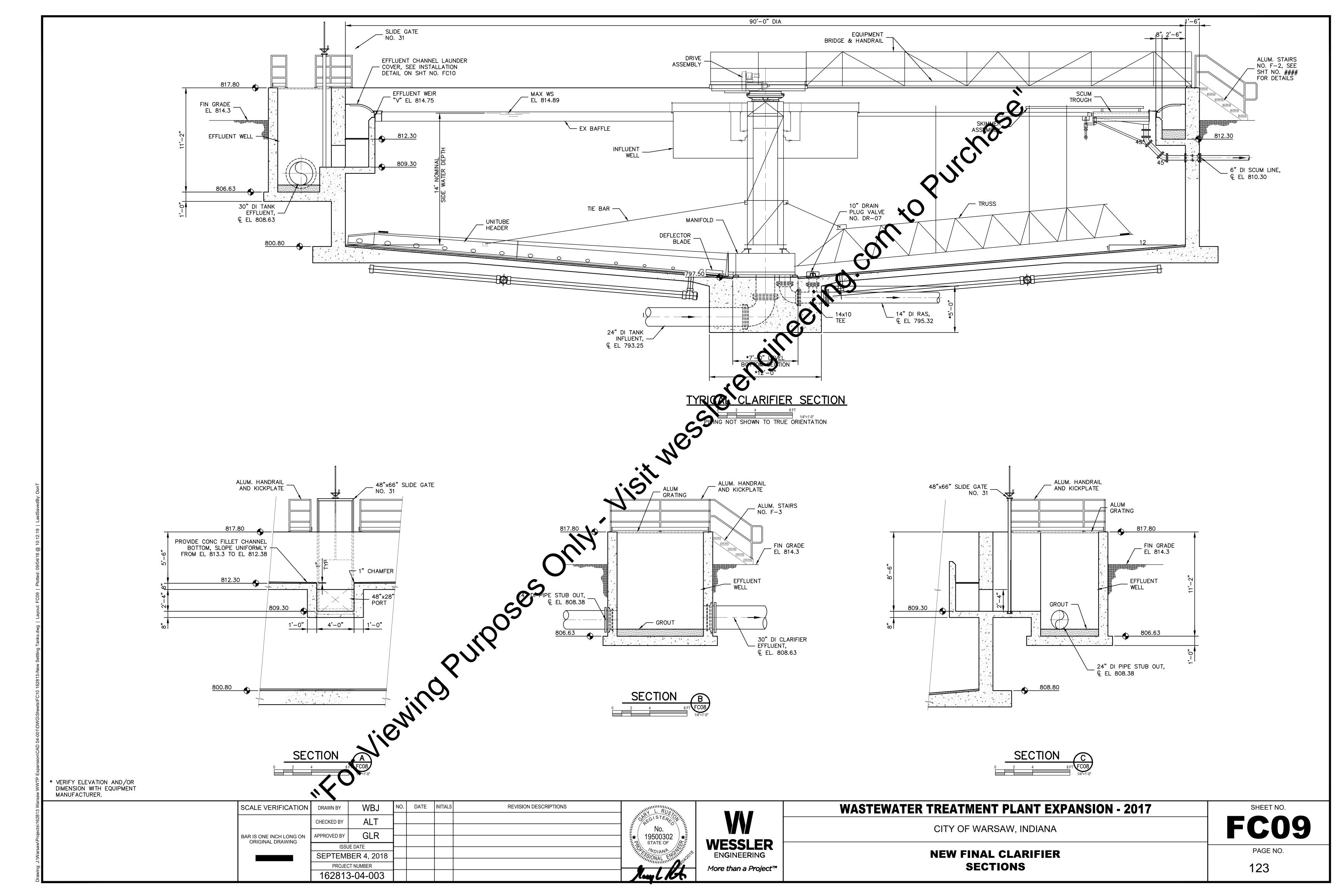


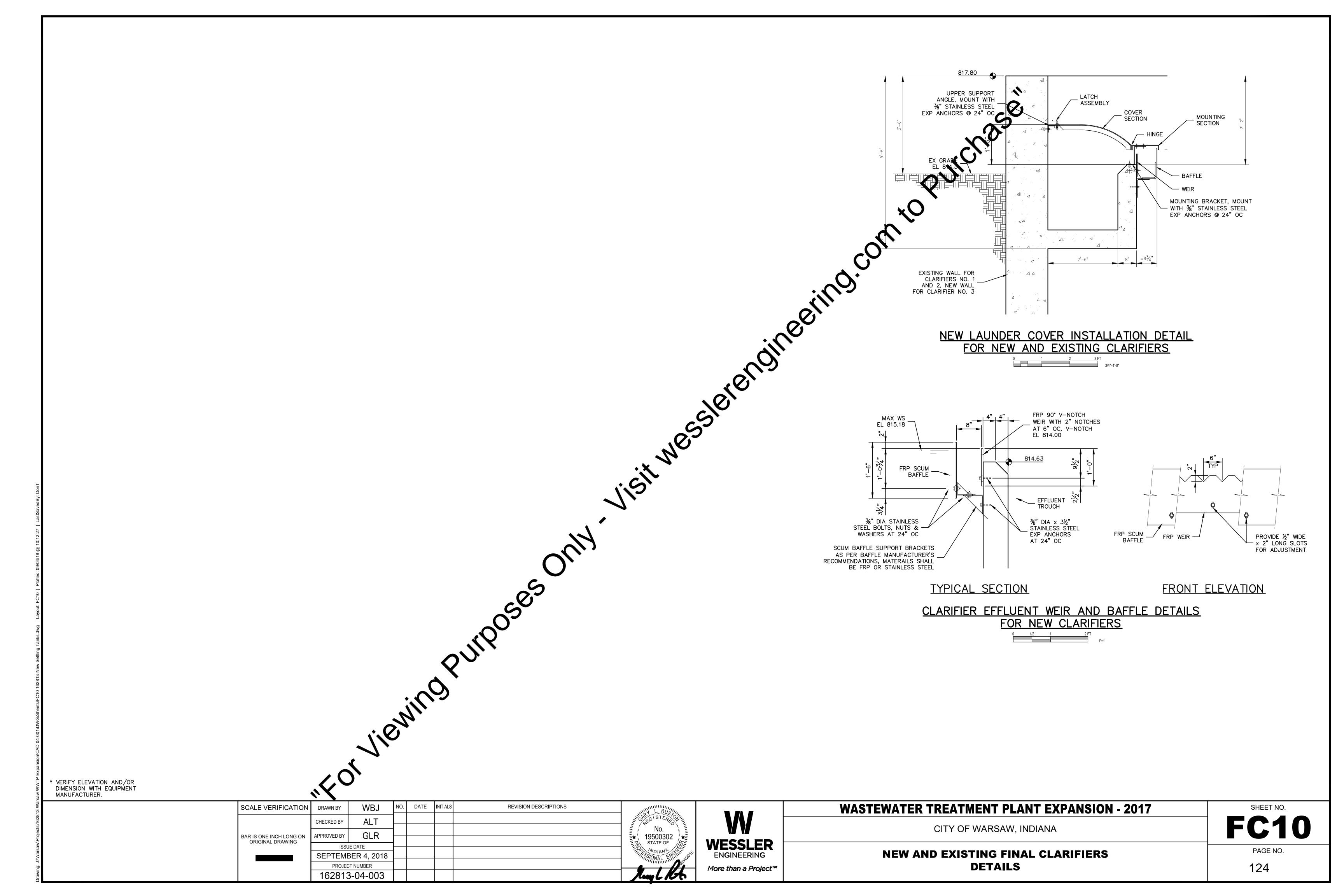








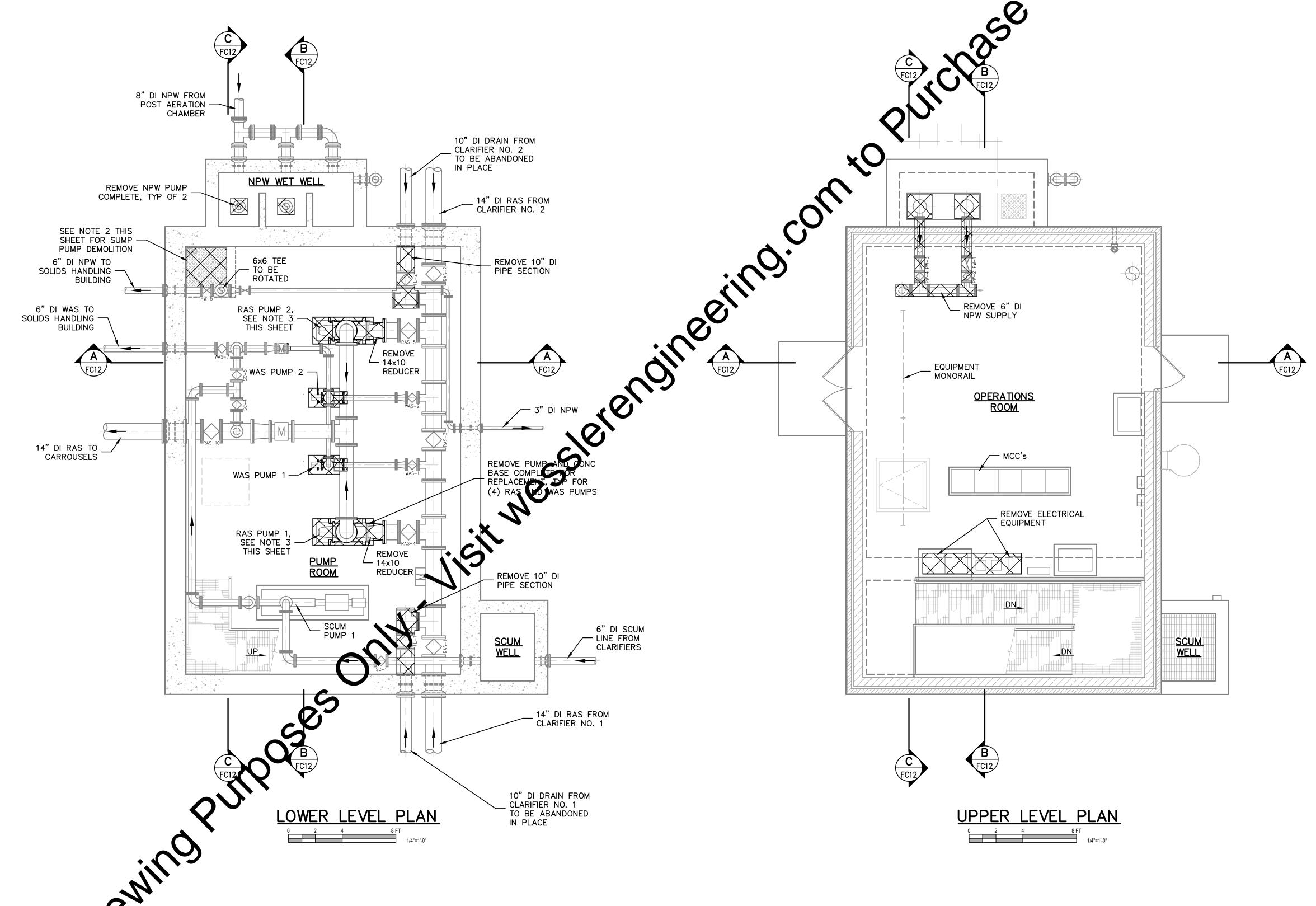




## SLUDGE PUMP STATION GENERAL DEMOLITION NOTES:

- THE FINISHED FLOOR ELEVATION FOR THE SLUDGE PUMP STATION HAS BEEN SHOWN AT 818.50 ON THE 2001 WASTEWATER TREATMENT PLANT PROJECT, BY JONES & HENRY ENGINEERS, LTD. THE WESSLER ENGINEERING SITE SURVEY FOR THIS PROJECT DETERMINED THE FINISHED FLOOR ELEVATION TO BE 815.80. ALL VERTICAL INFORMATION SHOWN FOR THIS STRUCTURE HAS BEEN REVISED TO AGREE WITH THE SITE SURVEY, WITH THE ASSUMPTION THAT VERTICAL DISTANCES REFERENCED ON THE JONES & HENRY DRAWINGS ARE ACCURATE.
- 2. REMOVE SUMP PUMP SYSTEM COMPLETE FOR REPLACEMENT. REMOVAL INCLUDES PUMPS, VALVES, DISCHARGE PIPING AS REQUIRED, ACCESS HATCH, ELECTRICAL CONNECTIONS, AND CONTROLS.
- 3. RAS PUMP NO. 1 AND RAS PUMP NO. 2 TO BE RELOCATED TO NEW SLUDGE PUMP STATION NO. 2.





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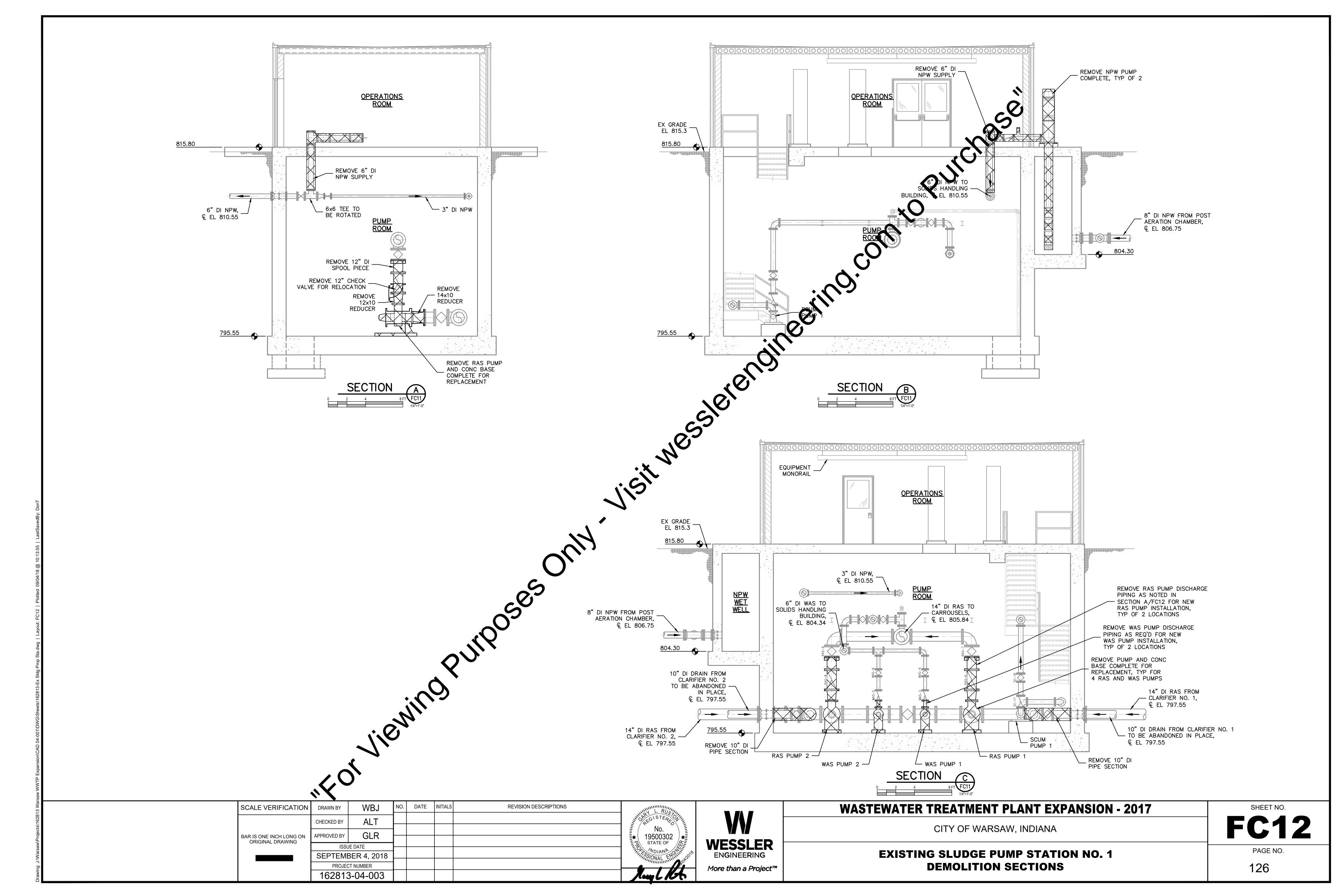


<b>WASTEWATER TREATMENT PLANT EXPANSION - 2017</b>	
CITY OF WARSAW, INDIANA	

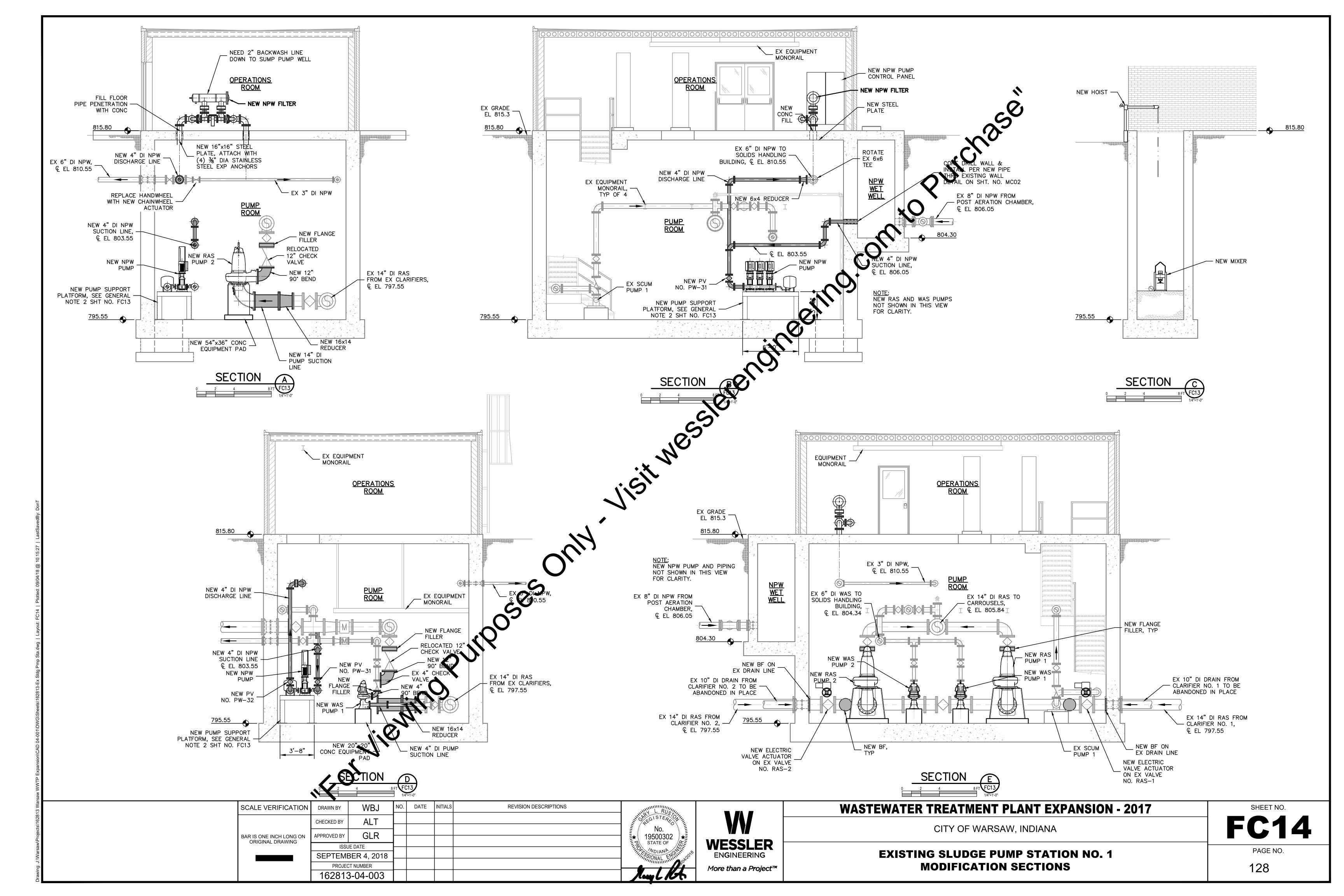
EXISTING SLUDGE PUMP STATION NO. 1
DEMOLITION PLANS

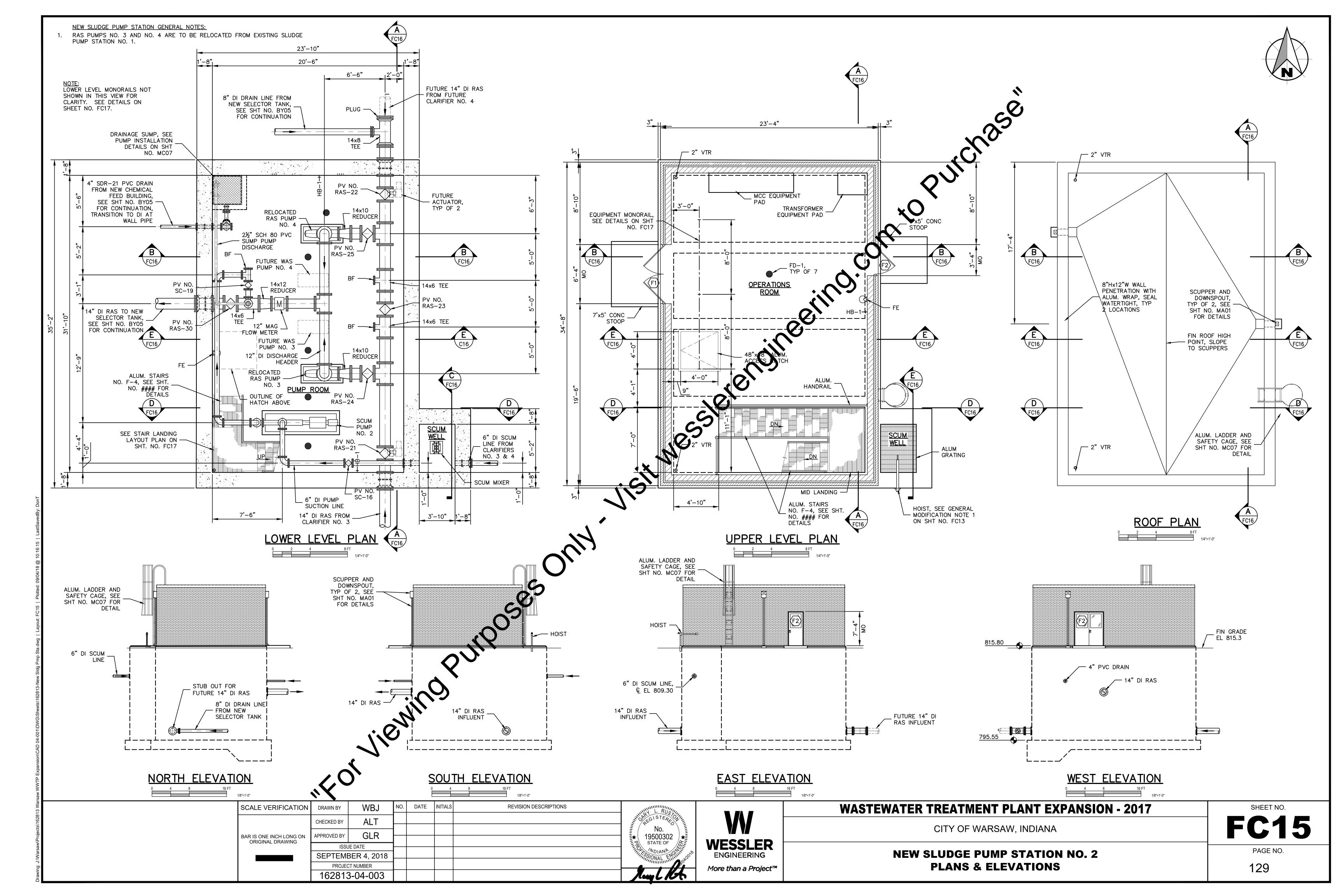
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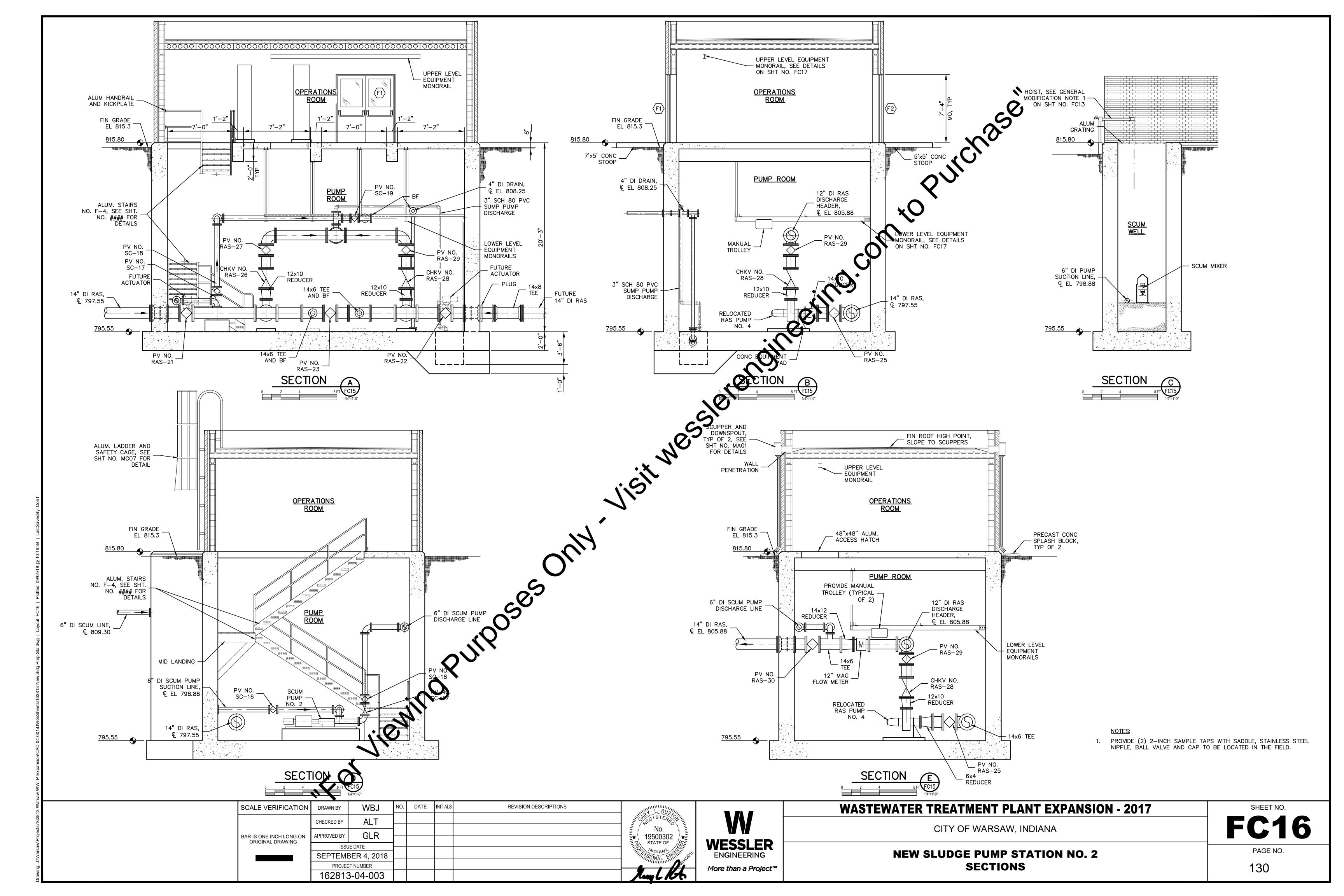
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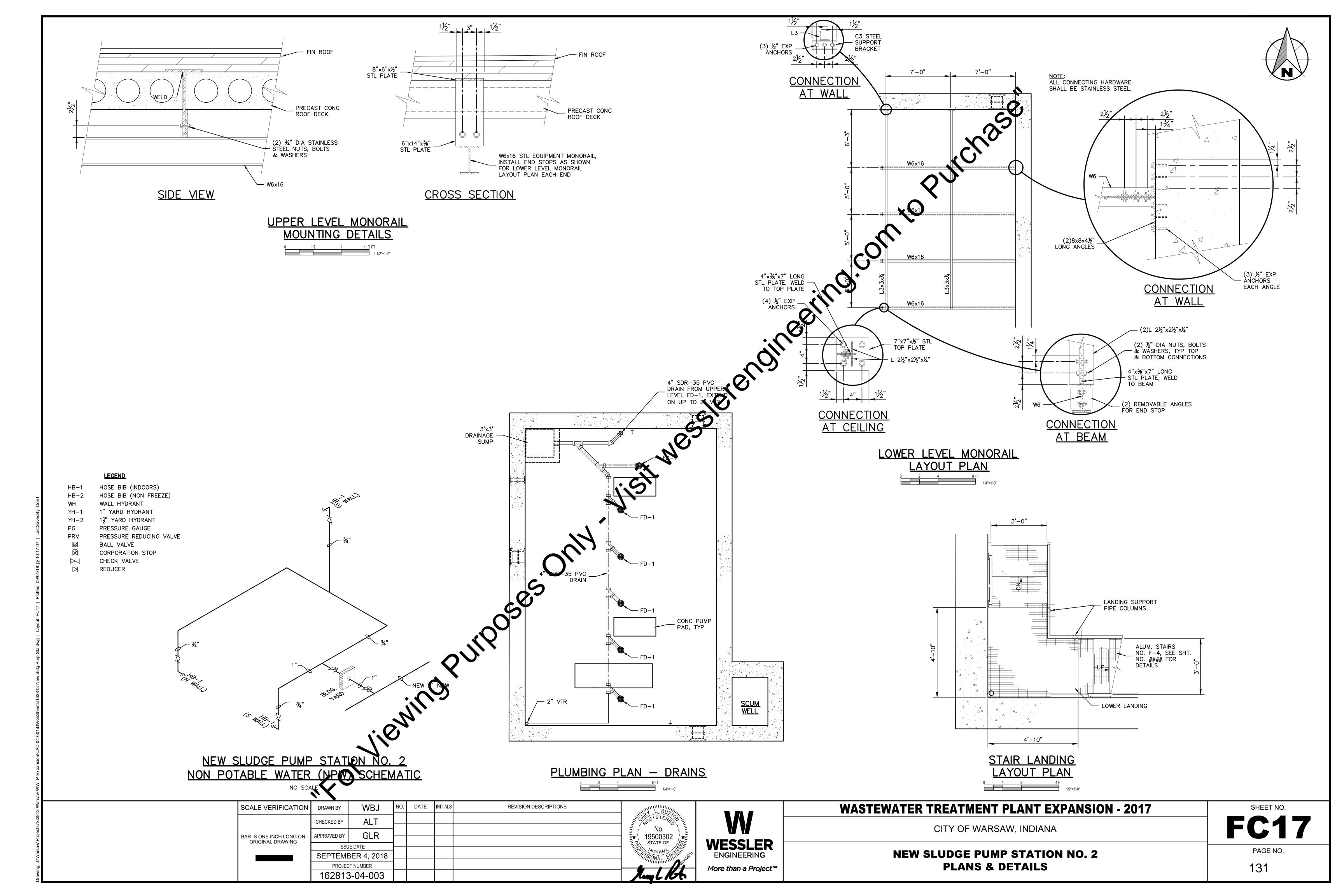


SLUDGE PUMP STATION GENERAL MODIFICATION NOTES: 1. FURNISH AND INSTALL (1) NEW 500 POUND CAPACITY HOIST FOR THE NEW SCUM MIXER REMOVAL, WITH A MINIMUM 42" REACH BOOM WITH A MINIMUM 34° UPWARD SWIVEL. FURNISH WITH A WALL MOUNTED BASE WITH A 360° HORIZONTAL SWIVEL, A MANUAL WINCH, AND A MINIMUM 30' OF 304 STAINLESS STEEL CABLE. THE HOIST SHALL BE A THERN FIRST MATE MODEL 5122, OR APPROVED EQUAL. MOUNT TO THE SCUM WALL WITH STAINLESS STEEL EXPANSION ANCHORS, SIZED AS RECOMMENDED BY THE HOIST MANUFACTURER. FURNISH AND INSTALL (1) STEEL PLATFORM, COATED, SUITABLE TO SUPPORT THE TOTAL WEIGHT OF THE NEW NPW PUMPING EQUIPMENT, BLADDER TANK, SUCTION AND DISCHARGE PIPING, AND ALL WATER CONTAINED IN THE PUMP AND PIPING. ATTACH STEEL BASE PLATES, COATED, TO BOTTOM OF THE (4) PLATFORM LEGS AND ATTACH TO CONCRETE FLOOR WITH MINIMUM (2) 3/8" STAINLESS STEEL EXPANSION ANCHORS PER BASE PLATE. INSTALL NEW SUMP PUMPS, PIPING, VALVES, AND ACCESS COVER IN EXISTING 3'x3'x3' SUMP AS SHOWN IN THE BUILDING SUMP PUMP INSTALLATION DETAILS ON SHEET NO. MCO7. RECONNECT TO EXISTING DISCHARGE PIPING AS REQUIRED. NEW ELECTRICAL AND CONTROLS AS SHOWN IN ASSOCIATED DRAWINGS. 4. PROVIDE 1" STAINLESS STEEL NIPPLES AND VALVES ON RAS AND WAS PUMP VOLUTES FOR 5. PROVIDE (2) 2" SAMPLE TAPS ON SADDLE, STAINLESS STEEL NIPPLE, BALL VALVE, AND CAP TO BE LOCATED IN THE FIELD. EX 8" DI NPW FROM POST AERATION CHAMBER INSTALL NEW ALUM. 22"x22"x¼"
PLATE COVERS OVER EX PUMP
PENETRATIONS, ATTACH TO EX
CONC SLAB WITH EX EXP AND LOSS EX 10" DI DRAIN FROM CLARIFIER NO. 2 TO BE ABANDONED CORE DRILL WALL & IN PLACE INSTALL PER NEW PIPE NPW WET WELL THRU EXISTING WALL DETAIL ON SHT. NO. MCO2 EX 14" DI RAS FROM CLARIFIER NO. 2 SEE NOTE 3 THIS SHEET FOR SUMP -PUMP MODIFICATIONS NEW ELECTRIC FILL FLOOR ROTATE EX VALVE ACTUATOR EX 6" DI NPW TO ON EX VALVE — PIPE PENETRATION SOLIDS HANDLING — 6x6 TEE NO. RAS-2 WITH CONC BUILDING PUMP 2 EX 6" DI WAS TO NEW 6x4 SOLIDS HANDLING -REDUCER BUILDING NEW 16x14 REDUCER EX EQUIPMENT MONORAIL **NEW PUMP SUPPORT** PLATFORM, SEE GENERAL **OPERATIONS** NOTE 2 THIS SHEET <u>ROOM</u> NEW WAS PUMP 2 EX 14" DI RAS TO CARROUSELS NEW 4" DI NPW DISCHARGE LINE NEW WAS PUMP 1 ─ EX MCC's FC14 NEW 4" DI NPW SUCTION + EX 42"x42" HATCH \_\_\_\_\_ OPENING ABOVE **NEW RAS** SCUM WELL SCUM WELL EX 6" DI SCUM — LINE FROM CLARIFIERS NEW SCUM MIXER NEW HOIST, SEE GENERAL MODIFICATION NOTE 1 THIS SHEET EX 14" DI RAS FROM CLARIFIER NO. 1 EX 10" DI DRAIN FROM CLARIFIER - NO. 1 TO BE ABANDONED IN PLACE LOWER LEVEL PLAN UPPER LEVEL PLAN **WASTEWATER TREATMENT PLANT EXPANSION - 2017** SCALE VERIFICATION WBJ DATE INITIALS REVISION DESCRIPTIONS **FC13** ALT CHECKED BY No. 19500302 STATE OF CITY OF WARSAW, INDIANA GLR APPROVED BY BAR IS ONE INCH LONG ON **WESSLER** ORIGINAL DRAWING ISSUE DATE PAGE NO. **EXISTING SLUDGE PUMP STATION NO. 1** ENGINEERING SEPTEMBER 4, 2018 **MODIFICATION PLANS** 127 PROJECT NUMBER More than a Project™ 162813-04-003









				MA	KE-U	P AIR	SCH	EDU	LE									
TAG	MANUFACTURER AND MODEL NUMBER	LOCATION	SUPPLY AIR	<b>.</b>		HEATING			FAN DATA		POWER SUPPLY			GAS	UNIT WEIGHT	OPTIONS	NOTE(S)	INTERLOCK WITH
IAG		LOCATION	CFM	E.S.P.	MBH INPUT	MBH OUTPUT	TEMP RISE	HP	RPM	F.L.A.	M.C.A.	VOLTAGE	PHASE	INLET SIZE	LBS	OPTIONS	NOTE(3)	INTERLOCK WITH
MUA-1	REZNOR RDH-75	ROOF	563	0.25	75	60.75	99.0	1/4	554	12	15	460	3	1.5	120	1,3,4	Α	THERMOSTAT
OPTIONS:																		

1. GFI CONVENIENCE OUTLET, 2. DDC CONTROLS, 3. INTEGRAL DISCONNECT, 4. ROOF CURB, 5. EXTRA FILTERS, 6. THRU-WALL INSTALLATION PACKAGE, 7. 3-WAY DISCHARGE DIFFUSER, 8. 4-WAY DISCHARGE DIFFUSER

\_12X10 DOWN

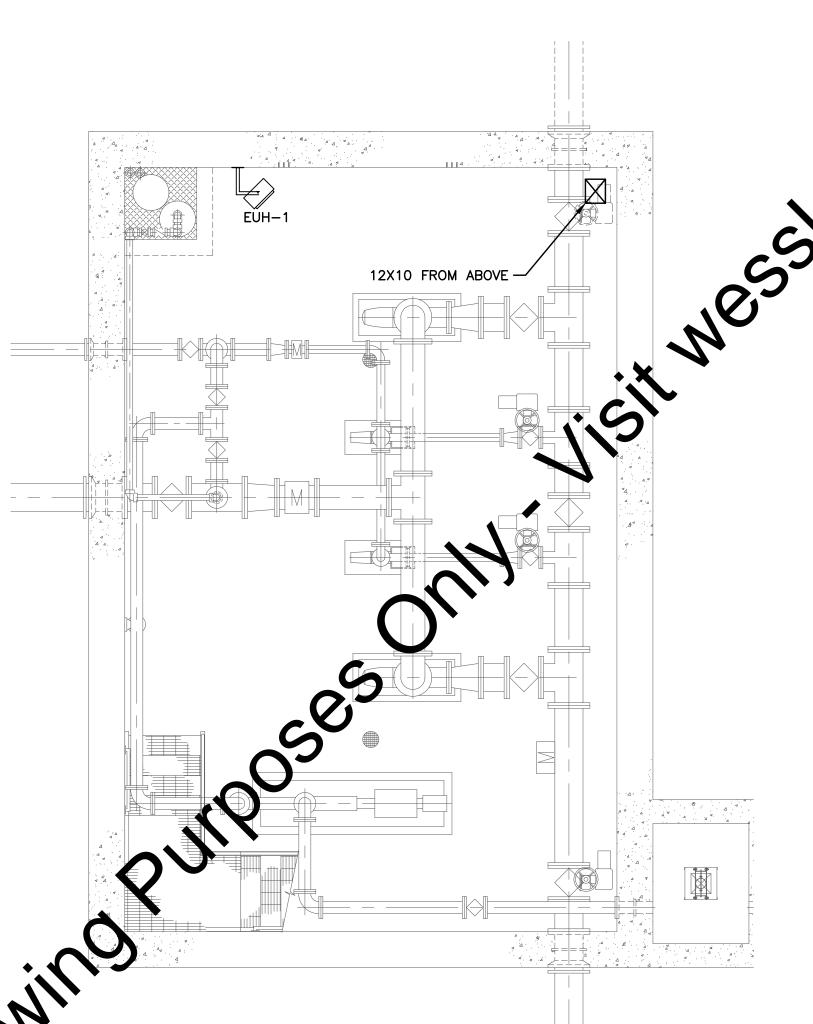
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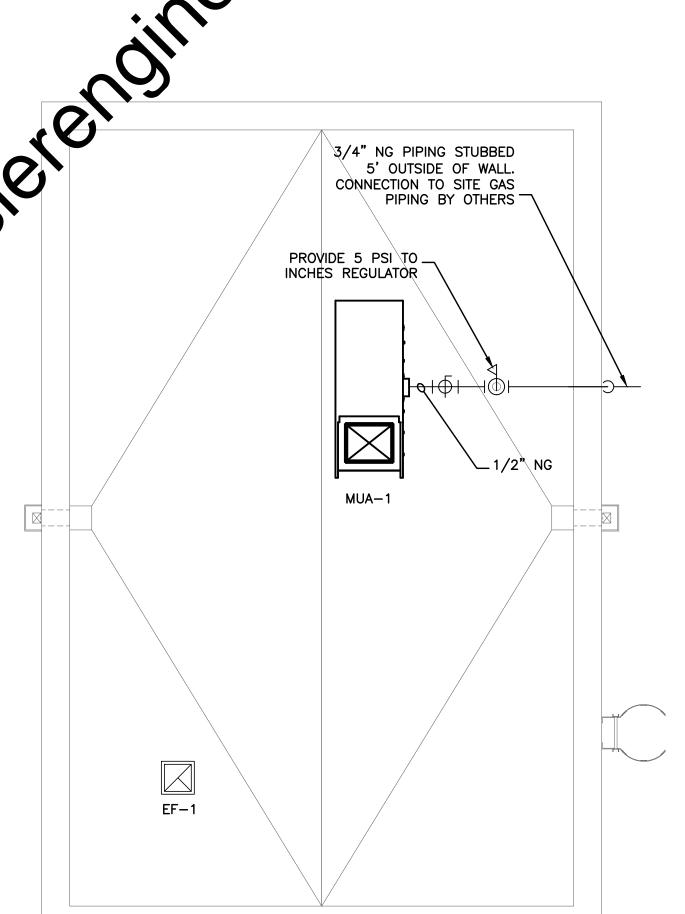
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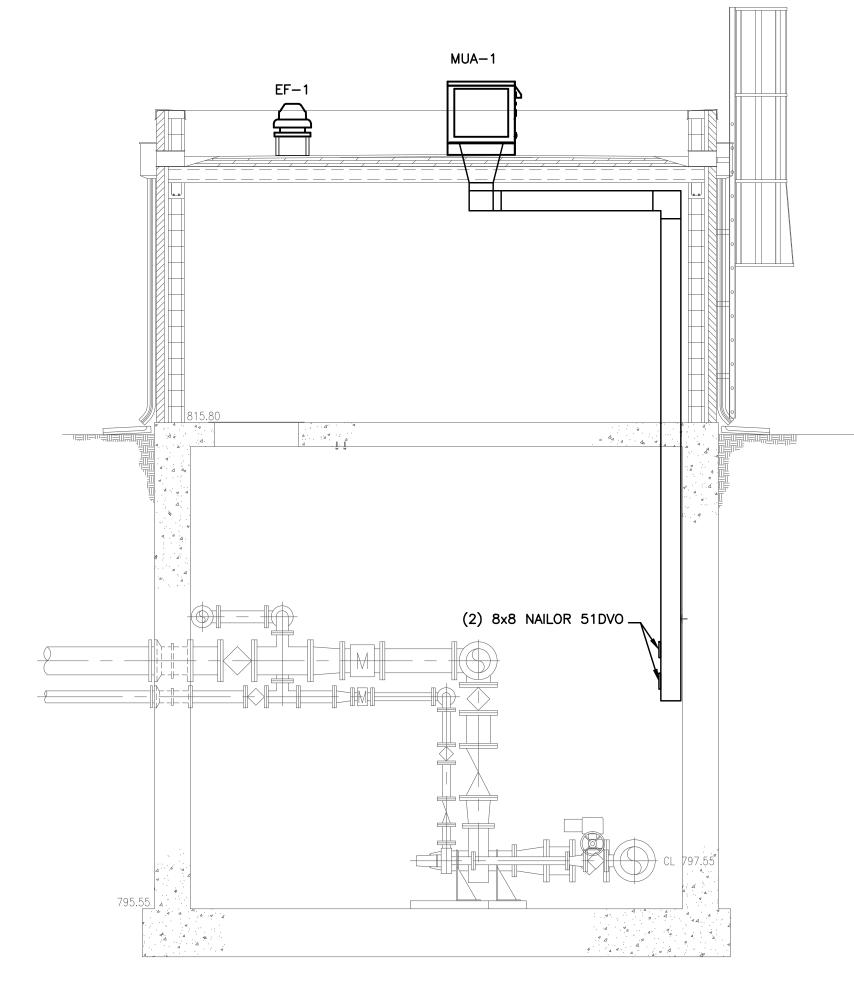
A. INCLUDES 2" THICK METAL MESH FILTERS.

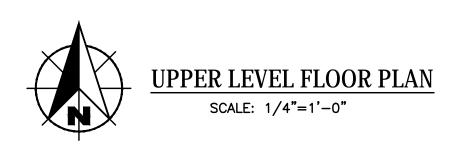
	EXHAUST FAN SCHEDULE														
TAG	LOCATION	TYPE	CFM	S.P.	FRPM	BHP (WATTS)	SONES	HP	MO <sup>-</sup> VOLT	TOR RPM	PH	UNIT WEIGHT LBS	ROOF OPENING	MANUFACTURER AND MODEL NUMBER	INTERLOCK WITH
EF-1	ROOF	ROOF	500	0.375	1550	(107)	8.4	1/8	115	1725	1	35	13.5" SQ	LOREN COOK 90C15DH	MUA-1
ACCESSO	CCESSORIES: BACKDRAFT DAMPER, DISCONNECT														

	ELECTRIC UNIT HEATER SCHEDULE																			
					FAN DA	ATA				ATIN 5	DATA		ELECT	RICAL D	ATA		ACCESSORIES		MANUFACTURED WITH MORE	
MARK	LOCATION	CONFIGURATION	AIRFLOW (CFM)	ESP	DRIVE	DESIGN SPEED(S)	HP		义	МВН	EAT	LAT	AMPS	VOLTS	PH	DISCONNECT SWITCH	INTEGRAL THERMOSTAT	WALL BRACKET	MANUFACTURER WITH MODEL NUMBER	NOTES
EUH-1	LOWER LEVEL	UNIT HEATER	700	1	AXIAL	1	1/00	5		17.1	-	_	12.1	480	3	Y	Y	Y	MARKEL 5500/QMARK QWD	1
EUH-2	UPPER LEVEL	UNIT HEATER	700	1	AXIAL	1	100	5	•	17.1	_	_	12.1	480	3	Y	Y	Y	MARKEL 5500/QMARK QWD	1
	NOTES: 1. WASHDOWN/CORF	ROSION RESISTANT	CONSTRU	JCTION	•		•	-			•	-	•			•	-		•	











LOWER LEVEL FLOOR PLAN

SCALE: 1/4"=1'-0"



SECTION
SCALE: 1/4"=1'-0"

132

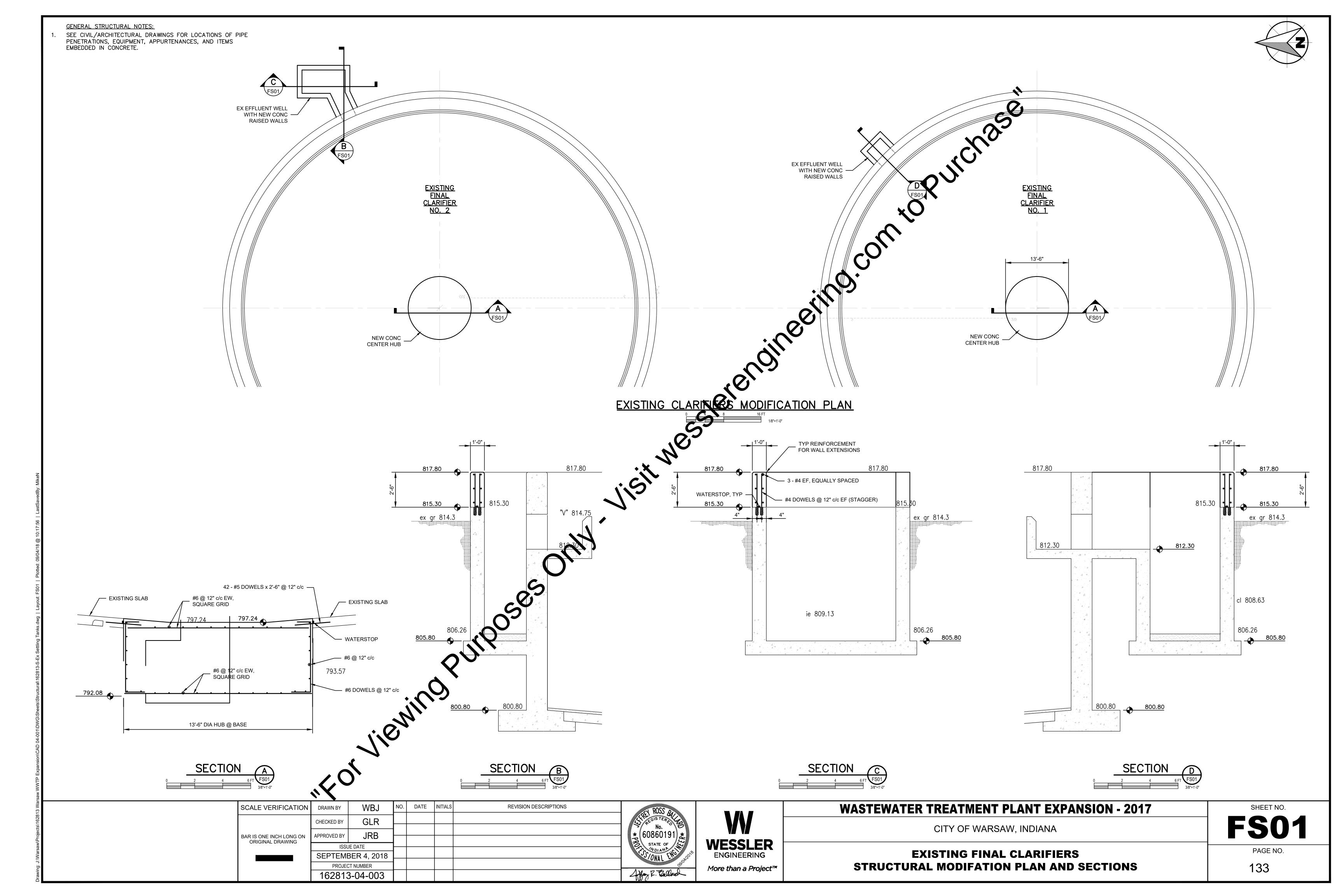
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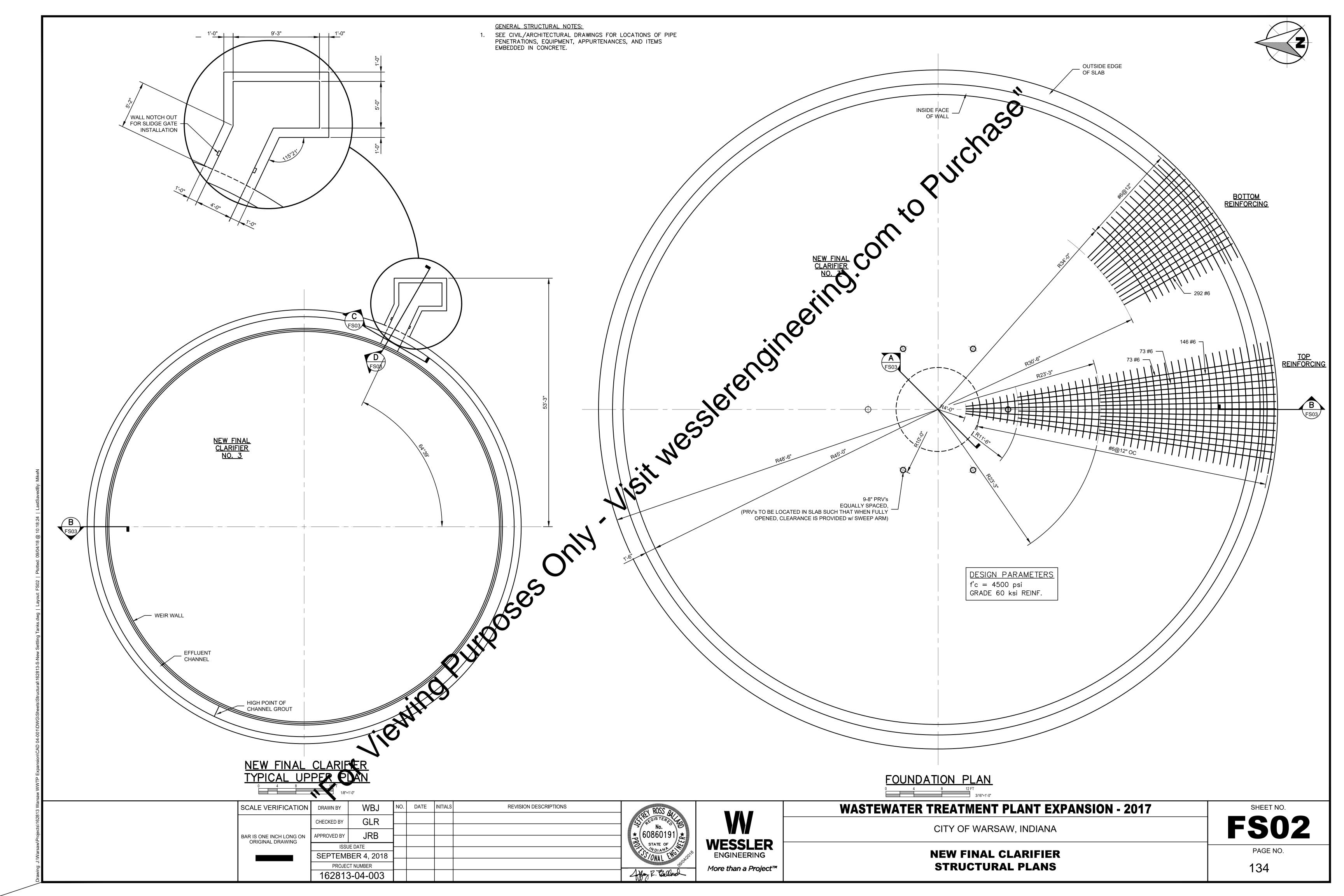
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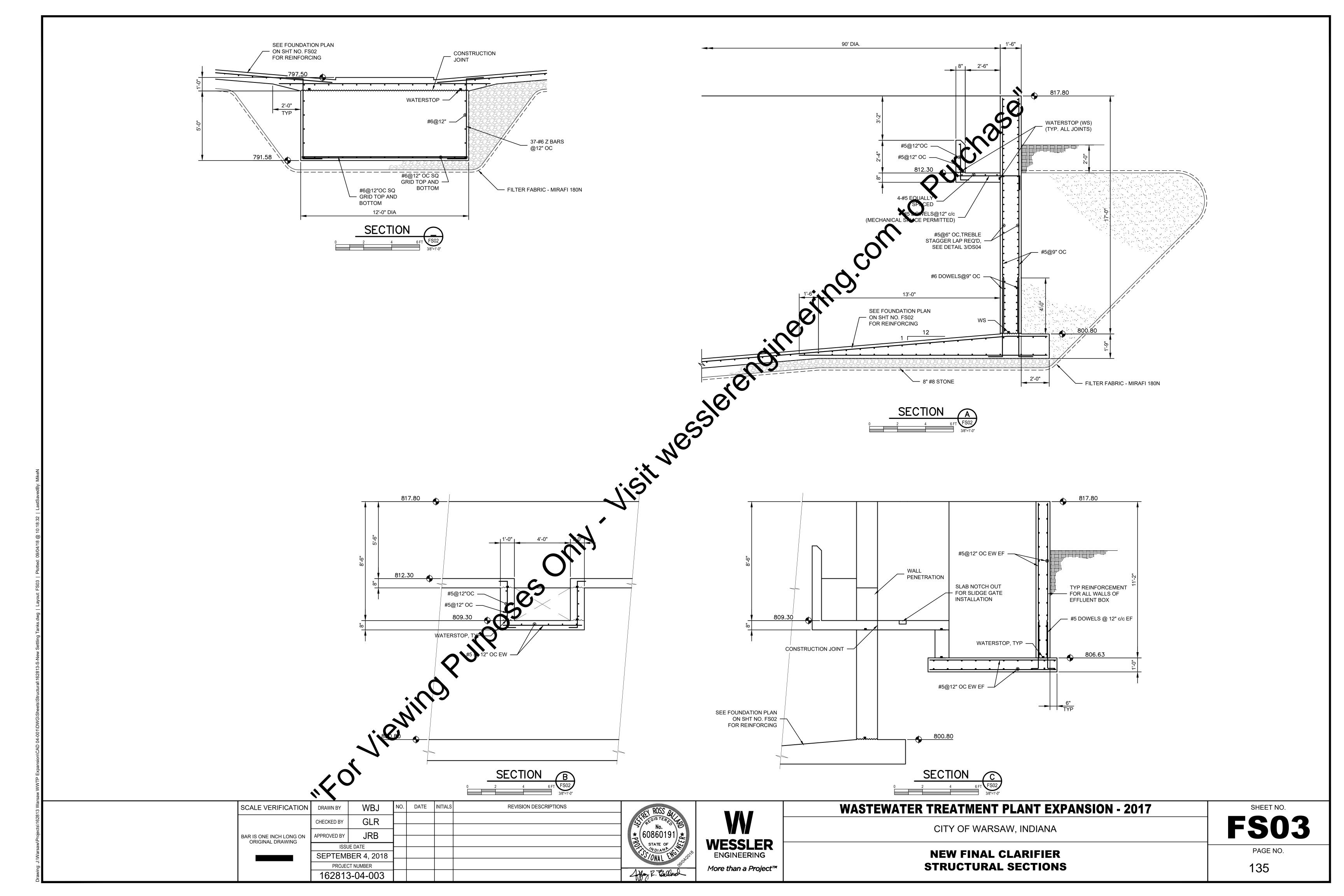


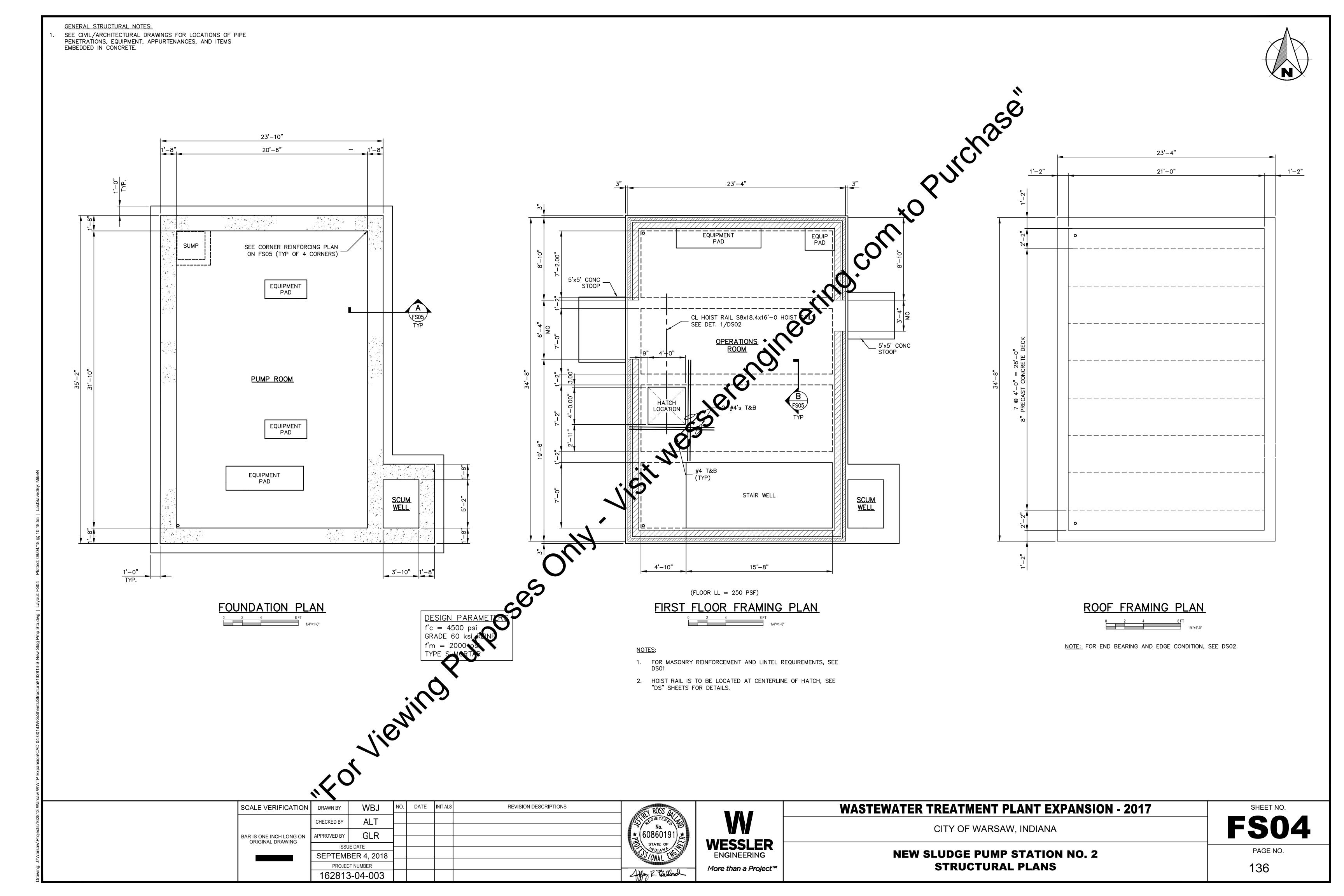
WASTEWATER TREATMENT PLANT EXPANSION - 2017	SHEET NO.
CITY OF WARSAW, INDIANA	FMO
NEW SLUDGE PUMP STATION NO. 2	PAGE NO.

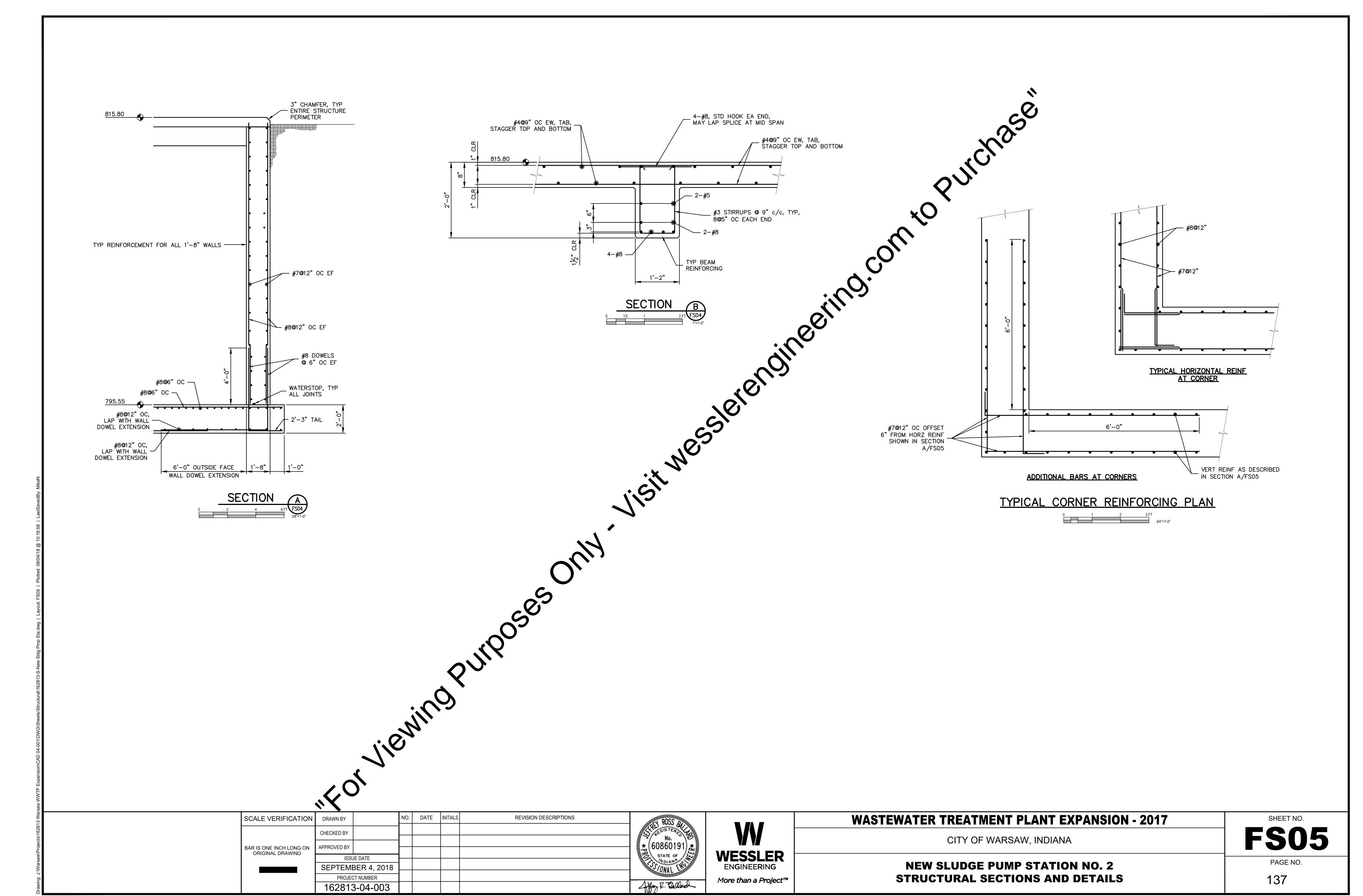
MECHANICAL PLAN

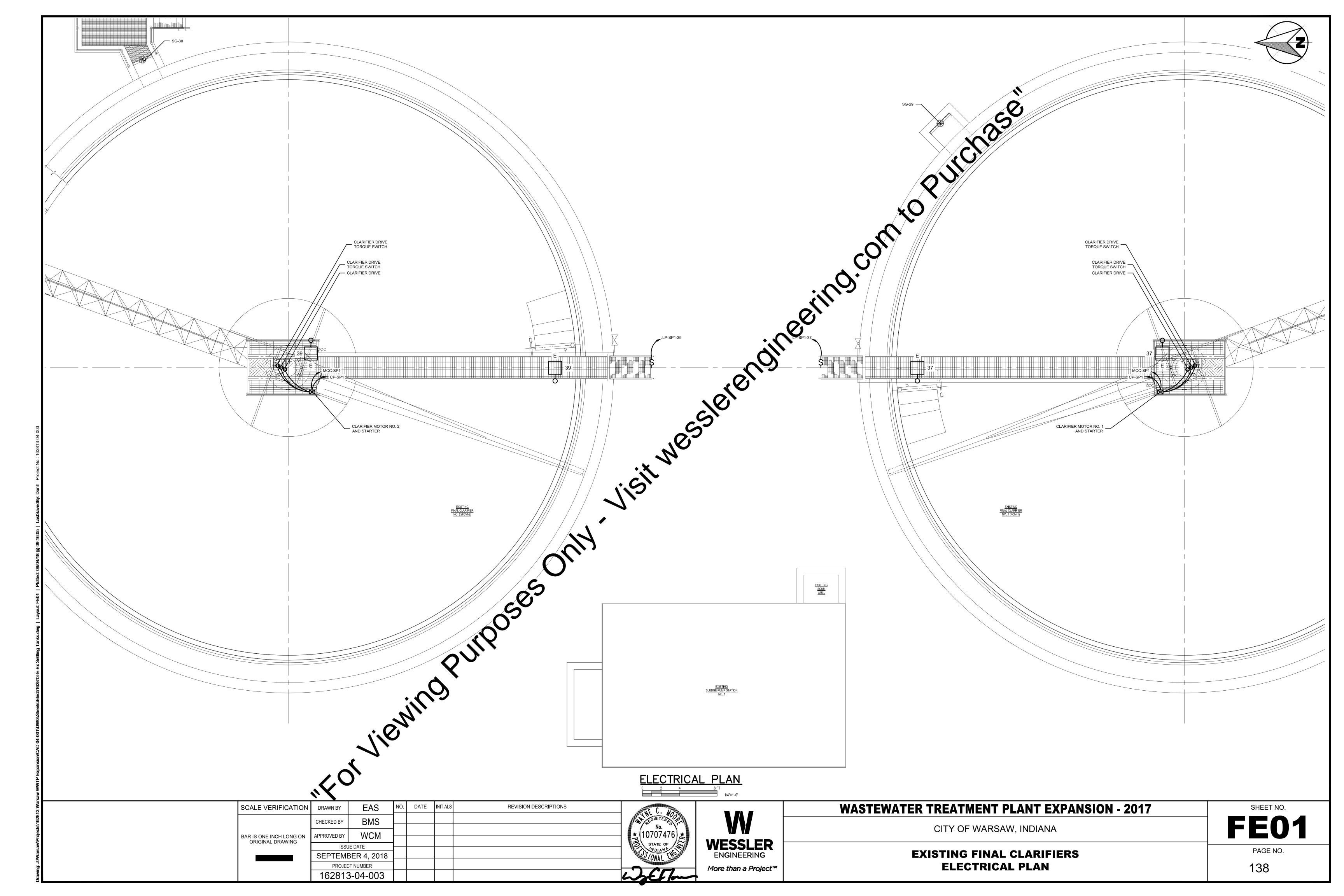


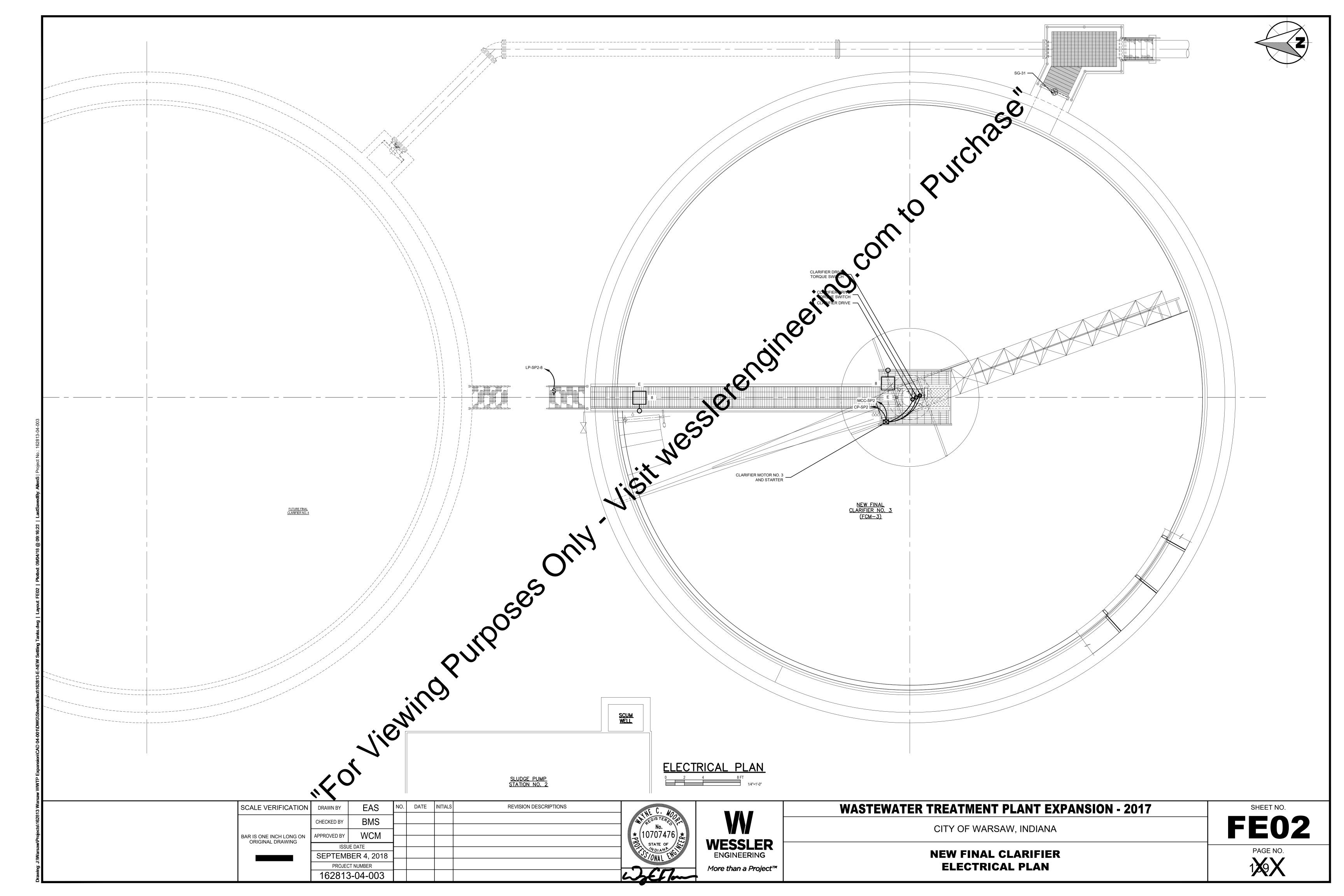


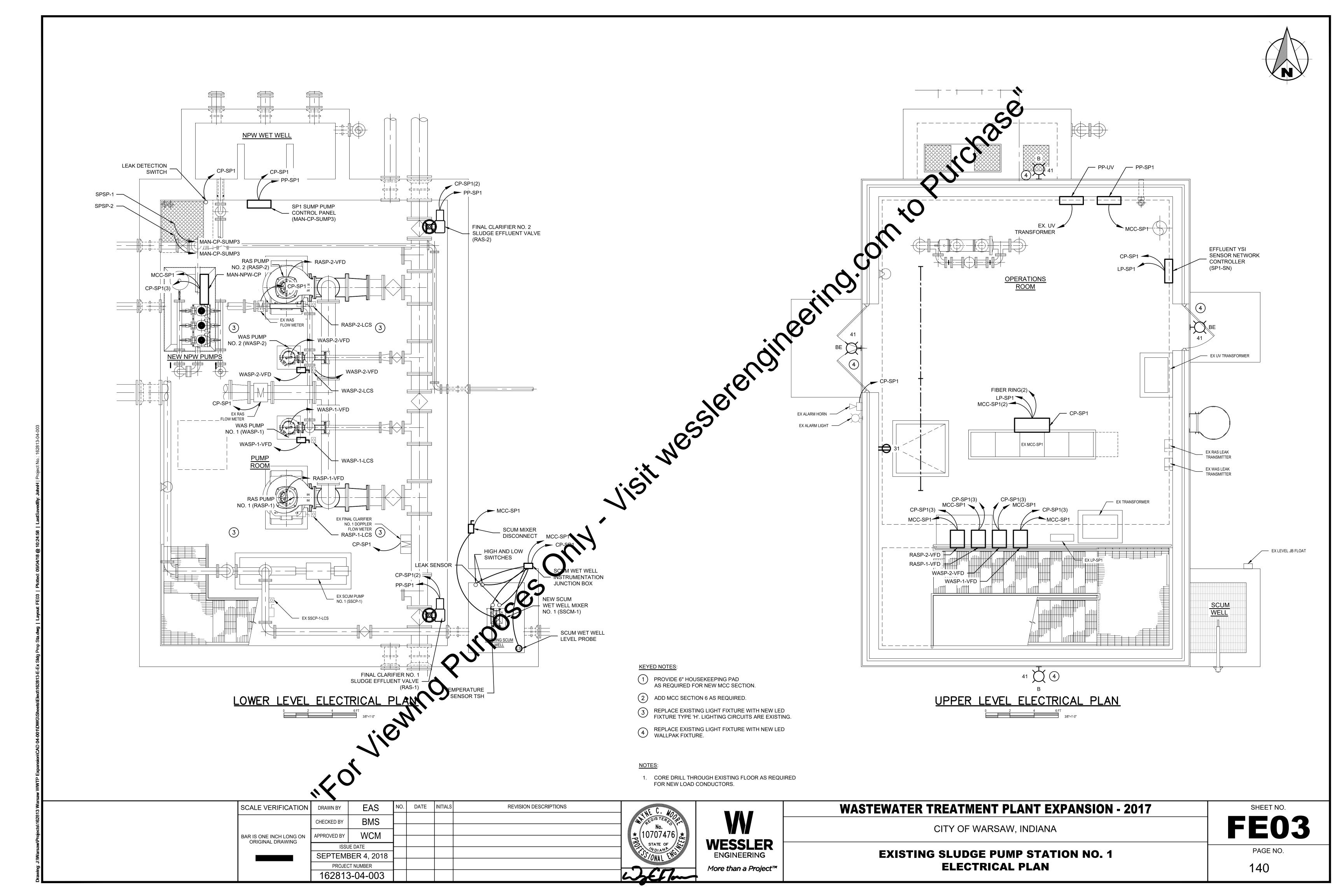




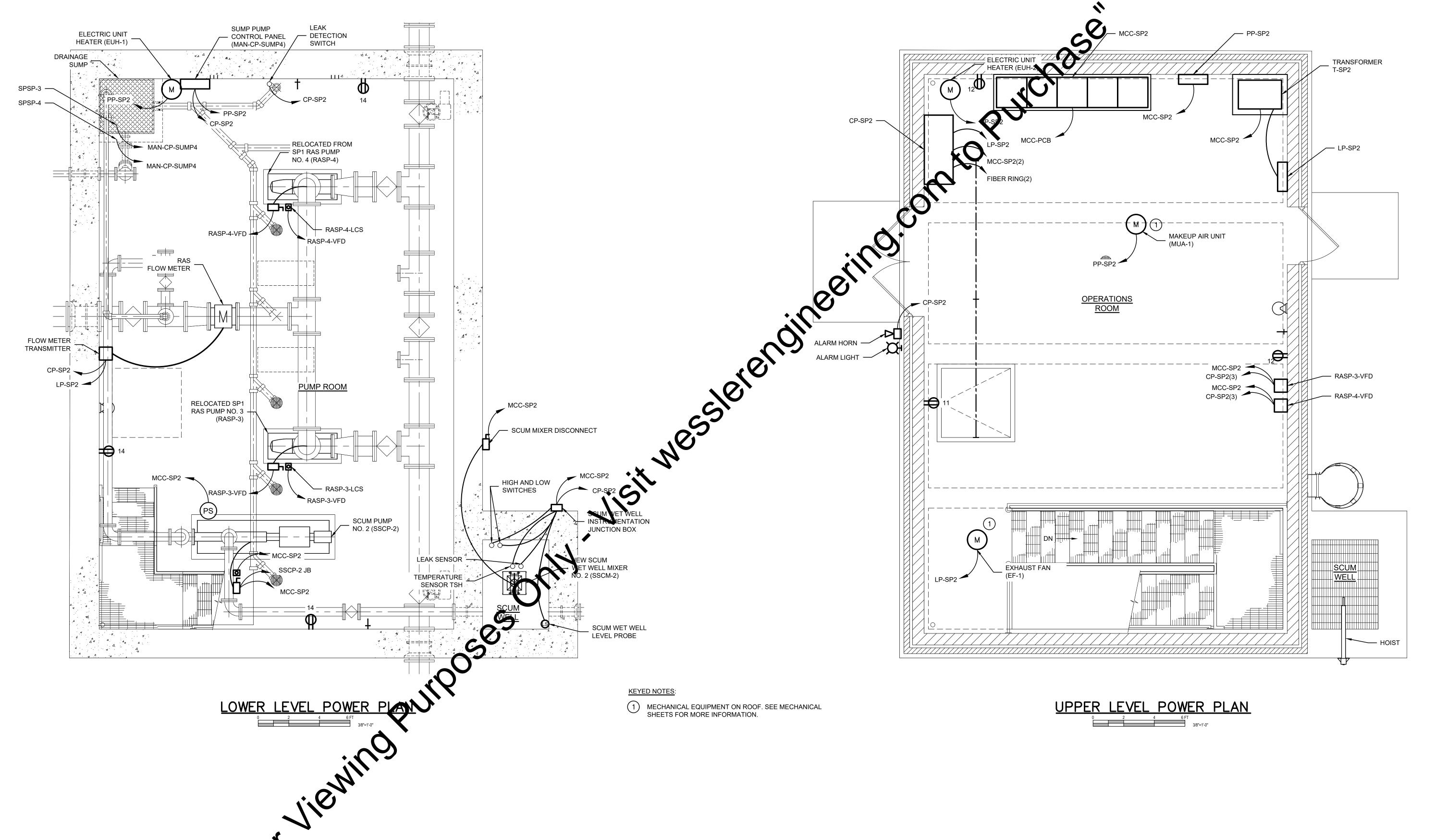












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PROJECT NUMBER

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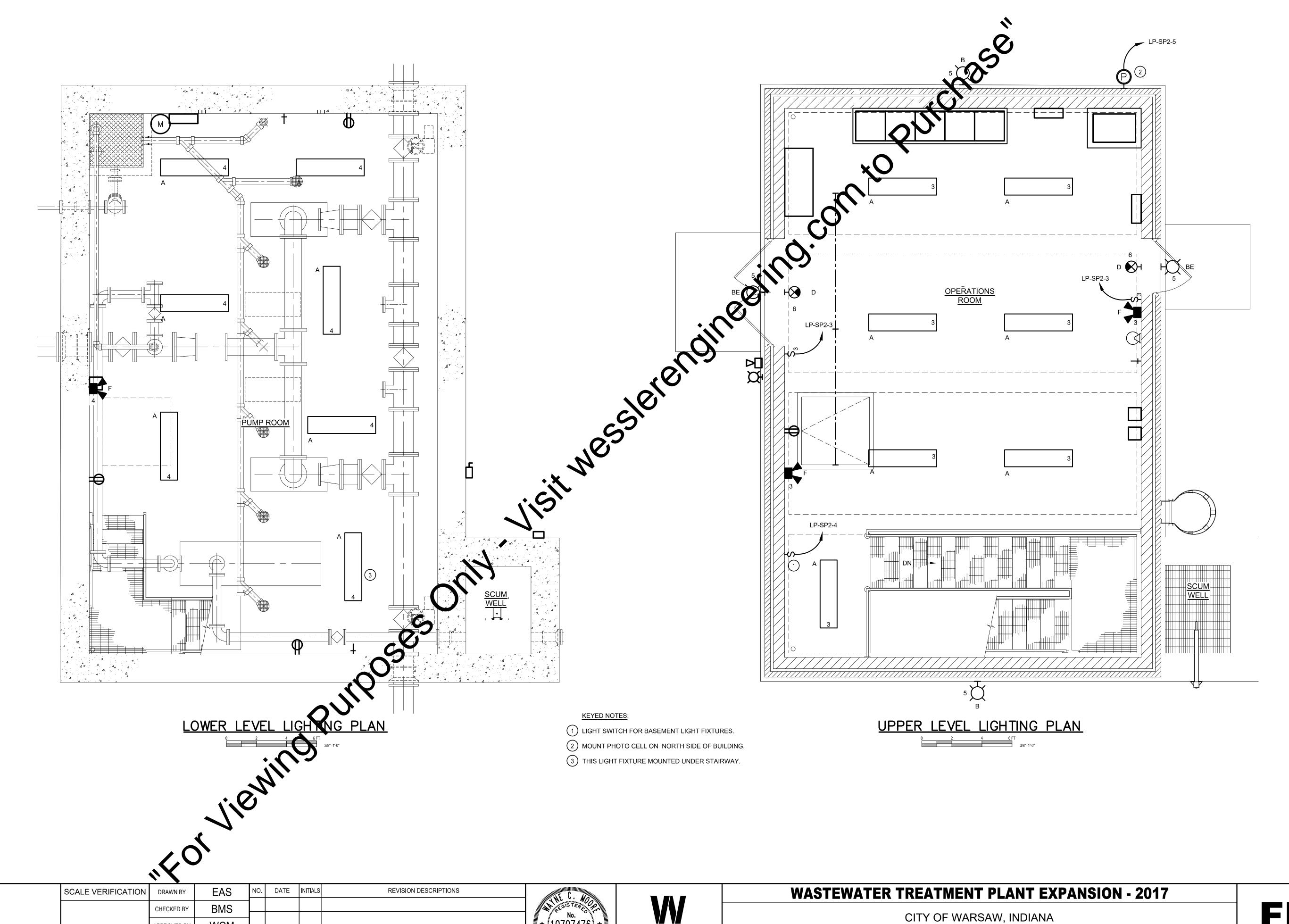
CITY OF WARSAW, INDIANA

NEW SLUDGE PUMP STATION NO. 2 POWER PLANS FE04

PAGE NO.

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WESSLER ENGINEERING

More than a Project™

WCM

ISSUE DATE

SEPTEMBER 4, 2018

PROJECT NUMBER

162813-04-003

APPROVED BY

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**NEW SLUDGE PUMP STATION NO. 2** 

FE05 PAGE NO.

**LIGHTING PLANS** 

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